

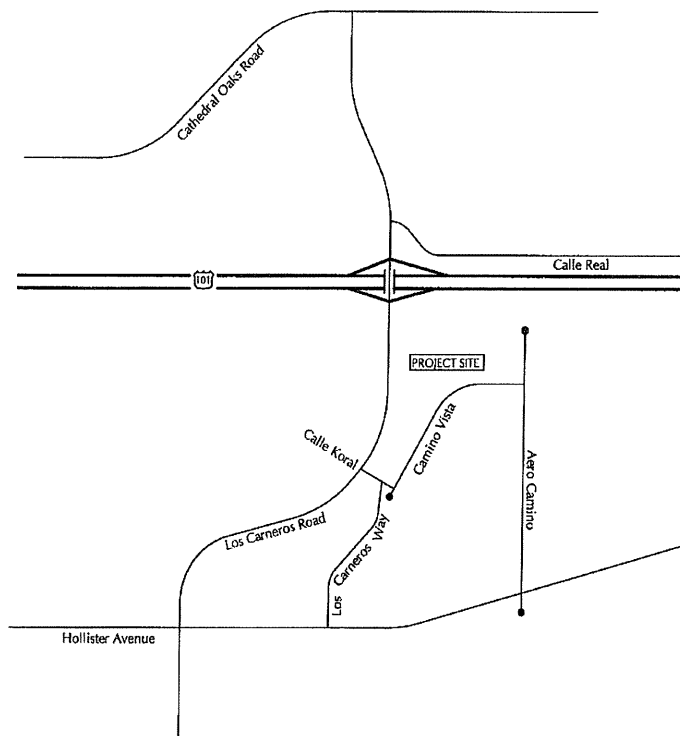
Appendix I

Traffic Study & Construction Trip Analysis



HERITAGE RIDGE PROJECT CITY OF GOLETA, CALIFORNIA

UPDATED TRAFFIC, CIRCULATION, AND PARKING STUDY



January 29, 2016

ATE Project #12089.01

Prepared for:
The Towbes Group, Inc.
21 East Victoria Street, Suite 200
Santa Barbara, California 93101



ASSOCIATED TRANSPORTATION ENGINEERS

100 North Hope Avenue, Suite 4, Santa Barbara, CA 93110-1686 • (805) 687-4418 • FAX (805) 682-850



ASSOCIATED TRANSPORTATION ENGINEERS

100 N. Hope Avenue, Suite 4, Santa Barbara, CA 93110 • (805) 687-4418 • FAX (805) 682-8509

Since 1978

Richard L. Pool, P.E.
Scott A. Schell, AICP, PTP

January 29, 2016

12089.01R05

Craig Minus
The Towbes Group, Inc.
21 East Victoria Street, Suite 200
Santa Barbara, California 93101

UPDATED TRAFFIC, CIRCULATION, AND PARKING STUDY FOR THE HERITAGE RIDGE PROJECT - CITY OF GOLETA

Associated Transportation Engineers (ATE) has prepared the following traffic, circulation, and parking study for the Heritage Ridge Project, located in the City of Goleta. The study evaluates potential traffic, circulation, and parking impacts associated with the project and identifies improvements where required. The following study has been updated to address comments provided in the Linscott, Law, & Greenspan peer review letter dated June 29, 2015 and by the City of Goleta in January 2016.

Associated Transportation Engineers

Scott A. Schell, AICP, PTP
Principal Transportation Planner

CONTENTS

INTRODUCTION	1
PROJECT DESCRIPTION	1
EXISTING CONDITIONS	1
Street Network	1
Existing Intersection Operations.....	5
PROJECT-SPECIFIC ANALYSIS	10
Project Trip Generation	10
Project Trip Distribution	10
Existing + Project Roadway Operations	11
Existing + Project Intersection Operations.....	14
Village at Los Carneros Project Improvements.....	16
CUMULATIVE ANALYSIS	18
Cumulative Traffic Volumes	18
Planned Improvements.....	18
Cumulative + Project Roadway Operations.....	18
Cumulative + Project Intersection Operations.....	21
Cumulative + Project + Improvements Intersection Operations.....	23
PARKING ANALYSIS	23
Parking Supply	23
City of Goleta Zoning Ordinance Parking Requirements	24
Parking Demand Analysis.....	24
SITE ACCESS AND CIRCULATION	26
ALTERNATIVE MODES OF TRANSPORTATION.....	27
Pedestrian Facilities	27
Bicycle Facilities.....	28
Transit.....	30
CONGESTION MANAGEMENT PROGRAM ANALYSIS.....	30
Impact Criteria	30
Potential Intersection Impacts	31
Potential Freeway Impacts	31

REFERENCES AND PERSONS CONTACTED.....	32
TECHNICAL APPENDIX.....	33

TABLES

Table 1	Existing Roadway Operations.....	5
Table 2	Existing Intersection Operations.....	7
Table 3	Project Trip Generation	10
Table 4	Project Trip Distribution	11
Table 5	Existing+ Project Roadway Operations.....	14
Table 6	Existing+ Project Intersection Operations - A.M. Peak Hour	15
Table 7	Existing+ Project Intersection Operations - P.M. Peak Hour.....	15
Table 8	Existing+ Project Intersection Operations – Los Carneros Road/Calle Koral Intersection w/ Western Leg	16
Table 9	Cumulative+ Project Roadway Operations.....	21
Table 10	Cumulative+ Project Intersection Operations - A.M. Peak Hour	22
Table 11	Cumulative+ Project Intersection Operations - P.M. Peak Hour.....	22
Table 12	Cumulative+ Project+ Improvements Intersection Operations – P.M. Peak Hour.....	23
Table 13	City of Goleta Zoning Ordinance Parking Requirements	24
Table 14	Senior Housing Parking Demands.....	25
Table 15	Apartment Parking Demands	26
Table 16	Public Park Parking Demands.....	26

FIGURES

Figure 1	Project Site Location.....	2
Figure 2	Project Site Plan	3
Figure 3	Existing Traffic Volumes	6
Figure 4	Intersection Lane Geometry and Traffic Controls.....	8
Figure 5	Project Trip Distribution and Assignment.....	12
Figure 6	Existing+ Project Traffic Volumes.....	13
Figure 7	Existing+ Project Traffic Volumes - Los Carneros Road/Calle Koral Intersection w/ Western Leg	17
Figure 8	Cumulative Traffic Volumes.....	19
Figure 9	Cumulative+ Project Traffic Volumes.....	20
Figure 10	Project-Added Driveway Volumes	28

INTRODUCTION

The following report contains an analysis of the potential traffic and circulation impacts associated with the Heritage Ridge Project, located in the City of Goleta. The report evaluates existing and future traffic conditions within the project study-area and recommends improvements where necessary. The report also contains an analysis of the project's site access, circulation, and parking plan. An analysis of the project's consistency with the policies outlined in the Congestion Management Program (CMP) is also provided. The following report has been updated to address comments provided in the Linscott, Law, & Greenspan peer review letter dated June 29, 2015 and additional comments provided by the City of Goleta in January 2016.

PROJECT DESCRIPTION

The project is proposing to construct 132 senior apartment units, 228 apartment units, and a 2-acre park on a currently vacant site located on the east side of Los Carneros Way north of the Calle Koral intersection in the western area of the City of Goleta. Figure 1 shows the project location within the City. Access to the site would be provided via three driveways on Camino Vista, which extends along the southern frontage of the site. The project is proposing to provide 152 parking spaces for the senior apartments, 345 parking spaces for the apartments, and 13 spaces for the public park. Figure 2 presents the project site plan.

EXISTING CONDITIONS

Street Network

The project site is served by a network of highways, arterial streets, and collector streets, as illustrated in Figure 1. The following text provides a brief discussion of the major components of the study-area street network.

U.S. Highway 101, located north of the project site, is a multi-lane interstate highway serving the Pacific coast between Los Angeles and the state of Washington. This highway is the principal route between the City of Goleta and the adjacent cities of Santa Barbara, Carpinteria, and Ventura to the south and the cities of Buellton and Santa Maria to the north. Access to U.S. Highway 101 would be provided via the Los Carneros Road interchange.

Hollister Avenue, located south of the project, is an arterial roadway that serves as the main east-west surface street through the community of Goleta. Hollister Avenue is a 4-lane divided arterial with on-street bike lanes. Within the study area, Hollister Avenue is signalized at the Los Carneros Road, Los Carneros Way, and Aero Camino intersections.



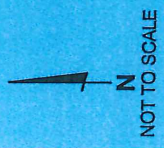
Source: Google Maps

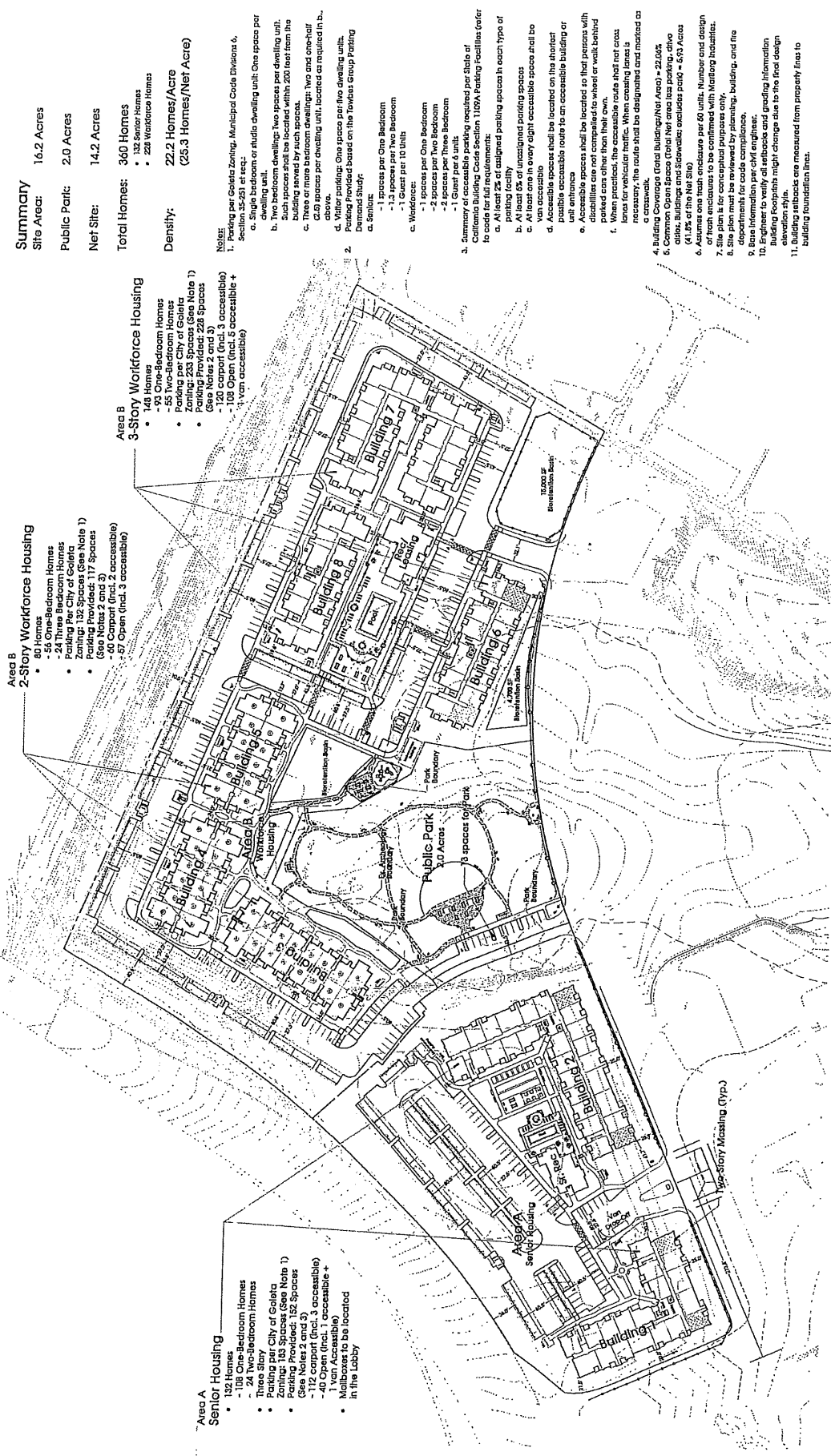
FIGURE 1

MMF - #12089.01

EXISTING STREET NETWORK AND PROJECT SITE LOCATION

ASSOCIATED
TRANSPORTATION
ENGINEERS





Summary
Site Area: 16.2 Acres
Public Park: 2.0 Acres
Net Site: 14.2 Acres
Total Homes: 360 Homes
• 132 Senior Homes
• 228 Workforce Homes

Density:
22.2 Homes/Acre
(25.3 Homes/Net Acre)

- Notes:**
- Parking per Goleta Zoning, Municipal Code Divisions 4, Section 35-251 et seq.:
 - Single bedroom or studio dwelling unit: One space per dwelling unit.
 - Each room in a dwelling: Two spaces per dwelling unit. Each space shall be located within 200 feet from the building served by such space.
 - Three or more bedroom dwellings: Two and one-half (2.5) spaces per dwelling unit, located as required in b. above.
 - Off-street parking: One space per two dwelling units.
 - Parking provided based on the various group rating Demand Study:
 - Senior:
 - 1 space per One Bedroom
 - 1.3 spaces per Two Bedroom
 - 1.5 spaces per Three Bedroom
 - 1.7 spaces per Four Bedroom
 - 1.9 spaces per Five Bedroom
 - 2.1 spaces per Six Bedroom
 - 2.3 spaces per Seven Bedroom
 - 2.5 spaces per Eight Bedroom
 - 2.7 spaces per Nine Bedroom
 - 2.9 spaces per Ten Bedroom
 - 3.1 spaces per Eleven Bedroom
 - 3.3 spaces per Twelve Bedroom
 - 3.5 spaces per Thirteen Bedroom
 - 3.7 spaces per Fourteen Bedroom
 - 3.9 spaces per Fifteen Bedroom
 - 4.1 spaces per Sixteen Bedroom
 - 4.3 spaces per Seventeen Bedroom
 - 4.5 spaces per Eighteen Bedroom
 - 4.7 spaces per Nineteen Bedroom
 - 4.9 spaces per Twenty Bedroom
 - 5.1 spaces per Twenty One Bedroom
 - 5.3 spaces per Twenty Two Bedroom
 - 5.5 spaces per Twenty Three Bedroom
 - 5.7 spaces per Twenty Four Bedroom
 - 5.9 spaces per Twenty Five Bedroom
 - 6.1 spaces per Twenty Six Bedroom
 - 6.3 spaces per Twenty Seven Bedroom
 - 6.5 spaces per Twenty Eight Bedroom
 - 6.7 spaces per Twenty Nine Bedroom
 - 6.9 spaces per Thirty Bedroom
 - 7.1 spaces per Thirty One Bedroom
 - 7.3 spaces per Thirty Two Bedroom
 - 7.5 spaces per Thirty Three Bedroom
 - 7.7 spaces per Thirty Four Bedroom
 - 7.9 spaces per Thirty Five Bedroom
 - 8.1 spaces per Thirty Six Bedroom
 - 8.3 spaces per Thirty Seven Bedroom
 - 8.5 spaces per Thirty Eight Bedroom
 - 8.7 spaces per Thirty Nine Bedroom
 - 8.9 spaces per Forty Bedroom
 - 9.1 spaces per Forty One Bedroom
 - 9.3 spaces per Forty Two Bedroom
 - 9.5 spaces per Forty Three Bedroom
 - 9.7 spaces per Forty Four Bedroom
 - 9.9 spaces per Forty Five Bedroom
 - 10.1 spaces per Forty Six Bedroom
 - 10.3 spaces per Forty Seven Bedroom
 - 10.5 spaces per Forty Eight Bedroom
 - 10.7 spaces per Forty Nine Bedroom
 - 10.9 spaces per Fifty Bedroom
 - 11.1 spaces per Fifty One Bedroom
 - 11.3 spaces per Fifty Two Bedroom
 - 11.5 spaces per Fifty Three Bedroom
 - 11.7 spaces per Fifty Four Bedroom
 - 11.9 spaces per Fifty Five Bedroom
 - 12.1 spaces per Fifty Six Bedroom
 - 12.3 spaces per Fifty Seven Bedroom
 - 12.5 spaces per Fifty Eight Bedroom
 - 12.7 spaces per Fifty Nine Bedroom
 - 12.9 spaces per Sixty Bedroom
 - 13.1 spaces per Sixty One Bedroom
 - 13.3 spaces per Sixty Two Bedroom
 - 13.5 spaces per Sixty Three Bedroom
 - 13.7 spaces per Sixty Four Bedroom
 - 13.9 spaces per Sixty Five Bedroom
 - 14.1 spaces per Sixty Six Bedroom
 - 14.3 spaces per Sixty Seven Bedroom
 - 14.5 spaces per Sixty Eight Bedroom
 - 14.7 spaces per Sixty Nine Bedroom
 - 14.9 spaces per Seventy Bedroom
 - 15.1 spaces per Seventy One Bedroom
 - 15.3 spaces per Seventy Two Bedroom
 - 15.5 spaces per Seventy Three Bedroom
 - 15.7 spaces per Seventy Four Bedroom
 - 15.9 spaces per Seventy Five Bedroom
 - 16.1 spaces per Seventy Six Bedroom
 - 16.3 spaces per Seventy Seven Bedroom
 - 16.5 spaces per Seventy Eight Bedroom
 - 16.7 spaces per Seventy Nine Bedroom
 - 16.9 spaces per Eighty Bedroom
 - 17.1 spaces per Eighty One Bedroom
 - 17.3 spaces per Eighty Two Bedroom
 - 17.5 spaces per Eighty Three Bedroom
 - 17.7 spaces per Eighty Four Bedroom
 - 17.9 spaces per Eighty Five Bedroom
 - 18.1 spaces per Eighty Six Bedroom
 - 18.3 spaces per Eighty Seven Bedroom
 - 18.5 spaces per Eighty Eight Bedroom
 - 18.7 spaces per Eighty Nine Bedroom
 - 18.9 spaces per Ninety Bedroom
 - 19.1 spaces per Ninety One Bedroom
 - 19.3 spaces per Ninety Two Bedroom
 - 19.5 spaces per Ninety Three Bedroom
 - 19.7 spaces per Ninety Four Bedroom
 - 19.9 spaces per Ninety Five Bedroom
 - 20.1 spaces per Ninety Six Bedroom
 - 20.3 spaces per Ninety Seven Bedroom
 - 20.5 spaces per Ninety Eight Bedroom
 - 20.7 spaces per Ninety Nine Bedroom
 - 20.9 spaces per One Hundred Bedroom
 - Workforce:
 - 1 space per One Bedroom
 - 2 spaces per Two Bedroom
 - 3 spaces per Three Bedroom
 - 4 spaces per Four Bedroom
 - 5 spaces per Five Bedroom
 - 6 spaces per Six Bedroom
 - 7 spaces per Seven Bedroom
 - 8 spaces per Eight Bedroom
 - 9 spaces per Nine Bedroom
 - 10 spaces per Ten Bedroom
 - 11 spaces per Eleven Bedroom
 - 12 spaces per Twelve Bedroom
 - 13 spaces per Thirteen Bedroom
 - 14 spaces per Fourteen Bedroom
 - 15 spaces per Fifteen Bedroom
 - 16 spaces per Sixteen Bedroom
 - 17 spaces per Seventeen Bedroom
 - 18 spaces per Eighteen Bedroom
 - 19 spaces per Nineteen Bedroom
 - 20 spaces per Twenty Bedroom
 - 21 spaces per Twenty One Bedroom
 - 22 spaces per Twenty Two Bedroom
 - 23 spaces per Twenty Three Bedroom
 - 24 spaces per Twenty Four Bedroom
 - 25 spaces per Twenty Five Bedroom
 - 26 spaces per Twenty Six Bedroom
 - 27 spaces per Twenty Seven Bedroom
 - 28 spaces per Twenty Eight Bedroom
 - 29 spaces per Twenty Nine Bedroom
 - 30 spaces per Thirty Bedroom
 - 31 spaces per Thirty One Bedroom
 - 32 spaces per Thirty Two Bedroom
 - 33 spaces per Thirty Three Bedroom
 - 34 spaces per Thirty Four Bedroom
 - 35 spaces per Thirty Five Bedroom
 - 36 spaces per Thirty Six Bedroom
 - 37 spaces per Thirty Seven Bedroom
 - 38 spaces per Thirty Eight Bedroom
 - 39 spaces per Thirty Nine Bedroom
 - 40 spaces per Forty Bedroom
 - 41 spaces per Forty One Bedroom
 - 42 spaces per Forty Two Bedroom
 - 43 spaces per Forty Three Bedroom
 - 44 spaces per Forty Four Bedroom
 - 45 spaces per Forty Five Bedroom
 - 46 spaces per Forty Six Bedroom
 - 47 spaces per Forty Seven Bedroom
 - 48 spaces per Forty Eight Bedroom
 - 49 spaces per Forty Nine Bedroom
 - 50 spaces per Fifty Bedroom
 - 51 spaces per Fifty One Bedroom
 - 52 spaces per Fifty Two Bedroom
 - 53 spaces per Fifty Three Bedroom
 - 54 spaces per Fifty Four Bedroom
 - 55 spaces per Fifty Five Bedroom
 - 56 spaces per Fifty Six Bedroom
 - 57 spaces per Fifty Seven Bedroom
 - 58 spaces per Fifty Eight Bedroom
 - 59 spaces per Fifty Nine Bedroom
 - 60 spaces per Sixty Bedroom
 - 61 spaces per Sixty One Bedroom
 - 62 spaces per Sixty Two Bedroom
 - 63 spaces per Sixty Three Bedroom
 - 64 spaces per Sixty Four Bedroom
 - 65 spaces per Sixty Five Bedroom
 - 66 spaces per Sixty Six Bedroom
 - 67 spaces per Sixty Seven Bedroom
 - 68 spaces per Sixty Eight Bedroom
 - 69 spaces per Sixty Nine Bedroom
 - 70 spaces per Seventy Bedroom
 - 71 spaces per Seventy One Bedroom
 - 72 spaces per Seventy Two Bedroom
 - 73 spaces per Seventy Three Bedroom
 - 74 spaces per Seventy Four Bedroom
 - 75 spaces per Seventy Five Bedroom
 - 76 spaces per Seventy Six Bedroom
 - 77 spaces per Seventy Seven Bedroom
 - 78 spaces per Seventy Eight Bedroom
 - 79 spaces per Seventy Nine Bedroom
 - 80 spaces per Eighty Bedroom
 - 81 spaces per Eighty One Bedroom
 - 82 spaces per Eighty Two Bedroom
 - 83 spaces per Eighty Three Bedroom
 - 84 spaces per Eighty Four Bedroom
 - 85 spaces per Eighty Five Bedroom
 - 86 spaces per Eighty Six Bedroom
 - 87 spaces per Eighty Seven Bedroom
 - 88 spaces per Eighty Eight Bedroom
 - 89 spaces per Eighty Nine Bedroom
 - 90 spaces per Ninety Bedroom
 - 91 spaces per Ninety One Bedroom
 - 92 spaces per Ninety Two Bedroom
 - 93 spaces per Ninety Three Bedroom
 - 94 spaces per Ninety Four Bedroom
 - 95 spaces per Ninety Five Bedroom
 - 96 spaces per Ninety Six Bedroom
 - 97 spaces per Ninety Seven Bedroom
 - 98 spaces per Ninety Eight Bedroom
 - 99 spaces per Ninety Nine Bedroom
 - 100 spaces per One Hundred Bedroom

Corrected
Conceptual Site Plan

Heritage Ridge
Goleta, Ca
Michael Towbes, LLC

PROJECT SITE PLAN

WILLIAM KEZMALCHALCH ASSOCIATES, INC. © 2016

September 12, 2016

The information shown and derived by this THE FORCES GROUP, INC.

ASSOCIATED
TRANSPORTATION
ENGINEERS

FIGURE 2

MMF - #12089.01

Los Carneros Road, located west of the project site, is a north-south arterial street. North of Hollister Avenue, Los Carneros Road extends as a 4- to 5-lane roadway connecting with the U.S. 101 interchange and continues north to its terminus at Cathedral Oak Road. Los Carneros Road has recently been widened to 4-lanes south of Hollister Avenue to Discovery Drive. South of Discovery Drive, Los Carneros Road continues as a 2-lane roadway with in Santa Barbara County and provides access to the Isla Vista-UCSB area. Within the study-area, Los Carneros Road is signalized at the U.S. 101 Northbound Ramps, Southbound Ramps, Calle Koral, and Hollister Avenue intersections.

Los Carneros Way is a 2-lane road located south of the project site that extends between Calle Koral and Hollister Avenue. Los Carneros Way is stop controlled at the Calle Koral intersection, and the Hollister Avenue/Los Carneros Way intersection is controlled by traffic signals.

Calle Koral, located southwest of the project, is a 2-lane road that extends from Los Carneros Road to Camino Vista. The Calle Koral/Los Carneros Road intersection is controlled by traffic signals and the Calle Koral/Camino Vista intersection is uncontrolled.

Aero Camino, located east of the project site, is a 2-lane road that serves the existing industrial land uses and extends north from Hollister Avenue to its terminus south of U.S. Highway 101. The Hollister Avenue/Aero Camino intersection is controlled by traffic signals.

Camino Vista, located along the southern frontage of the project site, is a 2-lane road that extends easterly from Calle Koral serving the Willow Springs I and Willow Springs II apartment complexes. The segment of Camino Vista between the Willow Springs I apartments and Aero Camino was recently constructed as part of the Willow Springs II development and is now open for public travel.

Recently Constructed Improvements

The City of Goleta recently finished replacing the Los Carneros Road bridge over the Union Pacific Railroad adjacent to the U.S. 101 interchange. The new bridge includes a dedicated right-turn lane for the northbound approach of Los Carneros Road to the U.S. 101 Southbound On-Ramp and two travel lanes in both directions. The right-turn lane extends southerly from the U.S. 101 Southbound On-Ramp to Calle Koral. Los Carneros Road was also widened south of the Calle Koral intersection to provide three northbound travel lanes. The construction project also installed Class II bike lanes in both directions and a pedestrian sidewalk along the west side of the road.

The segment of Camino Vista between the existing Willow Springs I apartments on the west and Aero Camino on the east was recently constructed as part of the Willow Springs II development. This new roadway segment is now open for public travel and provides a new travel route from the Aero Camino corridor to the Los Carneros Road interchange.

Existing Roadway Operations

Figure 3 shows the existing average daily traffic (ADT) volumes for the study-area roadways. Existing roadway volumes were obtained from counts conducted in 2013 (count data contained in the Technical Appendix for reference). The operational characteristics of the study-area roadways were analyzed based on the City of Goleta engineering roadway design capacities (summarized in the Technical Appendix). Table 1 shows the existing ADT volumes and the City's Acceptable Capacity thresholds for the key roadways in the project study-area.

Table 1
Existing Roadway Operations

Roadway Segment	Roadway Classification	Geometry	Acceptable Capacity	Existing ADT
Los Carneros Road s/o U.S. 101 SB Ramps (a)	Major Arterial	5-Lane	47,000	23,300
Los Carneros Road s/o Hollister Avenue (b)	Major Arterial	4-Lane	34,000	17,700
Hollister Avenue w/o Los Carneros Road	Major Arterial	4-Lane	34,000	17,300
Hollister Avenue e/o Los Carneros Road	Major Arterial	4-Lane	34,000	14,400

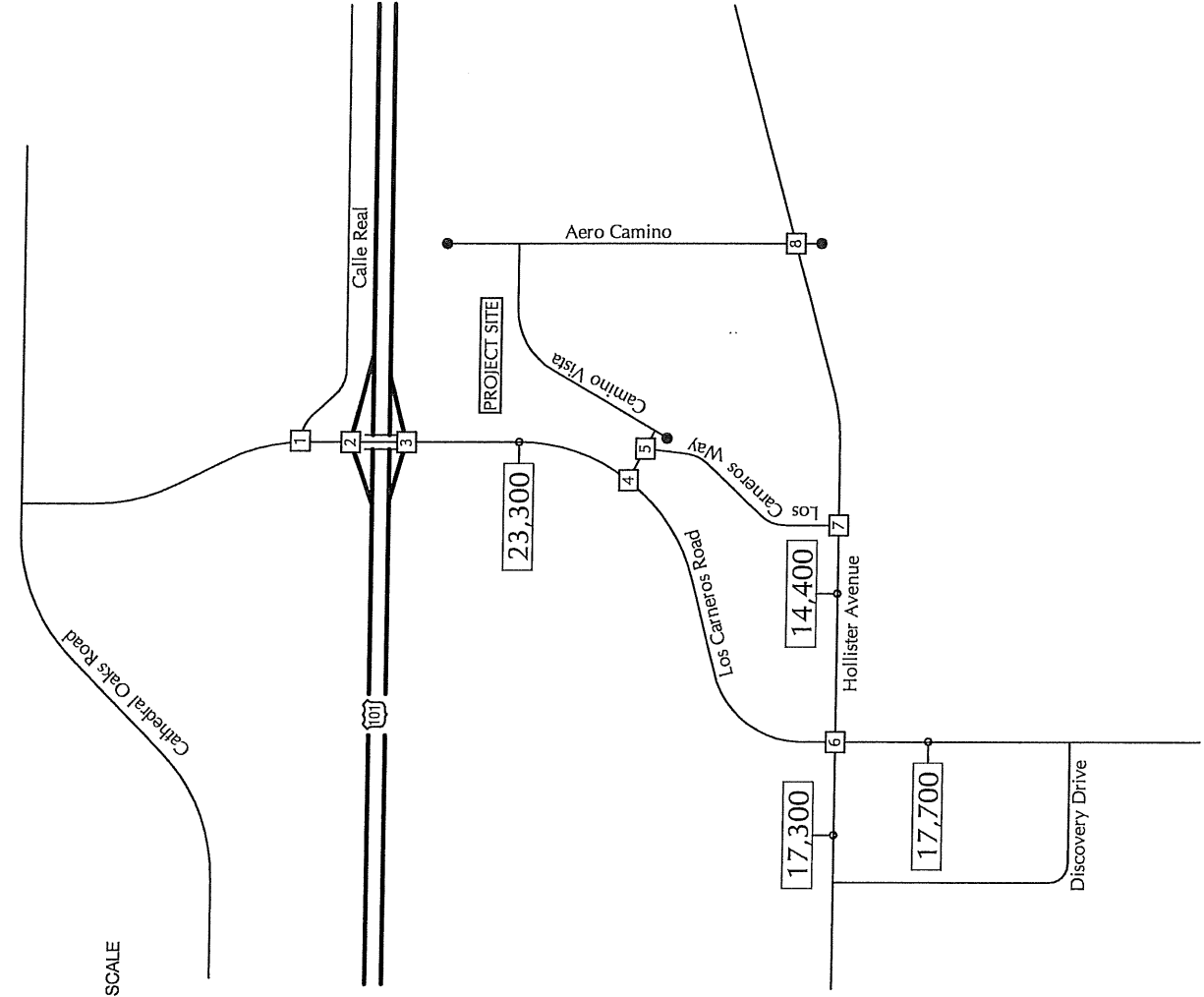
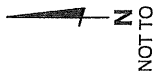
(a) Roadway recently widened to 5-lanes between U.S. 101 and Calle Koral.

(b) Roadway recently widened to 4-lanes between Hollister Avenue and Discovery Drive.

The data in Table 1 shows that the study-area roadway segments currently carry volumes within the City of Goleta's Acceptable Capacity designations.

Existing Intersection Operations

Because traffic flow on urban arterials is most constrained at intersections, detailed traffic flow analyses focus on the operating conditions of critical intersections during peak travel periods. In rating intersection operations, "Levels of Service" (LOS) A through F are used, with LOS A indicating free flow operations and LOS F indicating congested operations (more complete definitions of levels of service are included in the Technical Appendix). The City of Goleta has established LOS C as the minimum acceptable operating standard for intersections.



1	12(20) 86(197)	(14)24 (241)319	(162)396 (118)209 (6)5
2	250(423) 107(110)	(53)24 (2)7 (805)436	(276)618 (21)236
3	43(72) 601(1168)		(357)960 (160)773
4	97(134) 1(2) 46(216)		
5	101(247) 577(1137)	(120)295 (13)5	(4)26 (399)1394
6	44(25) 443(238) 127(121)	(41)29 (254)642 (76)127	(104)110 (282)546 (29)229
7	243(76) 385(325) 87(199)		34(22) 517(408)
8	67(38) 1(1) 104(31)		(89)30 (340)810 (3)5

LEGEND
 (XX)XX - (A.M.)P.M. Peak Hour Volume
 X - Average Daily Traffic Volume



ASSOCIATED
 TRANSPORTATION
 ENGINEERS

EXISTING TRAFFIC VOLUMES

FIGURE 3

MMF - #12089.01

Existing peak hour volumes for the study-area intersections were obtained from traffic counts conducted in 2013 and 2012 (traffic count data is contained in the Technical Appendix for reference). Figure 3 shows the peak hour turning movements for the study-area intersections. Figure 4 presents the existing lane geometry and traffic controls for the study-area intersections.

Levels of service were calculated for the signalized intersections using the "Intersection Capacity Utilization" (ICU) methodology. Levels of service for the stop-sign controlled and roundabout intersections were calculated using the methodology outlined in the Highway Capacity Manual (HCM).¹ Table 2 presents the existing levels of service for the study-area intersections (calculation worksheets are contained in the Technical Appendix).

Table 2
Existing Intersection Operations

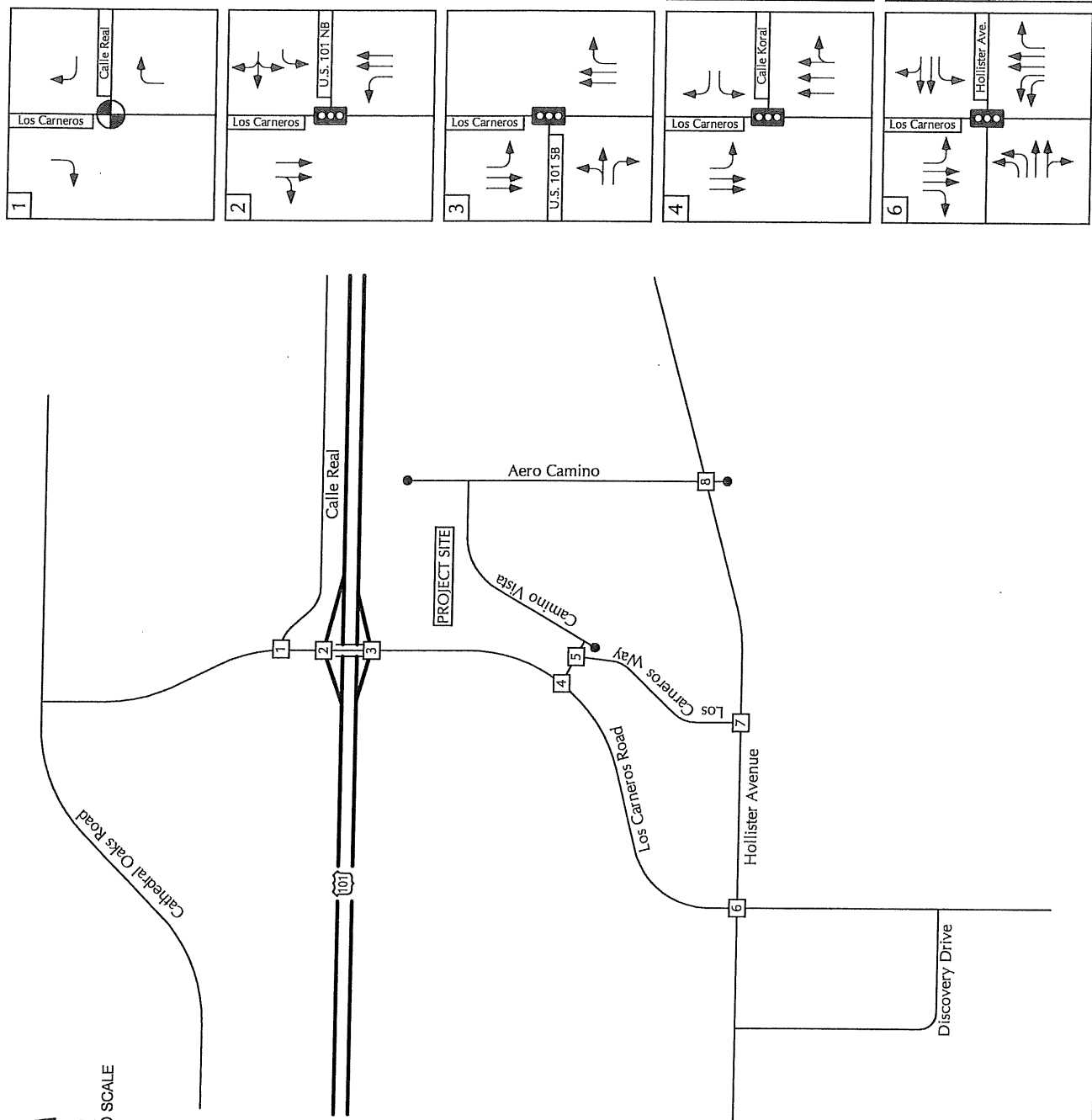
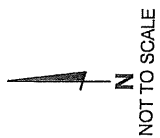
Intersection	Control	A.M. Peak Hour		P.M. Peak hour	
		V/C	LOS	V/C	LOS
Los Carneros Road/Calle Real (a)	Roundabout	6.4 sec.	LOS A	9.4 sec.	LOS A
Los Carneros Road/U.S. 101 NB Ramps	Signal	0.54	LOS A	0.49	LOS A
Los Carneros Road/U.S. 101 SB Ramps (b)	Signal	0.55	LOS A	0.72	LOS C
Los Carneros Road/Calle Koral (b)	Signal	0.46	LOS A	0.51	LOS A
Calle Koral/Los Carneros Way (a)	Stop Sign	8.3 sec.	LOS A	9.8 sec.	LOS A
Calle/Koral/Camino Vista	Yield	N/A	N/A	N/A	N/A
Los Carneros Road/Hollister Avenue	Signal	0.39	LOS A	0.59	LOS A
Los Carneros Way/Hollister Avenue	Signal	0.28	LOS A	0.43	LOS A
Aero Camino/Hollister Avenue	Signal	0.31	LOS A	0.44	LOS A

(a) Unsignalized intersection. LOS based on average weighted delay per vehicle in seconds.

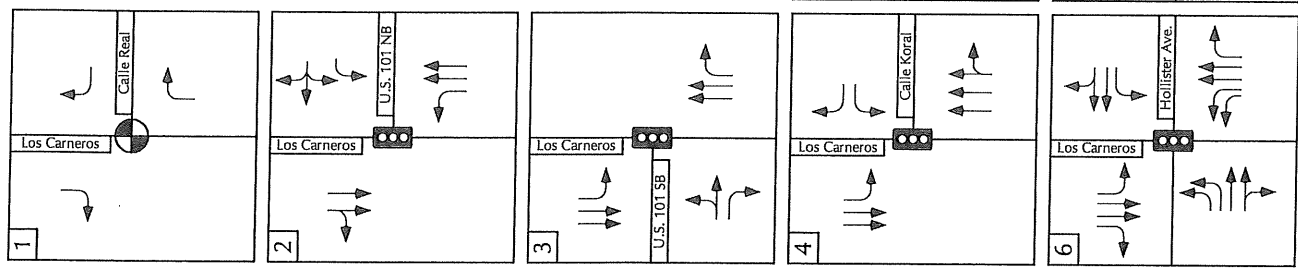
(b) LOS assumes recently completed improvements.

The data presented in Table 2 show that the study-area intersections currently operate acceptably at LOS C or better.

¹ Highway Capacity Manual, Transportation Research Special Report 209, National Research Council, 2010.



- LEGEND**
- Lane Geometry
 - Signalized Intersection
 - Stopped Approach
 - Roundabout



ASSOCIATED
TRANSPORTATION
ENGINEERS

INTERSECTION LANE GEOMETRY AND TRAFFIC CONTROLS

FIGURE 4

MMF - #12089.01

THRESHOLDS OF SIGNIFICANCE

The City of Goleta's CEQA traffic impact thresholds were used for this analysis and include the following criteria:

- A. The project will result in a significant impact on transportation and circulation if proposed project traffic increases the volume to capacity (V/C) ratio at local intersections by the values provided in the following table:

Significant Changes in Levels of Service	
Intersection Level of Service (Including Project)	Increase in V/C or Trips Greater Than
LOS A	0.20
LOS B	0.15
LOS C	0.10
LOS D	15 Trips
LOS E	10 Trips
LOS F	5 Trips

- B. The project's access to a major road or arterial road would require access that would create an unsafe situation, a new traffic signal, or major revisions to an existing traffic signal.
- C. The project would add traffic to a roadway that has design features (e.g., narrow width, road-side ditches, sharp curves, poor sight distance, inadequate pavement structure) that would become a potential safety problem with the addition of project traffic.
- D. Project traffic would utilize a substantial portion of an intersection's capacity where the intersection is currently operating at acceptable levels of service, but with cumulative traffic would degrade to or approach LOS D (V/C 0.80) or lower. Substantial is defined as a minimum change of 0.03 for an intersection which would operate from 0.80 to 0.85, a change of 0.02 for an intersection which would operate from 0.86 to 0.90 and a change of 0.01 for an intersection which would operate greater than 0.90 (LOS E or worse).

PROJECT-SPECIFIC ANALYSIS

Project Trip Generation

Trip generation estimates were developed for the Heritage Ridge Project using rates contained in the Institute of Transportation Engineers (ITE) Trip Generation report² and traffic counts conducted at the existing Willow Springs I apartment complex, located just south of the project site. For the proposed senior apartments, the trip generation analysis is based on the ITE rates for Senior Adult Housing - Attached (ITE Land Use Code #252). For the proposed apartments, the analysis uses the ITE Apartment rates (ITE Land Use Code #220) to calculate average daily trips.

A.M. and P.M. peak hour trip rates for the apartment units were developed from driveway counts conducted at the existing Willow Springs I apartments (driveway count data contained in the Technical Appendix). These rates better reflect local data and are slightly higher than the ITE average rates for apartment units. Table 3 presents the trip generation estimates for the Heritage Ridge Project.

Table 3
Project Trip Generation

Land Use	Size	ADT		A.M. Peak Hour		P.M. Peak Hour	
		Rate	Trips	Rate	Trips (In/Out)	Rate	Trips(In/Out)
Senior Apartments	132 Units	3.44	454	0.20	26 (9/17)	0.25	33 (18/15)
Apartments (a)	228 Units	6.65	1,516	0.65	148 (25/123)	0.66	150 (105/45)
Project Total:			1,970		174 (34/140)		183 (123/60)

(a) ADT rate based on ITE average rate for Apartments, AM and PM rates based on Willow Springs I study.

The data presented in Table 3 show that the project is forecast to generate 1,970 average daily trips, 174 A.M. peak hour trips, and 183 P.M. peak hour trips.

Project Trip Distribution

Trip distribution percentages were developed for the project based on existing traffic patterns observed at the Willow Springs I apartment complex. The distribution pattern assumes the extension of Camino Vista from Calle Koral to Aero Camino which was recently constructed as part of the Willow Springs II Project and is now open for vehicular access. Table 4 presents

² Trip Generation, Institute of Transportation Engineers, 9th Edition, 2012.

the trip distribution percentages developed for the project. Figure 5 illustrates the trip distribution pattern and shows the assignment of project-added traffic.

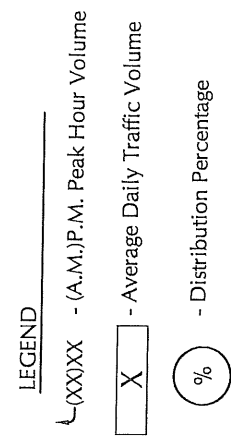
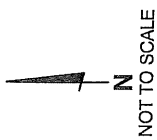
Table 4
Project Trip Distribution

Origin/Destination	Direction	Distribution %
U.S. 101	East	40%
	West	5%
Hollister Avenue	East (a)	10%
	West	20%
Los Carneros Road	South of Hollister	15%
Cathedral Oaks Road	East	5%
Calle Real	East	5%
Total:		100%

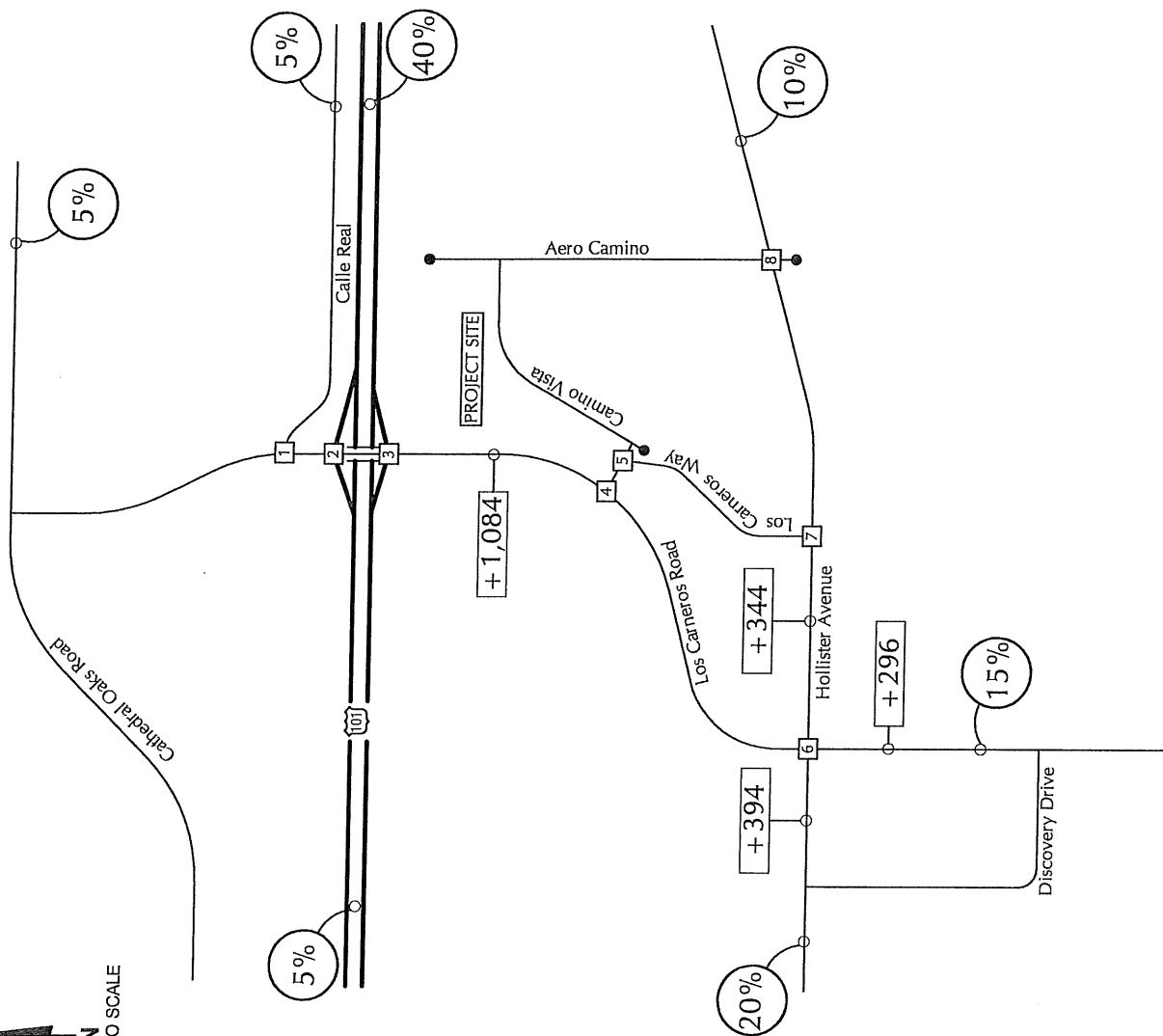
(a) Via Aero Camino

Existing + Project Roadway Operations

Existing + Project ADT volumes for the study-area roadways are shown on Figure 6. Table 5 presents the Existing and Existing + Project roadway volumes and identifies potential impacts based on the City of Goleta’s Acceptable Capacity thresholds.



1	6(2) →	└──(2)6 └──(7)3 └──(7)3
2	12(4) →	└──(13)49 └──(14)6 └──(7)3
3	61(17) →	└──(56)24 └──(21)9
4	6(2) →	└──(77)33 └──(24)10 └──(5)21
5	88(24) →	└──(101)43 └──(25)11 └──(7)22
6	4(10) 6(4) →	└──(14)6 └──(11)5 └──(3)9 └──(2)9
7	12(3) → 13(4) →	└──(22)7
8		└──(3)13

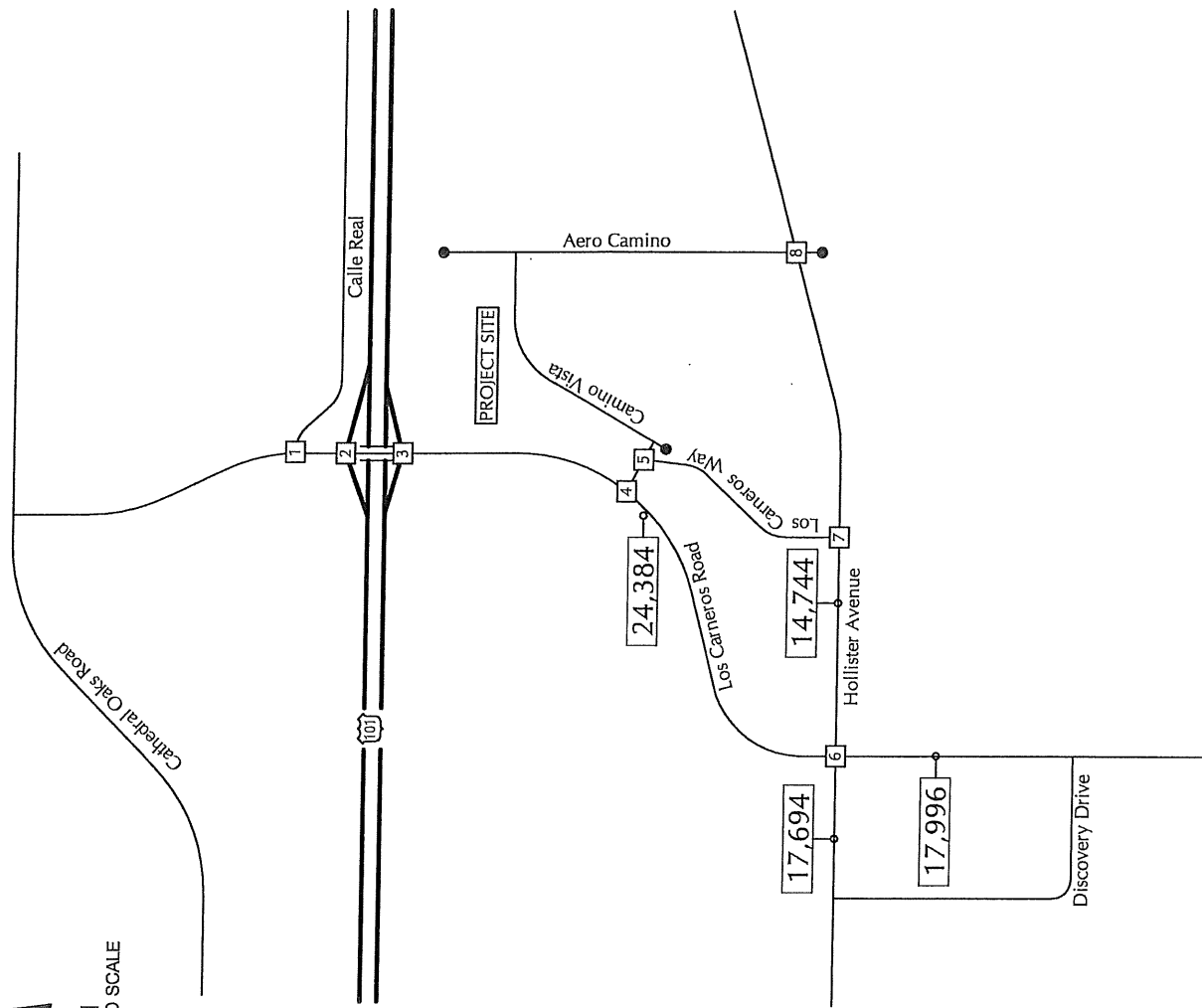
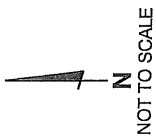


ASSOCIATED
TRANSPORTATION
ENGINEERS

PROJECT TRIP DISTRIBUTION AND ASSIGNMENT

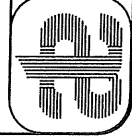
FIGURE 5

MMF - #12089.01



1	12(20) 92(199)	(14)24 (243)325 (169)399 (125)212 (6)5
2	262(427) 107(110)	(53)24 (2)7 (818)485 (290)624 (28)239
3	43(72) 662(1185)	(413)984 (181)782
4	97(134) 102 52(218)	(197)328 (37)15 (9)47 (399)1394
5	168(266) 577(1137)	(208)122 (66)18 (18)65 (40)225 159(81) 42(171)
6	17,694	(41)29 (268)648 (87)132
7	17,996	25(122) 26(52) 56(29) 517(408)
8	24,384	73(52) 1(1) 62(19) 16(46) 534(464) 13(16) (92)43 (340)810 (3)5 (2)5 (3)0 (7)33

LEGEND
 (XX)XX - (A.M.) P.M. Peak Hour Volume
 X - Average Daily Traffic Volume



ASSOCIATED
 TRANSPORTATION
 ENGINEERS

EXISTING + PROJECT TRAFFIC VOLUMES

FIGURE 6

MMF - #12089.01

**Table 5
Existing + Project Roadway Operations**

Roadway Segment	Average Daily Trips			% Change	Project Impact?	
	Acceptable Capacity	Existing ADT	Project Added ADT			Existing + Project ADT
Los Carneros Road s/o U.S. 101 SB Ramps	47,000	23,300	1,084	24,384	4.7%	No
Los Carneros Road s/o Hollister Avenue	34,000	17,700	296	17,996	1.7%	No
Hollister Avenue w/o Los Carneros Road	34,000	17,300	394	17,694	2.3%	No
Hollister Avenue e/o Los Carneros Road	34,000	14,400	344	14,744	2.4%	No

The data presented in Table 5 show that the Existing+Project roadway volumes would remain within the City’s Acceptable Capacity ratings with the addition of project traffic. The project would therefore not generate project-specific impacts to the study-area roadway segments.

Existing + Project Intersection Operations

Levels of service were calculated for the study-area intersections assuming the Existing + Project traffic volumes presented on Figure 6. As noted previously, the calculations assume completion of the Los Carneros Road Overhead Bridge Replacement Project that was recently completed, and the opening of the Camino Vista extension from Calle Koral to Aero Camino. The Camino Vista extension provides a new connection between Aero Camino and the Los Carneros Road interchange that alters the existing traffic patterns within the study-area. These traffic diversions are accounted for in the analysis. Tables 6 and 7 compare the Existing and Existing+Project levels of service and identify project-specific impacts based on City thresholds.

**Table 6
Existing + Project Intersection Operations – A.M. Peak Hour**

Intersection	Existing		Existing + Project		Project-Added		Project Impact?
	ICU/ Delay	LOS	ICU/ Delay	LOS	Trips	V/C	
Los Carneros Road/Calle Real	6.4 sec.	A	6.5 sec.	A	18	0.02	No
Los Carneros Rd/U.S. 101 NB Ramps	0.54	A	0.55	A	38	0.011	No
Los Carneros Rd/U.S. 101 SB Ramps (a)	0.55	A	0.56	A	96	0.005	No
Los Carneros Road/Calle Koral	0.46	A	0.46	A	125	0.001	No
Calle Koral/Los Carneros Way (b)	8.1 sec.	A	8.3 sec.	A	157	0.04	No
Calle Koral/Camino Vista (c)	N/A	N/A	N/A	N/A	157	N/A	No
Hollister Avenue/Los Carneros Road	0.39	A	0.40	A	61	0.008	No
Hollister Avenue/Los Carneros Way (b)	0.26	A	0.27	A	32	0.006	No
Hollister Avenue/Aero Camino (b)	0.29	A	0.30	A	17	0.009	No

- (a) Assumes completion of the Los Carneros Road Overhead Bridge Replacement Project.
 (b) Assumes diverted traffic on Camino Vista extension from Calle Koral to Aero Camino.
 (c) Level of Service not applicable, no conflicting movements.

**Table 7
Existing + Project Intersection Operations – P.M. Peak Hour**

Intersection	Existing		Existing + Project		Project-Added		Project Impact?
	V/C	LOS	V/C	LOS	Trips	V/C	
Los Carneros Road/Calle Real	9.4 sec.	A	9.5 sec.	A	18	0.02	No
Los Carneros Rd/U.S. 101 NB Ramps	0.49	A	0.51	A	70	0.021	No
Los Carneros Rd/U.S. 101 SB Ramps (a)	0.72	C	0.73	C	100	0.013	No
Los Carneros Road/Calle Koral	0.51	B	0.56	C	131	0.052	No
Calle Koral/Los Carneros Way (b)	9.5 sec.	A	10.2 sec.	B	164	0.072	No
Calle Koral/Camino Vista (c)	N/A	N/A	N/A	N/A	164	N/A	No
Hollister Avenue/Los Carneros Road	0.59	A	0.60	A	64	0.008	No
Hollister Avenue/Los Carneros Way (b)	0.42	A	0.43	A	33	0.015	No
Hollister Avenue/Aero Camino (b)	0.44	A	0.44	A	19	0.006	No

- (a) Assumes completion of the Los Carneros Road Overhead Bridge Replacement Project.
 (b) Assumes diverted traffic on Camino Vista extension from Calle Koral to Aero Camino.
 (c) Level of Service not applicable, no conflicting movements.

The data presented in Tables 6 and 7 show that the study-area intersections are forecast to operate at LOS C or better with the addition of project traffic. The project would not generate significant impacts to the study-area intersections based on the City’s project-specific traffic impact thresholds.

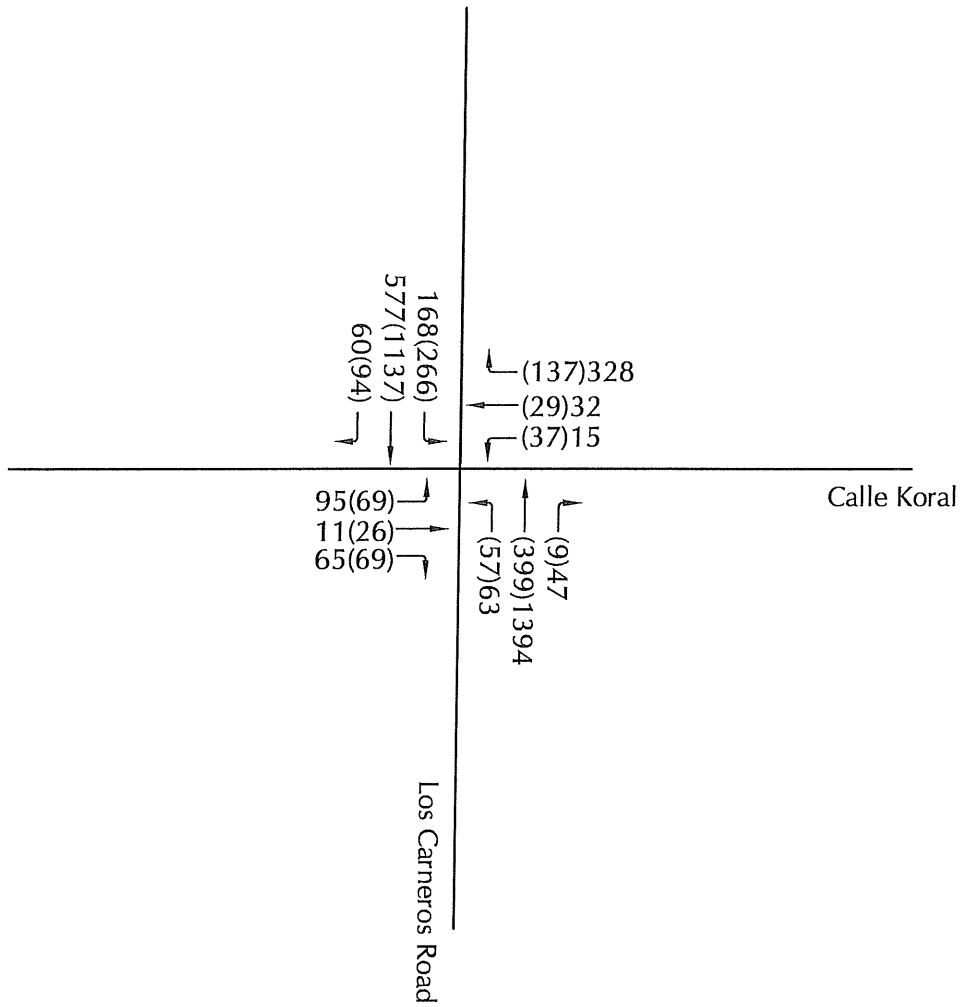
Village at Los Carneros Project Improvements

The Village at Los Carneros Project is programmed to build the western leg of the Los Carneros Road/Calle Koral intersection in order to provide access to the project site. Levels of service were calculated for the Los Carneros Road/Calle Koral intersection assuming completion of the western leg to determine the effects of project traffic on the intersection. Figure 7 presents the Existing+Project Traffic Volumes for the Los Carneros Road/Calle Koral intersection. Table 8 presents the Existing+Project levels of service for the Los Carneros Road/Calle Koral intersection assuming construction of the western leg.

**Table 8
Existing + Project Levels of Service
Los Carneros Road/Calle Koral Intersection w/ Western Leg**

Intersection	Existing		Existing + Project		Project-Added		Project Impact?
	V/C	LOS	V/C	LOS	Trips	V/C	
Los Carneros Road/Calle Koral – A.M.	0.59	A	0.61	B	125	0.015	No
Los Carneros Road/Calle Koral – P.M	0.57	A	0.62	B	131	0.052	No

The data presented in Table 8 indicate that the Los Carneros Road/Calle Koral Intersection with the western leg is forecast to operate at LOS B during the peak hour periods with Existing+Project traffic volumes. The project would therefore not generate significant impacts to this location.



Calle Koral

Los Carneros Road



NOT TO SCALE

LEGEND

└(XX)XX - (A.M.)P.M. Peak Hour Volume



ASSOCIATED
TRANSPORTATION
ENGINEERS

EXISTING + PROJECT TRAFFIC VOLUMES
LOS CARNEROS ROAD/CALLE KORAL WITH WESTERN LEG

FIGURE 7

MMF - #12098.01

CUMULATIVE ANALYSIS

Cumulative Traffic Volumes

Cumulative traffic volumes were forecast using the City's traffic model. The cumulative forecasts include traffic generated by the approved and pending projects proposed within the City of Goleta (the most current list of approved and pending projects from September 2015 is contained in the Technical Appendix for reference) as well as development of the UCSB Long Range Development Plan (LRDP), the Santa Barbara Airport Specific Plan and terminal expansion, and regional growth in the Goleta-Santa Barbara area. Cumulative ADT growth was developed by applying a 10% factor to the P.M. peak hour cumulative traffic additions to the study-area intersections. It is noted that the traffic model has been updated since the time this report was first submitted. The current traffic model does not include the Goleta Target Project which was assumed under the initial model run. Cumulative traffic volumes are shown on Figure 8.

Planned Improvements

The planned improvements that are assumed in the City's traffic model that would affect traffic patterns within the study area are outlined below.

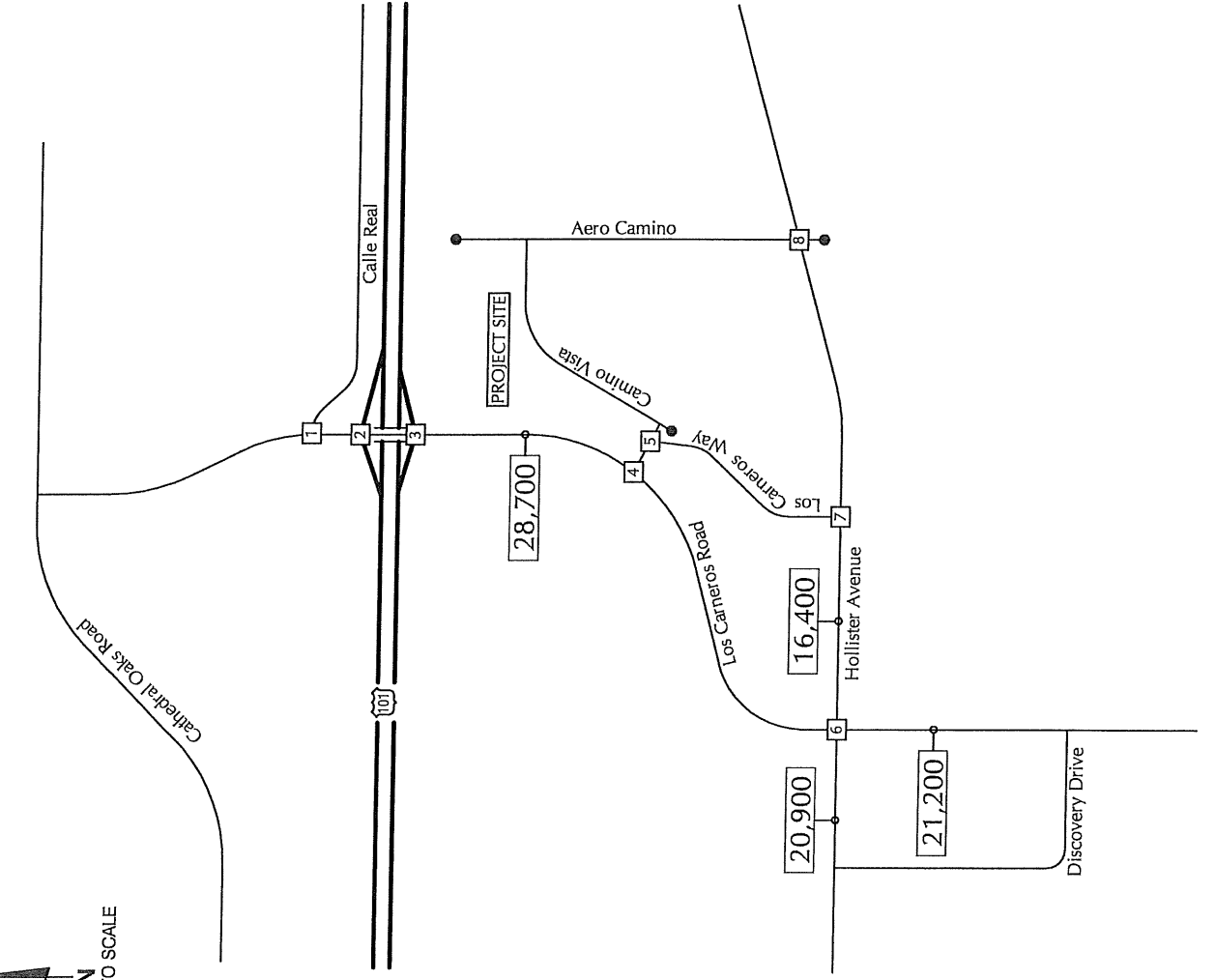
- Construction of the western leg at the Los Carneros Road/Calle Koral intersection to provide access to the Village at Los Carneros Project located west of the intersection.

Cumulative + Project Roadway Operations

Cumulative + Project ADT volumes are shown on Figure 9. Table 9 compares the Cumulative and Cumulative + Project roadway volumes and identifies the impact of project-added traffic based on the City of Goleta's Acceptable Capacity thresholds.



NOT TO SCALE



LEGEND
 (XXX)XX - (A.M.) P.M. Peak Hour Volume
 X - Average Daily Traffic Volume

13(25) 140(245)	(14)27 (274)424	(221)439 (145)260 (6)5
--------------------	--------------------	------------------------------

436(412) 128(107)	(53)33 (2)11 (1233)493	(313)667 (34)236
----------------------	------------------------------	---------------------

48(83) 881(1562)	(475)1157 (213)790	
---------------------	-----------------------	--

190(279) 706(1434) 60(94)	(212)363 (29)32 (39)7	(7)34 (407)1489 (57)63
---------------------------------	-----------------------------	------------------------------

61(40) 518(429) 193(361)	(41)34 (307)664 (76)132	(129)142 (361)707 (66)273
--------------------------------	-------------------------------	---------------------------------

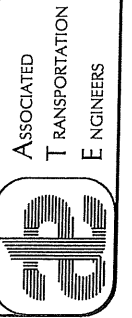
163(103) 72(209)	(95)37 (4)17 (63)73 (185)365
---------------------	---------------------------------------

35(174) 45(44)	(11)332 (357)760
-------------------	---------------------

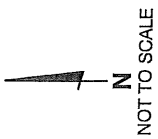
81(46) 1(1) 144(38)	(96)44 (409)883 (3)5
---------------------------	----------------------------

CUMULATIVE TRAFFIC VOLUMES

FIGURE 8



ASSOCIATED
 T RANSPORTATION
 E NGINEERS



LEGEND
 (XXX)X - (A.M.) P.M. Peak Hour Volume
 X - Average Daily Traffic Volume

1	13(25) 146(247)	(14)27 (276)430	(228)442 (152)263 (6)5
2	448(416) 128(107)	(53)33 (2)11 (1246)542	(327)673 (41)239
3	48(83) 942(1579)		(531)1181 (254)799
4	113(134) 1(2) 81(247)		
5	257(298) 706(1434) 60(94)	(289)396 (29)32 (63)17	(12)55 (407)1489 (57)63
6	95(69) 11(26) 65(69)		(41)34 (321)670 (87)137
7	61(40) 522(439) 199(375)		(132)151 (363)716 (66)273
8	336(60) 518(405) 117(238)		
9	87(60) 1(1) 144(38)	(196)80 (66)18	(70)95 (185)365
10	31(81) 589(505) 13(16)		(11)332 (357)760
11	142(179) 575(423)		
12			(99)57 (409)883 (3)5
13			(3)0 (3)33

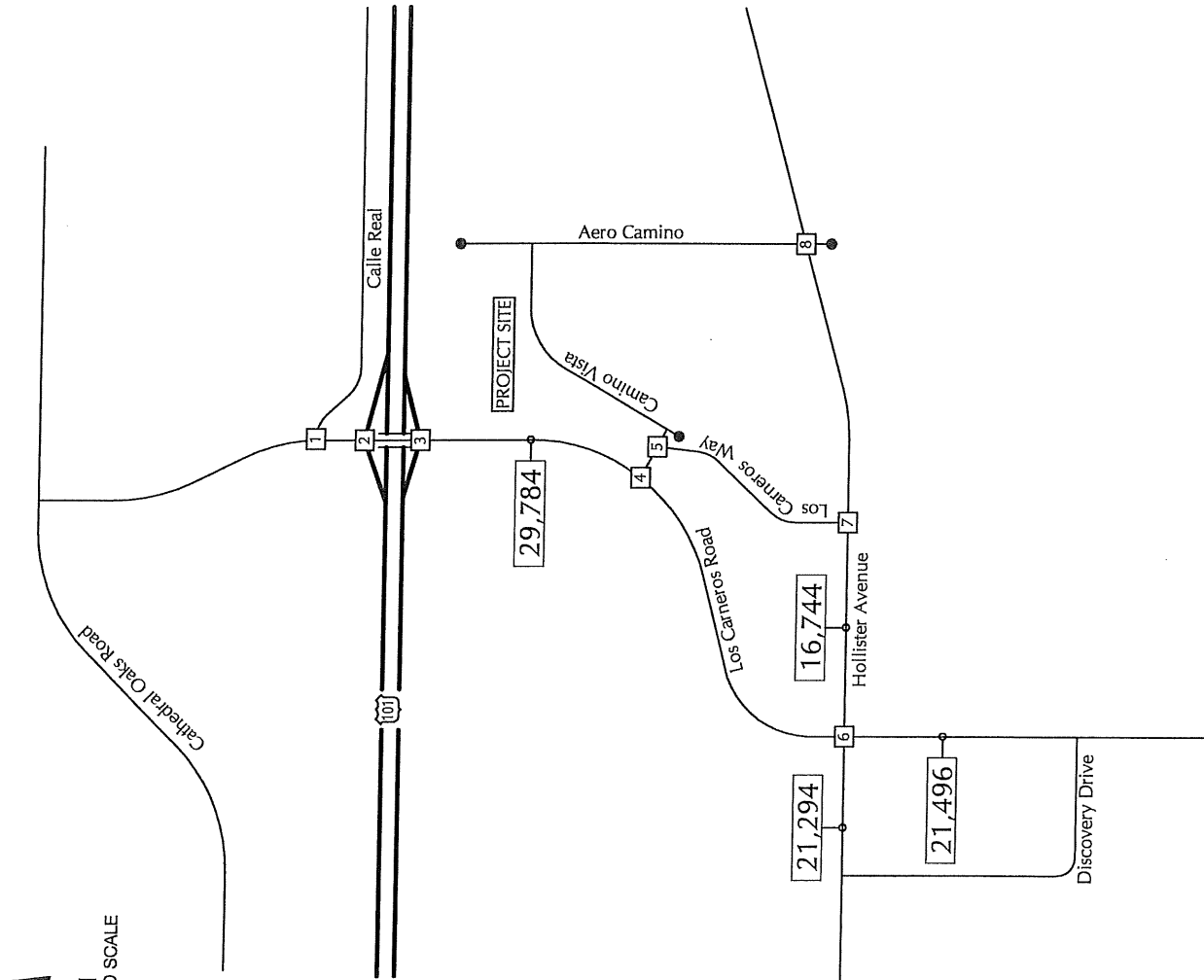
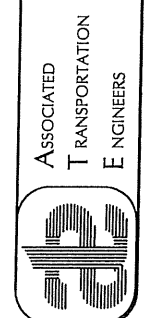


FIGURE 9

CUMULATIVE + PROJECT TRAFFIC VOLUMES



ASSOCIATED
 TRANSPORTATION
 ENGINEERS

Table 9
Cumulative + Project Roadway Operations

Roadway Segment	Average Daily Trips			% Change	Project Impact?	
	Acceptable Capacity	Cumulative ADT	Project Added ADT			Cumulative + Project ADT
Los Carneros Road s/o U.S. 101 SB Ramps	47,000	28,700	1,084	29,784	3.8%	No
Los Carneros Road s/o Hollister Avenue	34,000	21,200	296	23,496	1.4%	No
Hollister Avenue w/o Los Carneros Road	34,000	20,900	394	21,294	1.9%	No
Hollister Avenue e/o Los Carneros Road	34,000	16,400	344	16,744	2.1%	No

The data presented in Table 9 show that the Cumulative + Project roadway volumes would remain within the City’s Acceptable Capacity ratings with the addition of project traffic. The project would therefore not generate cumulative impacts to the study-area roadway segments.

Cumulative + Project Intersection Operations

Cumulative and Cumulative + Project levels of service were calculated for the study-area intersections assuming the traffic volumes presented on Figures 8 and 9. Tables 10 and 11 compare the Cumulative and Cumulative + Project levels of service and identify cumulative impacts based on City of Goleta thresholds.

Table 10
Cumulative + Project Intersection Operations – A.M. Peak Hour

Intersection	Cumulative		Cumulative + Project		Project-Added		Impact?
	V/C	LOS	V/C	LOS	Trips	V/C	
Los Carneros Road/Calle Real (a)	7.3 sec.	A	7.5 sec.	A	18	0.03	No
Los Carneros Rd/U.S. 101 NB Ramps	0.68	B	0.69	B	38	0.011	No
Los Carneros Rd/U.S. 101 SB Ramps	0.67	B	0.68	B	96	0.005	No
Los Carneros Road/Calle Koral	0.70	B	0.72	C	125	0.015	No
Calle Koral/Los Carneros Way (a)	9.5 sec.	A	10.2 sec.	B	157	0.039	No
Hollister Avenue/Los Carneros Road	0.56	A	0.58	A	61	0.016	No
Hollister Avenue/Los Carneros Way	0.41	A	0.42	A	32	0.007	No
Hollister Avenue/Aero Camino	0.34	A	0.35	A	17	0.010	No

(a) Unsignalized intersection.

Table 11
Cumulative + Project Intersection Operations – P.M. Peak Hour

Intersection	Cumulative		Cumulative + Project		Project-Added		Impact?
	V/C	LOS	V/C	LOS	Trips	V/C	
Los Carneros Road/Calle Real (a)	11.8 sec.	B	12.1 sec.	B	18	0.02	No
Los Carneros Rd/U.S. 101 NB Ramps	0.57	A	0.59	A	70	0.020	No
Los Carneros Rd/U.S. 101 SB Ramps	0.84	D	0.85	D	100	0.013	No
Los Carneros Road/Calle Koral	0.66	B	0.71	C	131	0.052	No
Calle Koral/Camino Vista (a)	12.1 sec.	B	14.8 sec.	B	164	0.072	No
Hollister Avenue/Los Carneros Road	0.62	B	0.63	B	64	0.010	No
Hollister Avenue/Los Carneros Way	0.52	A	0.54	A	33	0.015	No
Hollister Avenue/Aero Camino	0.47	A	0.48	A	19	0.007	No

Bolded values exceed City's LOS C operating standard.

(a) Unsignalized intersection.

The data presented in Tables 10 and 11 indicate that the majority of the study-area intersections are forecast to operate at LOS C or better with Cumulative+Project traffic volumes, with the exception of the Los Carneros Road/U.S. 101 SB Ramps intersection

which is forecast to operate at LOS D during the P.M. peak hour with Cumulative and Cumulative+Project traffic. The project would not generate significant cumulative impacts to this locations as the project's traffic additions (V/C increase of 0.013) would not exceed the City's LOS D impact threshold (V/C increase of 0.03).

Cumulative + Project + Improvements Intersection Operations

As shown in Table 11, the U.S. 101 SB Ramps/Los Carneros Road intersection is forecast to operate below the City's LOS C operating standard with Cumulative+Project traffic volumes. The Goleta Transportation Improvement Plan (GTIP), which is responsible for funding future improvement projects in the City, has a programmed improvement to install a free right-turn lane on the northbound approach of the U.S. 101 SB Ramps/Los Carneros Road intersection. Table 12 presents the P.M. peak hour LOS for the U.S. 101 SB Ramps/Los Carneros Road with the GTIP improvement.

**Table 12
Cumulative + Project + Improvements Intersection Operations - P.M. Peak Hour**

Intersection	Cumulative		Cumulative+ Project		Project -Added Trips	Change in V/C	Impact?
	ICU	LOS	ICU	LOS			
Los Carneros Rd/U.S. 101 SB Ramps	0.45	A	0.45	A	100	0.003	No

The data presented in Table 12 show that the U.S. 101 SB Ramps/Los Carneros Road would operate acceptably at LOS A with the identified improvements.

PARKING ANALYSIS

Parking Supply

The project is proposing to provide 152 parking spaces (112 covered carport spaces and 40 uncovered surface spaces) for the senior apartments, 345 spaces for the apartment units (180 covered carport spaces and 165 uncovered surface spaces), and 13 uncovered parking spaces for the park (all public park parking spaces will be signed). The parking supplies for the individual components of the project would not be shared, thus separate parking analyses were completed for each component of the project.

City of Goleta Zoning Ordinance Parking Requirements

Table 13 presents the City of Goleta’s Zoning Ordinance parking requirements for each project component.

Table 13
City of Goleta Zoning Ordinance Parking Requirements

Land-Use	Size	Parking Rate	Zoning Ordinance	Parking Provided
<u>Senior Apartments</u>				
1 Bedroom Apartments	108 Units	1 Space/Unit	108 Spaces	
2 Bedroom Apartments	24 Units	2 Spaces/Unit	48 Spaces	
Visitor Parking	132 Units	1 Space/5 Units	27 Spaces	
Sub-Total:			183 Spaces	152 Spaces
<u>Apartments</u>				
1 Bedroom Apartments	149 Units	1 Space/Unit	149 Spaces	
2 Bedroom Apartments	55 Units	2 Spaces/Unit	110 Spaces	
3 Bedroom Apartments	24 Units	2.5 Spaces/Unit	60 Spaces	
Visitor Parking	228 Units	1 Space/5 Units	46 Spaces	
Sub-Total:			365 Spaces	345 Spaces
Public Park	2 Acres	N/A	N/A	13 Spaces

The data presented in Table 13 show that the City’s parking requirement for the senior apartments is 183 spaces. The proposed parking supply of 152 spaces would not meet the City’s Zoning Ordinance parking requirement for this component of the project. The City’s parking requirement for the apartment units is 365 spaces. The proposed parking supply of 345 spaces would not meet the City’s Zoning Ordinance parking requirement for this component of the project. The project is therefore requesting a parking modification to the Zoning Ordinance requirements based on the findings of the parking demand analysis presented below. The Zoning Ordinance does not contain parking requirements for parks, therefore a parking demand analysis was also completed for this project component.

Parking Demand Analysis

The actual parking demand for any given land use may be different than the Zoning Ordinance parking requirements. In order to evaluate the adequacy of the proposed parking supply, ATE completed a parking demand analysis for each component of the project, as reviewed below.

Parking Demand Rates. Parking demand rates for the senior apartments were developed using parking data collected at three similar senior apartment complexes owned and operated by the Towbes Group in Santa Barbara, Carpinteria, and Ventura. Parking demand

rates for the apartment units were developed using parking data collected at five similar apartment complexes owned and operated by the Towbes Group in the Goleta area (including Willow Springs I which is located adjacent to the proposed project site). The parking survey data collected at the various study sites is contained in the Technical Appendix. Parking demand rates for the proposed park were obtained from the ITE Parking Generation report.³

The parking demand rates used for the analysis were developed by correlating the peak demands observed at the survey sites to the number of bedrooms as opposed to the number of units. This approach was taken to account for the fact that the Heritage Ridge Project contains a larger percentage of one-bedroom units than many of the other apartment sites studied (65% of the apartments and 82% of the senior apartments would be one bedroom units). The parking demands associated with 1-bedroom units are typically higher per bedroom than the demands associated with 2 and 3-bedroom units.

The survey data was plotted to show the relationship between the percentage of 1 bedroom units at each of the study sites and the corresponding peak parking demand rate per bedroom (see Technical Appendix for data plots). This approach provides a statistically accurate method of developing parking forecasts based on the number of bedrooms provided in the development, and follows the procedures used in the City of Goleta's Zoning Ordinance which also determines parking requirements based on the number of bedrooms.

Senior Apartments. Table 14 presents the peak parking demands for the senior housing units based on the average and equation rates developed from the parking survey data.

Table 14
Senior Housing Parking Demands

Land-Use	Size	Peak Demand Rate	Peak Parking Demand	Parking Provided	Parking Surplus
Senior Apartments	156 Bedrooms	0.70 spaces/Bedroom (Average)	109 Spaces	152 Spaces	43 spaces
	156 Bedrooms	0.75 spaces/Bedroom (Equation)	117 Spaces	152 Spaces	35 spaces

The data presented in Table 14 show that the peak parking demand forecast for the senior apartments range from 109 to 117 spaces. The proposed parking supply of 152 spaces would adequately accommodate the peak parking demands associated with the senior apartments and provide a reserve supply of 35 to 43 spaces.

³ Parking Generation, Institute of Transportation Engineers, 4th Edition, 2010.

Apartments. Table 15 presents the peak parking demands for the apartments based on the average and equation rates developed from the parking survey data.

**Table 15
Apartment Parking Demands**

Land-Use	Size	Peak Demand Rate	Peak Parking Demand	Parking Provided	Parking Surplus
Apartments	331 Bedrooms	0.89 spaces/Bedroom (Average)	295 Spaces	345 Spaces	50 Spaces
	331 Bedrooms	0.98 spaces/Bedroom (Equation)	324 Spaces	345 Spaces	21 Spaces

Table 15 shows that the peak parking demand forecast for the apartment units range from 295 to 324 spaces. The proposed parking supply of 345 spaces would adequately accommodate the peak parking demands associated with the apartment units and provide a reserve supply of 21 to 50 spaces.

Public Park. The ITE Parking Generation report contains data for City Parks (ITE Land-Use #411) that show peak parking demand rates ranging from 5.10 vehicles per acre for active parks and 2.30 vehicles per acre for passive parks. Table 16 shows the parking demands for the park based on this data.

**Table 16
Public Park Parking Demands**

Land-Use	Size	Peak Demand Rate	Peak Parking Demand	Parking Provided	Parking Surplus
Public Park	2 Acres	5.10/Acre (Active)	10 Spaces	13 Spaces	3 Spaces
	2 Acres	2.30/Acre (Passive)	5 Spaces	13 Spaces	8 Spaces

Table 16 shows that the peak parking demands for the public park range from 5 to 10 spaces. The proposed parking supply of 13 spaces would adequately accommodate the peak parking demands associated with the park and would provide a reserve supply of 3 to 8 spaces.

SITE ACCESS AND CIRCULATION

Access to the project site would be provided via three driveway connections to Camino Vista. The project site plan (Figure 2) shows that the western most driveway that serves the senior apartments would be aligned opposite the driveway that serves the existing Willow Springs I site and that the eastern most driveway that serves the work-force apartments would

be aligned opposite the driveway that serves the Willow Springs II site that was recently constructed. The middle driveway connection would provide access to the public park as well as the work-force apartments. The proposed alignment of the two main driveways opposite the Willow Springs I and II driveways would allow for efficient ingress/egress for vehicles entering and leaving the site. All three project driveways would stop controlled with STOP signs installed at the driveway approaches and Camino Vista would be uncontrolled. The driveway spacing (approximately 350'-500' between driveways) would allow for vehicle ingress and egress into and out from the project site without interfering with through traffic on Camino Vista. It is recommended that red curb be installed on Camino Vista adjacent to the site driveways to provide sufficient sight distance for drivers exiting the site.

Figure 10 shows the Existing + Project traffic volumes at the three driveways. Given the level of traffic on Camino Vista (642 ADT), the project driveways would operate acceptably in the LOS A-B range with stop-sign control on the outbound driveway approaches (LOS calculation worksheets contained in the Technical Appendix for reference). Left-turn lanes at the project driveways would not be warranted due to the relatively low volumes of through traffic on Camino Vista.

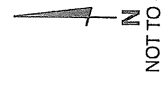
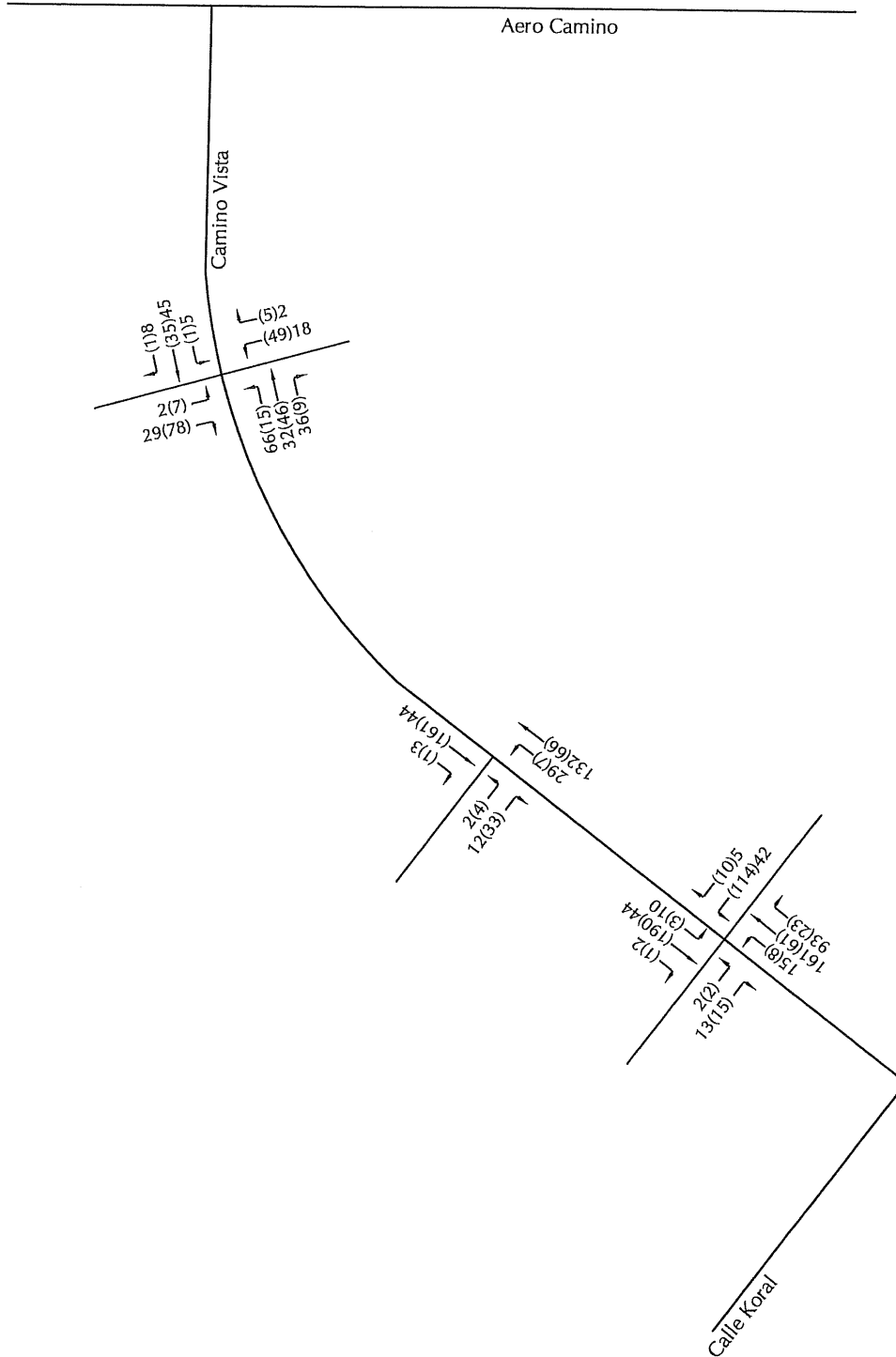
Internal circulation through the site would be provided via 28-foot wide private roadways that would circulate through the senior housing and work-force housing parcels. ATE reviewed the proposed internal circulation plan using AutoTURN software. The proposed design would adequately accommodate the turning maneuvers of large passenger vehicles. It is noted that a vehicle connection between the two parcels would not be provided due to existing archaeological sensitive areas and the Camino Vista street grades.

ALTERNATIVE MODES OF TRANSPORTATION

Pedestrian Facilities

The project is proposing to construct pedestrian sidewalks along the north side of Camino Vista adjacent to the project site. The proposed sidewalks would connect to the existing pedestrian facilities currently provided in the Willow Springs area. Existing pedestrian crosswalks are provided at the proposed senior apartment's driveway and new crosswalks would be installed at the work-force housing driveway. The Project would install a network of pedestrian pathways that would connect the work-force apartment buildings to the public park area.

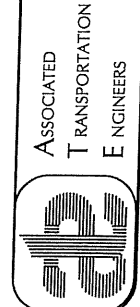
The project as designed consists of two product types. The southwestern portion of the site is Senior Housing and the remainder of the site is Workforce Housing. Due to archaeological sensitive areas and the existing Camino Vista street grades, the Workforce Housing portion of the project is approximately 10' to 12' higher in elevation than the adjacent Senior Housing neighborhood. This elevation difference, in addition to the



LEGEND
 (XX)XX - (A.M.)P.M. Peak Hour Volume

FIGURE 10

EXISTING + PROJECT DRIVEWAY VOLUMES



ASSOCIATED
 TRANSPORTATION
 ENGINEERS

MMF - #12089.01

difference in neighborhood types, preclude an internal connection between the two neighborhoods.

With respect to direct access from the project to Los Carneros Road or Calle Koral, the Tract Map denotes a waiver of abutter's access rights along both public streets. In addition, the grade differential between the finished grades of the portion of the project site adjacent to Los Carneros Road and Calle Koral ranges from 7' to 25', which makes pedestrian/bicycle/vehicle access impractical.

As to an easement dedication adjacent to the railroad property for pedestrian/bicycle access, such an access route would terminate before reaching Los Carneros Road due to the location of the Los Carneros Road embankment relative to the Project property line. Thus, this pedestrian/bicycle path would not serve any properties further to the west. In addition, pedestrian/bicycle access from the northeast corner of the project to the Goleta Train Station is impeded by lack of adjacent property rights and the Los Carneros Creek Channel, which would require a bridge crossing.

The pedestrian system provided in the project study-area would accommodate the new pedestrian traffic generated by the Project. The project-added traffic that would cross the sidewalk on Camino Vista (174 -183 peak hour trips) would not significantly impact pedestrian safety or operations of the surrounding pedestrian facilities.

Bicycle Facilities

Class II bicycle lanes are currently provided along both sides of Camino Vista adjacent to the project site. The Camino Vista bicycle lanes connect to the existing Class II bicycle lanes provided on Calle Koral, Los Carneros Road, and Hollister Avenue. Census data collected in 2010 show that 6% of commuters in the Goleta travel to work on bicycles (census data contained in the Technical Appendix for reference). For a project the size of Heritage Ridge (228 apartment units; senior units do not generate significant amount of commuter traffic) there would be approximately 14 new bicycle riders that would commute during the peak hour periods. The Project would encourage bicycle riding among site residents by providing a bicycle parking area at each residential building and the recreational building with a total of approximately 77 bicycle parking spaces. An additional 8 bicycle parking spaces are provided at the park.

The increase in bicycle ridership generated by the project would not significantly impact the operations of the bicycle facilities in the vicinity of the project site. The project-added traffic that would cross the Class III bicycle lane on Camino Vista (174 -183 peak hour trips) would not significantly impact bicyclist safety or the operations of the surrounding bicycle facilities.

Transit

The Santa Barbara Metropolitan Transit District (MTD) provides local bus service for the region. The nearest bus stops to the project site are located on Hollister Avenue at the Aero Camino intersection (approximately 0.3 miles south of the project site). The existing bus stops are served by MTD Lines 6 and 12x, which provide transit service to/from downtown Santa Barbara to the Old Town Goleta and Camino Real Marketplace areas. Data published on the MTD website indicate that in November 2015, Line 6 carried an average of 33.9 passengers per operating hour, which is slightly below the system wide average of 35.7 passengers per operating hour, and Line 12x carried an average of 36.3 passengers per operating hour, which is slightly higher than the system wide average. The data also shows that both routes experienced 4-10 "at capacity" loads and 3-4 "too full to board" loads during the month of November 2015 (MTD data contained in the Technical Appendix for reference).

Census data collected in 2010 show that 5% of commuters in the Goleta area utilize public transportation (census data contained in the Technical Appendix for reference). For a project the size of Heritage Ridge (228 apartment units; senior units do not generate significant amount of commuter traffic), there would be approximately 11 new transit users that would commute during the peak hour periods (7-9 AM/4-6 PM). There are currently 22 busses that serve the site during the peak hour periods, thus the Project would add less than 1 rider per bus. The new bus riders generated by the project would therefore not measurably impact the operations of the transit routes that serve the site.

CONGESTION MANAGEMENT PROGRAM ANALYSIS

Impact Criteria

The Santa Barbara County Association of Governments (SBCAG) has developed a set of traffic impact thresholds to assess the impacts of land use decisions made by local jurisdictions on regional transportation facilities located within the Congestion Management Program (CMP) roadway system. The following guidelines were developed by SBCAG to determine the significance of project-generated traffic impacts on the regional CMP system.

1. For any roadway or intersection operating at "Level of Service" (LOS) A or B, a decrease of two levels of service resulting from the addition of project-generated traffic.
2. For any roadway or intersection operating at LOS C, project-added traffic that results in LOS D or worse.

3. For intersections within the CMP system with existing congestion, the following table defines significant impacts.

Level of Service	Project-Added Peak Hour Trips
LOS D	20
LOS E	10
LOS F	10

4. For freeway or highway segments with existing congestion, the following table defines significant impacts.

Level of Service	Project-Added Peak Hour Trips
LOS D	100
LOS E	50
LOS F	50

Potential Intersection Impacts

The Los Carneros Road/U.S. 101 NB Ramps, Los Carneros Road/U.S. 101 SB Ramps, and Los Carneros Road/Hollister Avenue intersections are located within the CMP network. As shown on Tables 6 and 7, the CMP intersections are forecast to operate at LOS C or better with Existing+Project traffic volumes. The project would not generate a significant project impact to the CMP network based on CMP impact criteria.

Potential Freeway Impacts

The proposed project is forecast to add 9 P.M. peak hour trips to U.S. 101 north of Los Carneros Road and 73 P.M. peak hour trips to U.S. 101 south of Los Carneros Road. The CMP threshold for freeway impacts is 50 trips for segments operating at LOS E or LOS F and 100 trips for segments operating at LOS D. Data provided by SBCAG indicates that the segment of U.S. 101 between Los Carneros Road and Fairview Avenue currently operates at LOS C⁴ (SBCAG Freeway LOS data contained in the Technical Appendix for reference). Based on these CMP impact criteria, the project would not generate a significant impact to the freeway segment located between Los Carneros Road and Fairview Avenue.



⁴ Congestion Management Program Biennial Review, SBCAG, May 15, 2014.

REFERENCES AND PERSONS CONTACTED

Associated Transportation Engineers

Scott A. Schell, AICP, PTP Principal Transportation Planner

Dan Dawson, PTP Supervising Transportation Planner

Matthew Farrington, Transportation Planner I

References

Highway Capacity Manual, Transportation Research Board, National Research Council, 2010.

Trip Generation, Institute of Transportation Engineers, 9th Edition, 2012.

Parking Generation Institute of Transportation Engineers, 4th Edition, 2010.

Congestion Management Program Biennial Review, SBCAG, May 15, 2014.

Persons Contacted

Damkowitch, Jim, Kittelson Associates

Milan, Marti, City of Goleta

TECHNICAL APPENDIX

CONTENTS:

LEVEL OF SERVICE DEFINITIONS

TRAFFIC COUNT DATA

INTERSECTION LEVEL OF SERVICE CALCULATION WORKSHEETS

- Reference 1 Los Carneros Road/Calle Real
- Reference 2 U.S. 101 NB Ramps/Los Carneros Road
- Reference 3 U.S. 101 SB Ramps/Los Carneros Road
- Reference 4 Los Carneros Road/Calle Koral
- Reference 5 Calle Koral/Los Carneros Way - Calle Koral/Camino Vista
- Reference 6 Hollister Avenue/Los Carneros Road
- Reference 7 Hollister Avenue/Los Carneros Way
- Reference 8 Hollister Avenue/Aero Camino

WILLOWSPRINGS DRIVEWAY COUNT DATA

UNSIGNALIZED INTERSECTION V/C INCREASE CALCULATIONS

CITY OF GOLETA TRAFFIC MODEL DATA/CUMULATIVE PROJECT LIST

PARKING SURVEY DATA AND DEMAND RATE CALCULATION

DRIVEWAY LEVEL OF SERVICE CALCULATION WORKSHEETS

2010 CENSUS MODE OF TRANSPORTATION TO WORK DATA

MTD BUS RIDERSHIP DATA

SBCAG FREEWAY LOS DATA

LEVEL OF SERVICE DEFINITIONS

Table 5. Roadway Classification & Level of Service Thresholds*

City of Goleta Functional Street Classification	City of Goleta Purpose and Design Factors	City of Goleta ADT Design Capacity			City of Goleta LOS C ADT Threshold		
		2 Lanes	4 Lanes	4+ Lanes ¹	2 Lanes	4 Lanes	4+ Lanes ¹
Major Arterial (MA)	Continuous roadways that carry through traffic between various neighborhoods and communities, frequently providing access to major traffic generators such as shopping areas, employment centers, and higher density residential areas. Roadways would have a minimum of 12 foot wide lanes with shoulders. Signals are typically spaced at a minimum 0.5-mile intervals.	17,900	42,480	58,750	14,300	34,000	47,000
Minor Arterial (MNA)	Roadways that serve as a secondary type of arterial facility carrying local and through traffic within communities, frequently connecting neighborhood areas within the City, providing access to shopping areas, employment centers, and higher density residential areas. Roadways would have a minimum of 12-foot wide lanes with shoulders. Signal intervals typically range from 0.25 to 0.5 mile.	15,700	37,680	NA	12,500	30,100	NA
Collector Streets (Col)	Roadways designed to collect traffic from local streets and connect to major or minor arterials. Collector Streets provide access to local streets within residential and commercial areas and connect streets of higher classifications to permit adequate traffic circulation. Generally no more than 2 travel lanes and signalized at intersections with arterial roadways.	11,600	NA	NA	9,280	NA	NA
Local Streets (L)	Roadways designed to provide access to individual properties carrying traffic to and from a collector street. Intended to serve adjacent uses and are not intended for through traffic. Designed with two lanes and close to moderately close driveways.	9,100	NA	NA	7,280	NA	NA
County Functional Street Classification	County Purpose and Design Factors	County ADT Design Capacity			County LOS C ADT Threshold		
		2 Lanes	4 Lanes	4+ Lanes ¹	2 Lanes	4 Lanes	4+ Lanes ¹
Primary 1 (P-1)	Roadways designed to serve primarily non-residential development. Roadways would have a minimum of 12-foot wide lanes with shoulders and few curb cuts. Signals would be spaced at 1 mile or more intervals.	19,900	47,760	NA	15,900	38,200	NA
Primary 2 (P-2)	Roadways designed to serve a high proportion of non-residential development with some residential lots and few or no driveway curb cuts. Roadways would have a minimum of 12-foot wide lanes with few curb cuts. Signals spacing at minimum of 1/2 mile.	17,900	42,480	NA	14,300	34,000	NA
Primary 3 (P-3)	Roadways designed to serve non-residential development and residential development. More frequent driveways are acceptable. Potential signal spacing of 1/2 to 3/4 mile.	15,700	37,680	NA	12,500	30,100	NA
Secondary 1 (S-1)	Roadways designed to serve non-residential development and large lot residential development with well spaced driveways. Roadways would be 2-lanes with infrequent driveways. Signals would generally occur at intersections of primary roadways.	11,600	NA	NA	9,300	NA	NA
Secondary 2 (S-2)	Roadways designed to serve residential and non-residential land uses. Roadways would be 2-lanes with close to moderately spaced driveways.	9,100	NA	NA	7,300	NA	NA
Secondary 3 (S-3)	Roadways designed to primarily serve residential with small to medium size lots. Roadways would be 2-lanes with more frequent driveways.	7,900	NA	NA	6,300	NA	NA

* Source: City of Goleta & County of Santa Barbara Public Works Department

Signalized Intersection Level of Service Definitions

LOS	Delay ^a	V/C Ratio	Definition
A	< 10.0	< 0.60	Progression is extremely favorable. Most vehicles arrive during the green phase. Many vehicles do not stop at all.
B	10.1 - 20.0	0.61 - 0.70	Good progression, short cycle lengths, or both. More vehicles stop than with LOS A, causing higher levels of delay.
C	20.1 - 35.0	0.71 - 0.80	Only fair progression, longer cycle lengths, or both, result in higher cycle lengths. Cycle lengths may fail to serve queued vehicles, and overflow occurs. Number of vehicles stopped is significant, though many still pass through intersection without stopping.
D	35.1 - 55.0	0.81 - 0.90	Congestion becomes more noticeable. Unfavorable progression, long cycle lengths and high v/c ratios result in longer delays. Many vehicles stop, and the proportion of vehicles not stopping declines. Individual cycle failures are noticeable.
E	55.1 - 80.0	0.91 - 1.00	High delay values indicate poor progression, long cycle lengths and high v/c ratios. Individual cycle failures are frequent
F	> 80.0	> 1.00	Considered unacceptable for most drivers, this level occurs when arrival flow rates exceed the capacity of lane groups, resulting in many individual cycle failures. Poor progression and long cycle lengths may also contribute to high delay levels.

^a Average control delay per vehicle in seconds.

Unsignalized Intersection Level of Service Definitions

The HCM¹ uses *control delay* to determine the level of service at unsignalized intersections. Control delay is the difference between the travel time actually experienced at the control device and the travel time that would occur in the absence of the traffic control device. Control delay includes deceleration from free flow speed, queue move-up time, stopped delay and acceleration back to free flow speed.

LOS	Control Delay Seconds per Vehicle
A	< 10.0
B	10.1 - 15.0
C	15.1 - 25.0
D	25.1 - 35.0
E	35.1 - 50.0
F	> 50.0

¹ Highway Capacity Manual, National Research Board, 2000



TRAFFIC COUNT DATA

VOLUME

Los Carneros Rd S/o US Hwy 101

Day: Thursday
Date: 2/9/2012

City: Goleta
Project #: CA12_8007_001

DAILY TOTALS					NB	SB	EB	WB	Total		
					12,352	10,997	0	0	23,349		
AM Period	NB	SB	EB	WB	TOTAL	PM Period	NB	SB	EB	WB	TOTAL
00:00	38	27			65	12:00	250	148			398
00:15	30	23			53	12:15	267	173			440
00:30	17	34			51	12:30	228	163			391
00:45	19	104	26	110	45 214	12:45	173	918	281	765	454 1683
01:00	27	27			54	13:00	180	201			381
01:15	19	22			41	13:15	198	209			407
01:30	22	28			50	13:30	174	160			334
01:45	22	90	65	142	87 232	13:45	204	756	172	742	376 1498
02:00	38	38			76	14:00	195	147			342
02:15	24	16			40	14:15	181	188			369
02:30	9	10			19	14:30	215	150			365
02:45	12	83	8	72	20 155	14:45	183	774	147	632	330 1406
03:00	14	3			17	15:00	231	153			384
03:15	10	2			12	15:15	259	148			407
03:30	6	8			14	15:30	285	154			439
03:45	13	43	9	22	22 65	15:45	230	1005	174	629	404 1634
04:00	5	7			12	16:00	300	166			466
04:15	7	4			11	16:15	292	165			457
04:30	7	13			20	16:30	412	143			555
04:45	6	25	17	41	23 66	16:45	331	1335	163	637	494 1972
05:00	13	24			37	17:00	505	165			670
05:15	12	16			28	17:15	434	142			576
05:30	34	42			76	17:30	394	182			576
05:45	17	76	80	162	97 238	17:45	290	1623	157	646	447 2269
06:00	27	75			102	18:00	266	151			417
06:15	22	109			131	18:15	240	176			416
06:30	46	114			160	18:30	186	163			349
06:45	62	157	195	493	257 650	18:45	187	879	147	637	334 1516
07:00	45	183			228	19:00	169	112			281
07:15	96	198			294	19:15	141	87			228
07:30	131	197			328	19:30	117	114			231
07:45	119	391	352	930	471 1321	19:45	102	529	96	409	198 938
08:00	147	402			549	20:00	111	118			229
08:15	134	272			406	20:15	90	100			190
08:30	129	263			392	20:30	75	93			168
08:45	142	552	214	1151	356 1703	20:45	111	387	88	399	199 786
09:00	116	166			282	21:00	78	110			188
09:15	130	145			275	21:15	89	100			189
09:30	136	139			275	21:30	70	86			156
09:45	133	515	145	595	278 1110	21:45	47	284	79	375	126 659
10:00	132	125			257	22:00	56	67			123
10:15	131	105			236	22:15	74	78			152
10:30	174	99			273	22:30	82	64			146
10:45	150	587	117	446	267 1033	22:45	59	271	57	266	116 537
11:00	144	112			256	23:00	48	51			99
11:15	172	135			307	23:15	39	47			86
11:30	225	141			366	23:30	56	35			91
11:45	249	790	147	535	396 1325	23:45	35	178	28	161	63 339
TOTALS	3413	4699			8112	TOTALS	8939	6298			15237
SPLIT %	42.1%	57.9%			34.7%	SPLIT %	58.7%	41.3%			65.3%

DAILY TOTALS					NB	SB	EB	WB	Total
					12,352	10,997	0	0	23,349
AM Peak Hour	11:45	07:45			07:45	PM Peak Hour	16:30	12:30	16:45
AM Pk Volume	994	1289			1818	PM Pk Volume	1682	854	2316
Pk Hr Factor	0.931	0.802			0.828	Pk Hr Factor	0.833	0.760	0.864
7 - 9 Volume	943	2081	0	0	3024	4 - 6 Volume	2958	1283	4241
7 - 9 Peak Hour	08:00	07:45			07:45	4 - 6 Peak Hour	16:30	16:45	16:45
7 - 9 Pk Volume	552	1289	0	0	1818	4 - 6 Pk Volume	1682	652	2316
Pk Hr Factor	0.939	0.802	0.000	0.000	0.828	Pk Hr Factor	0.833	0.896	0.864

VOLUME

Los Carneros Rd S/o Hollister Ave

Day: Thursday
Date: 2/9/2012

City: Goleta
Project #: CA12_8007_002

DAILY TOTALS					NB	SB	EB	WB	Total		
					8,952	8,731	0	0	17,683		
AM Period	NB	SB	EB	WB	TOTAL	PM Period	NB	SB	EB	WB	TOTAL
00:00	36	37			73	12:00	165	118			283
00:15	27	30			57	12:15	166	141			307
00:30	19	44			63	12:30	164	124			288
00:45	22	104	31	142	53 246	12:45	129	624	172	555	301 1179
01:00	29	27			56	13:00	144	159			303
01:15	15	28			43	13:15	153	141			294
01:30	25	37			62	13:30	141	117			258
01:45	34	103	67	159	101 262	13:45	172	610	129	546	301 1156
02:00	44	40			84	14:00	177	111			288
02:15	26	24			50	14:15	141	137			278
02:30	13	12			25	14:30	134	149			283
02:45	10	93	9	85	19 178	14:45	134	586	132	529	266 1115
03:00	13	7			20	15:00	152	124			276
03:15	9	3			12	15:15	160	149			309
03:30	4	6			10	15:30	182	135			317
03:45	12	38	6	22	18 60	15:45	149	643	161	569	310 1212
04:00	5	8			13	16:00	199	139			338
04:15	8	2			10	16:15	167	149			316
04:30	7	9			16	16:30	195	131			326
04:45	6	26	9	28	15 54	16:45	185	746	155	574	340 1320
05:00	9	10			19	17:00	277	183			460
05:15	11	6			17	17:15	226	139			365
05:30	26	14			40	17:30	195	177			372
05:45	21	67	19	49	40 116	17:45	153	851	179	678	332 1529
06:00	24	21			45	18:00	181	178			359
06:15	19	30			49	18:15	161	176			337
06:30	29	32			61	18:30	141	185			326
06:45	45	117	71	154	116 271	18:45	142	625	180	719	322 1344
07:00	35	49			84	19:00	131	127			258
07:15	59	90			149	19:15	110	102			212
07:30	108	97			205	19:30	80	130			210
07:45	94	296	131	367	225 663	19:45	89	410	122	481	211 891
08:00	97	141			238	20:00	92	127			219
08:15	110	97			207	20:15	92	123			215
08:30	107	111			218	20:30	86	114			200
08:45	136	450	124	473	260 923	20:45	103	373	102	466	205 839
09:00	122	101			223	21:00	98	124			222
09:15	95	110			205	21:15	105	111			216
09:30	119	112			231	21:30	71	95			166
09:45	126	462	127	450	253 912	21:45	54	328	87	417	141 745
10:00	100	74			174	22:00	55	89			144
10:15	107	79			186	22:15	76	71			147
10:30	150	93			243	22:30	87	60			147
10:45	121	478	108	354	229 832	22:45	54	272	65	285	119 557
11:00	106	88			194	23:00	64	51			115
11:15	98	112			210	23:15	40	48			88
11:30	129	121			250	23:30	44	55			99
11:45	130	463	117	438	247 901	23:45	39	187	37	191	76 378
TOTALS	2697	2721			5418	TOTALS	6255	6010			12265
SPLIT %	49.8%	50.2%			30.6%	SPLIT %	51.0%	49.0%			69.4%

DAILY TOTALS					NB	SB	EB	WB	Total
					8,952	8,731	0	0	17,683
AM Peak Hour	11:45	11:45			11:45	PM Peak Hour	16:30	18:00	16:45
AM Pk Volume	625	500			1125	PM Pk Volume	883	719	1537
Pk Hr Factor	0.941	0.887			0.916	Pk Hr Factor	0.797	0.972	0.835
7 - 9 Volume	746	840	0	0	1586	4 - 6 Volume	1597	1252	2849
7 - 9 Peak Hour	08:00	07:45			08:00	4 - 6 Peak Hour	16:30	17:00	16:45
7 - 9 Pk Volume	450	480	0	0	923	4 - 6 Pk Volume	883	678	1537
Pk Hr Factor	0.827	0.851	0.000	0.000	0.888	Pk Hr Factor	0.797	0.926	0.835

VOLUME

Hollister Ave W/o Los Carneros Rd

Day: Thursday
Date: 2/9/2012

City: Goleta
Project #: CA12_8007_006

DAILY TOTALS					NB	SB	EB	WB	Total					
					0	0	8,633	8,643	17,276					
AM Period	NB	SB	EB	WB	TOTAL	PM Period	NB	SB	EB	WB	TOTAL			
00:00			6	12	18	12:00			187	273	460			
00:15			11	14	25	12:15			201	225	426			
00:30			10	10	20	12:30			191	185	376			
00:45			3	30	11	47	12:45		244	823	188	871	343	1694
01:00			7	4	11	13:00			232	193	425			
01:15			4	4	8	13:15			193	159	352			
01:30			11	6	17	13:30			205	172	377			
01:45			16	38	9	23	13:45		180	810	163	687	343	1497
02:00			2	15	17	14:00			152	125	277			
02:15			5	6	11	14:15			146	141	287			
02:30			3	6	9	14:30			146	128	274			
02:45			4	14	5	32	14:45		141	585	130	524	271	1109
03:00			3	5	8	15:00			144	207	351			
03:15			1	1	2	15:15			166	140	306			
03:30			5	4	9	15:30			145	188	333			
03:45			2	11	19	29	15:45		149	604	146	681	295	1285
04:00			8	6	14	16:00			157	195	352			
04:15			5	6	11	16:15			128	176	304			
04:30			6	7	13	16:30			155	191	346			
04:45			17	36	8	27	16:45		152	592	202	764	354	1356
05:00			5	9	14	17:00			213	263	476			
05:15			14	6	20	17:15			177	277	454			
05:30			23	17	40	17:30			189	243	432			
05:45			42	84	26	58	17:45		159	738	185	968	344	1706
06:00			43	21	64	18:00			159	143	302			
06:15			51	31	82	18:15			111	152	263			
06:30			53	35	88	18:30			103	142	245			
06:45			105	252	48	135	18:45		114	487	114	551	228	1038
07:00			67	55	122	19:00			85	96	181			
07:15			103	75	178	19:15			65	81	146			
07:30			141	68	209	19:30			85	73	158			
07:45			200	511	103	301	19:45		85	320	66	316	151	636
08:00			169	134	303	20:00			70	76	146			
08:15			127	108	235	20:15			53	57	110			
08:30			141	105	246	20:30			57	74	131			
08:45			124	561	87	434	20:45		62	242	52	259	114	501
09:00			131	118	249	21:00			30	61	91			
09:15			104	94	198	21:15			54	54	108			
09:30			121	109	230	21:30			40	46	86			
09:45			115	471	95	416	21:45		31	155	30	191	61	346
10:00			123	113	236	22:00			37	29	66			
10:15			96	107	203	22:15			24	38	62			
10:30			117	105	222	22:30			14	30	44			
10:45			137	473	120	445	22:45		22	97	20	117	42	214
11:00			164	148	312	23:00			13	29	42			
11:15			141	134	275	23:15			18	16	34			
11:30			162	183	345	23:30			20	21	41			
11:45			174	641	219	684	23:45		7	58	17	83	24	141
TOTALS			3122	2631	5753	TOTALS			5511	6012	11523			
SPLIT %			54.3%	45.7%	33.3%	SPLIT %			47.8%	52.2%	66.7%			

DAILY TOTALS					NB	SB	EB	WB	Total		
					0	0	8,633	8,643	17,276		
AM Peak Hour			11:45	11:45	11:45	PM Peak Hour			12:45	16:45	16:45
AM Pk Volume			753	902	1655	PM Pk Volume			874	985	1716
Pk Hr Factor			0.937	0.826	0.899	Pk Hr Factor			0.895	0.889	0.901
7 - 9 Volume	0	0	1072	735	1807	4 - 6 Volume	0	0	1330	1732	3062
7 - 9 Peak Hour			07:30	07:45	07:45	4 - 6 Peak Hour			17:00	16:45	16:45
7 - 9 Pk Volume	0	0	637	450	1087	4 - 6 Pk Volume	0	0	738	985	1716
Pk Hr Factor	0.000	0.000	0.796	0.840	0.897	Pk Hr Factor	0.000	0.000	0.866	0.889	0.901

VOLUME

Hollister Ave E/o Los Carneros Way

Day: Thursday
Date: 2/9/2012

City: Goleta
Project #: CA12_8007_005

DAILY TOTALS					NB	SB	EB	WB	Total			
					0	0	6,967	7,475	14,442			
AM Period	NB	SB	EB	WB	TOTAL	PM Period	NB	SB	EB	WB	TOTAL	
00:00			10	12	22	12:00			165	214	379	
00:15			11	13	24	12:15			150	193	343	
00:30			6	10	16	12:30			167	159	326	
00:45			2	29	10	12:45			217	699	167	733
01:00			6	3	9	13:00			191	171	362	
01:15			3	5	8	13:15			170	146	316	
01:30			6	6	12	13:30			175	160	335	
01:45			12	27	11	13:45			145	681	161	638
02:00			4	13	17	14:00			142	114	256	
02:15			5	6	11	14:15			117	112	229	
02:30			3	7	10	14:30			119	118	237	
02:45			4	16	5	14:45			139	517	125	469
03:00			6	6	12	15:00			135	180	315	
03:15			4	3	7	15:15			136	133	269	
03:30			3	4	7	15:30			128	166	294	
03:45			3	16	12	15:45			140	539	134	613
04:00			8	7	15	16:00			119	160	279	
04:15			4	3	7	16:15			101	150	251	
04:30			6	7	13	16:30			122	173	295	
04:45			13	31	5	16:45			119	461	170	653
05:00			5	9	14	17:00			149	220	369	
05:15			10	6	16	17:15			136	214	350	
05:30			18	14	32	17:30			126	190	316	
05:45			42	75	17	17:45			103	514	153	777
06:00			34	17	51	18:00			122	118	240	
06:15			48	22	70	18:15			94	132	226	
06:30			49	34	83	18:30			71	116	187	
06:45			83	214	31	18:45			88	375	109	475
07:00			55	42	97	19:00			67	76	143	
07:15			74	65	139	19:15			49	70	119	
07:30			86	66	152	19:30			55	71	126	
07:45			143	358	71	19:45			72	243	60	277
08:00			105	97	202	20:00			53	72	125	
08:15			110	91	201	20:15			44	53	97	
08:30			102	82	184	20:30			43	68	111	
08:45			96	413	80	20:45			46	186	43	236
09:00			101	104	205	21:00			31	44	75	
09:15			80	76	156	21:15			47	36	83	
09:30			79	86	165	21:30			39	40	79	
09:45			92	352	99	21:45			30	147	31	151
10:00			112	93	205	22:00			31	38	69	
10:15			82	95	177	22:15			16	28	44	
10:30			99	95	194	22:30			19	25	44	
10:45			126	419	119	22:45			15	81	15	106
11:00			109	113	222	23:00			17	22	39	
11:15			109	122	231	23:15			21	16	37	
11:30			138	171	309	23:30			22	30	52	
11:45			149	505	199	23:45			9	69	15	83
TOTALS			2455	2264	4719	TOTALS			4512	5211	9723	
SPLIT %			52.0%	48.0%	32.7%	SPLIT %			46.4%	53.6%	67.3%	

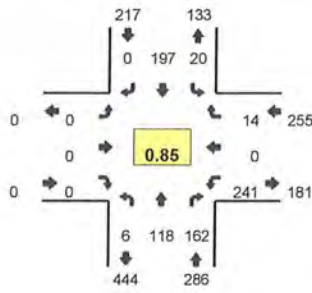
DAILY TOTALS					NB	SB	EB	WB	Total		
					0	0	6,967	7,475	14,442		
AM Peak Hour			11:45	11:30	11:45	PM Peak Hour			12:45	16:45	12:00
AM Pk Volume			631	777	1396	PM Pk Volume			753	794	1432
Pk Hr Factor			0.945	0.908	0.921	Pk Hr Factor			0.868	0.902	0.932
7 - 9 Volume	0	0	771	594	1365	4 - 6 Volume	0	0	975	1430	2405
7 - 9 Peak Hour			07:45	08:00	07:45	4 - 6 Peak Hour			16:45	16:45	16:45
7 - 9 Pk Volume	0	0	460	350	801	4 - 6 Pk Volume	0	0	530	794	1324
Pk Hr Factor	0.000	0.000	0.804	0.902	0.936	Pk Hr Factor	0.000	0.000	0.889	0.902	0.897

Type of peak hour being reported: Intersection Peak

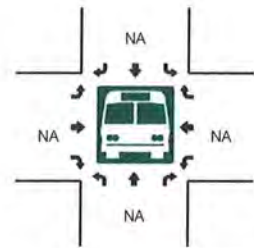
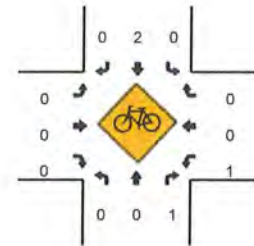
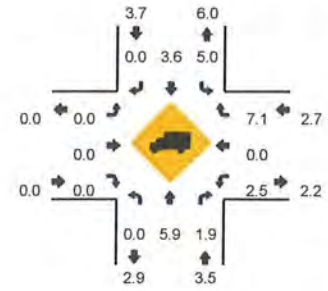
Method for determining peak hour: Total Entering Volume

LOCATION: Los Carneros Rd -- Calle Real
CITY/STATE: Goleta, CA

QC JOB #: 10938823
DATE: Thu, Apr 04 2013



Peak-Hour: 7:50 AM -- 8:50 AM
Peak 15-Min: 7:50 AM -- 8:05 AM



R* = RTOR

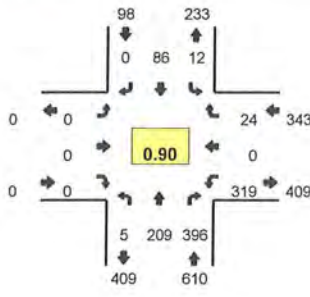
5-Min Count Period	Los Carneros Rd (Northbound)					Los Carneros Rd (Southbound)					Calle Real (Eastbound)					Calle Real (Westbound)					Total	Hourly Totals	
	Beginning At	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U			R*
7:00 AM	0	6	4	0	0	1	7	0	0	0	0	0	0	0	0	0	6	0	0	0	0	24	
7:05 AM	0	5	0	0	0	0	10	0	0	0	0	0	0	0	0	0	12	0	0	0	0	27	
7:10 AM	0	2	5	0	0	0	11	0	0	0	0	0	0	0	0	0	4	0	0	0	0	22	
7:15 AM	0	5	6	0	0	0	9	0	0	0	0	0	0	0	0	0	5	0	0	0	0	25	
7:20 AM	0	3	4	0	0	1	10	0	0	0	0	0	0	0	0	0	17	0	0	0	0	35	
7:25 AM	0	4	5	0	0	2	13	0	0	0	0	0	0	0	0	0	11	0	0	0	0	35	
7:30 AM	0	8	10	0	0	0	17	0	0	0	0	0	0	0	0	0	15	0	1	0	0	51	
7:35 AM	0	3	7	1	0	1	12	0	0	0	0	0	0	0	0	0	9	0	0	0	0	33	
7:40 AM	0	6	6	0	0	0	15	0	0	0	0	0	0	0	0	0	11	0	0	0	0	38	
7:45 AM	0	2	8	1	0	1	21	0	0	0	0	0	0	0	0	0	21	0	1	0	0	55	
7:50 AM	0	11	13	1	0	1	18	0	1	0	0	0	0	0	0	0	29	0	0	0	0	74	
7:55 AM	0	17	16	1	0	0	22	0	0	0	0	0	0	0	0	0	20	0	2	0	0	78	497
8:00 AM	0	10	16	0	0	1	22	0	0	0	0	0	0	0	0	0	21	0	1	0	0	71	544
8:05 AM	0	9	13	1	0	2	21	0	0	0	0	0	0	0	0	0	22	0	0	0	0	68	585
8:10 AM	0	9	11	0	0	0	16	0	0	0	0	0	0	0	0	0	15	0	0	0	0	51	614
8:15 AM	0	14	14	0	0	3	10	0	0	0	0	0	0	0	0	0	30	0	2	0	0	73	662
8:20 AM	0	7	16	1	0	1	20	0	0	0	0	0	0	0	0	0	15	0	1	0	0	61	688
8:25 AM	0	9	8	0	0	0	9	0	0	0	0	0	0	0	0	0	15	0	0	0	0	41	694
8:30 AM	0	7	16	1	0	2	21	0	0	0	0	0	0	0	0	0	13	0	3	0	0	63	706
8:35 AM	0	7	13	0	0	3	14	0	0	0	0	0	0	0	0	0	15	0	1	0	0	53	726
8:40 AM	0	7	11	0	0	3	16	0	0	0	0	0	0	0	0	0	21	0	0	0	0	58	746
8:45 AM	0	11	15	1	0	3	8	0	0	0	0	0	0	0	0	0	25	0	4	0	0	67	758
8:50 AM	0	6	14	0	0	3	24	0	0	0	0	0	0	0	0	0	16	0	2	0	0	65	749
8:55 AM	0	6	23	0	0	0	7	0	0	0	0	0	0	0	0	0	17	0	0	0	0	53	724
Peak 15-Min Flowrates	Northbound					Southbound					Eastbound					Westbound					Total		
All Vehicles	0	152	180	8	0	8	248	0	4	0	0	0	0	0	0	280	0	12	0	0	892		
Heavy Trucks	0	4	8			0	4	0			0	0	0			0	0	0			16		
Pedestrians	0		0			0		0			4					0					4		
Bicycles	0	0	1			0	0	0			0	0	0			0	0	0			1		
Railroad																							
Stopped Buses																							

Comments: N/A

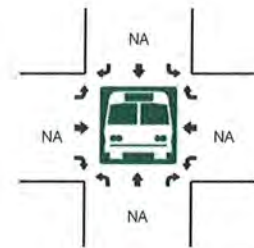
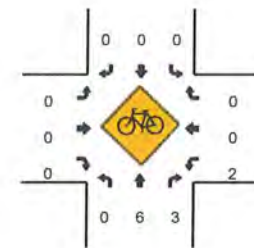
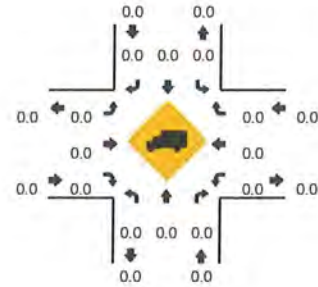
Type of peak hour being reported: Intersection Peak

Method for determining peak hour: Total Entering Volume

LOCATION: Los Carneros Rd -- Calle Real
CITY/STATE: Goleta, CA
QC JOB #: 10938824
DATE: Thu, Apr 04 2013



Peak-Hour: 4:55 PM -- 5:55 PM
Peak 15-Min: 5:10 PM -- 5:25 PM



R* = RTOR

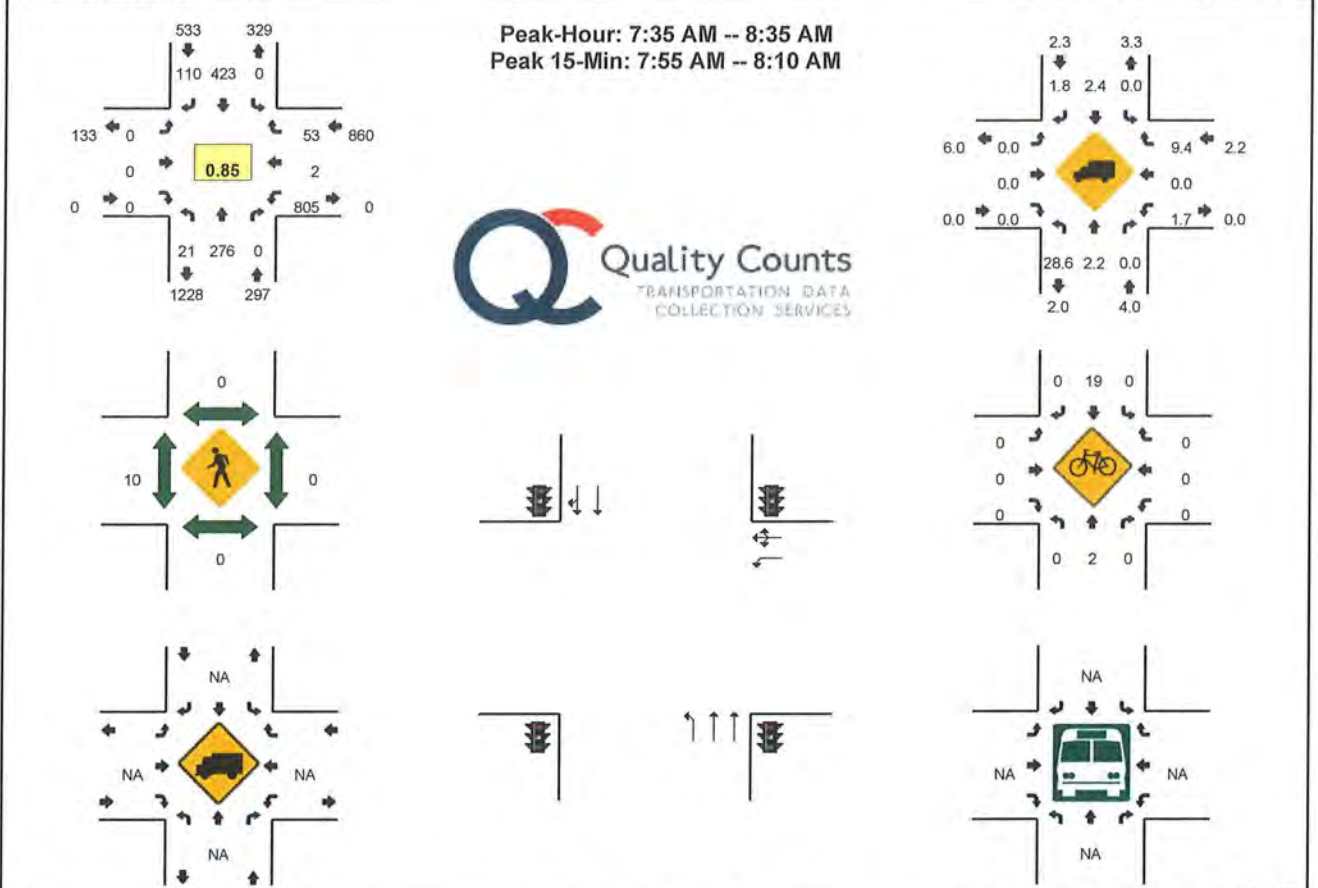
5-Min Count Period Beginning At	Los Carneros Rd (Northbound)					Los Carneros Rd (Southbound)					Calle Real (Eastbound)					Calle Real (Westbound)					Total	Hourly Totals
	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*		
4:00 PM	0	15	14	0	0	0	3	0	0	0	0	0	0	0	0	20	0	2	0	0	54	
4:05 PM	0	11	21	2	0	3	17	0	0	0	0	0	0	0	0	49	0	7	0	0	110	
4:10 PM	0	14	25	1	0	1	8	0	0	0	0	0	0	0	0	30	0	1	0	0	80	
4:15 PM	0	13	21	0	0	1	8	0	0	0	0	0	0	0	0	19	0	1	0	0	63	
4:20 PM	0	12	19	0	0	0	8	0	0	0	0	0	0	0	0	23	0	1	0	0	63	
4:25 PM	0	9	14	0	0	1	10	0	0	0	0	0	0	0	0	20	0	1	0	0	55	
4:30 PM	0	7	33	2	0	0	6	0	0	0	0	0	0	0	0	22	0	2	0	0	72	
4:35 PM	0	13	28	0	0	1	3	0	0	0	0	0	0	0	0	20	0	3	0	0	68	
4:40 PM	0	17	32	2	0	1	2	0	0	0	0	0	0	0	0	18	0	1	0	0	73	
4:45 PM	0	10	25	0	0	2	3	0	0	0	0	0	0	0	0	24	0	1	0	0	65	
4:50 PM	0	10	30	0	0	0	12	0	0	0	0	0	0	0	0	26	0	1	0	0	79	
4:55 PM	0	23	29	0	0	3	9	0	0	0	0	0	0	0	0	15	0	3	0	0	82	864
5:00 PM	0	11	31	1	0	2	7	0	0	0	0	0	0	0	0	29	0	2	0	0	83	893
5:05 PM	0	17	32	1	0	1	9	0	0	0	0	0	0	0	0	18	0	4	0	0	82	865
5:10 PM	0	22	33	0	0	0	9	0	0	0	0	0	0	0	0	30	0	0	0	0	94	879
5:15 PM	0	27	39	0	0	1	15	0	0	0	0	0	0	0	0	28	0	2	0	0	112	928
5:20 PM	0	19	35	0	0	1	2	0	0	0	0	0	0	0	0	26	0	4	0	0	87	952
5:25 PM	0	19	33	0	0	0	3	0	0	0	0	0	0	0	0	32	0	0	0	0	87	984
5:30 PM	0	15	38	1	0	1	5	0	0	0	0	0	0	0	0	22	0	2	1	0	85	997
5:35 PM	0	21	26	1	0	0	4	0	0	0	0	0	0	0	0	33	0	1	0	0	86	1015
5:40 PM	0	13	38	0	0	0	15	0	0	0	0	0	0	0	0	29	0	1	0	0	96	1038
5:45 PM	0	7	32	0	0	1	7	0	0	0	0	0	0	0	0	26	0	2	0	0	75	1048
5:50 PM	0	15	30	1	0	2	1	0	0	0	0	0	0	0	0	30	0	3	0	0	82	1051
5:55 PM	0	9	36	1	0	2	6	0	0	0	0	0	0	0	0	19	0	3	0	0	76	1045
Peak 15-Min Flowrates	Northbound					Southbound					Eastbound					Westbound					Total	
Beginning At	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*		
All Vehicles	0	272	428	0	0	8	104	0	0	0	0	0	0	0	0	336	0	24	0	0	1172	
Heavy Trucks	0	0	0			0	0	0			0	0	0			0	0	0			0	
Pedestrians		0					8					12					0				20	
Bicycles	0	1	1			0	0	0			0	0	0			0	0	0			2	
Railroad																						
Stopped Buses																						

Comments: N/A

Type of peak hour being reported: Intersection Peak

Method for determining peak hour: Total Entering Volume

LOCATION: Los Carneros Rd -- US 101 NB Ramps
 CITY/STATE: Goleta, CA
 QC JOB #: 10938619
 DATE: Wed, May 22 2013



5-Min Count Period Beginning At	Los Carneros Rd (Northbound)					Los Carneros Rd (Southbound)					US 101 NB Ramps (Eastbound)					US 101 NB Ramps (Westbound)					Total	Hourly Totals
	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*		
7:00 AM	0	8	0	0	0	0	14	2	0	0	0	0	0	0	0	48	0	2	0	0	74	
7:05 AM	3	13	0	0	0	0	12	2	0	0	0	0	0	0	0	44	0	1	0	1	76	
7:10 AM	3	5	0	0	0	0	16	4	0	1	0	0	0	0	0	42	0	1	0	1	73	
7:15 AM	3	10	0	0	0	0	26	5	0	0	0	0	0	0	0	38	0	1	0	2	85	
7:20 AM	5	16	0	0	0	0	12	2	0	5	0	0	0	0	0	39	0	3	0	1	83	
7:25 AM	1	13	0	0	0	0	27	5	0	1	0	0	0	0	0	39	0	1	0	0	87	
7:30 AM	2	8	0	0	0	0	23	3	0	2	0	0	0	0	0	37	0	5	0	0	80	
7:35 AM	3	12	0	0	0	0	32	7	0	0	0	0	0	0	0	49	0	4	0	2	109	
7:40 AM	1	16	0	0	0	0	36	10	0	0	0	0	0	0	0	50	0	2	0	3	118	
7:45 AM	3	13	0	0	0	0	34	13	0	0	0	0	0	0	0	45	0	1	0	1	110	
7:50 AM	1	26	0	0	0	0	28	8	0	1	0	0	0	0	0	83	1	3	0	1	152	
7:55 AM	2	25	0	0	0	0	51	13	0	0	0	0	0	0	0	78	0	5	0	0	174	1221
8:00 AM	2	30	0	0	0	0	40	5	0	0	0	0	0	0	0	71	0	3	0	1	152	1299
8:05 AM	2	22	0	0	0	0	31	10	0	0	0	0	0	0	0	96	0	9	0	1	171	1394
8:10 AM	1	32	0	0	0	0	32	12	0	0	0	0	0	0	0	73	0	2	0	2	154	1475
8:15 AM	1	29	0	0	0	0	33	10	0	0	0	0	0	0	0	76	0	3	0	0	152	1542
8:20 AM	1	31	0	0	0	0	38	6	0	2	0	0	0	0	0	43	0	1	0	0	122	1581
8:25 AM	0	18	0	0	0	0	35	6	0	0	0	0	0	0	0	90	1	4	0	2	156	1650
8:30 AM	4	22	0	0	0	0	33	6	0	1	0	0	0	0	0	51	0	3	0	0	120	1690
8:35 AM	1	8	0	0	0	0	21	6	0	1	0	0	0	0	0	64	3	2	0	0	106	1687
8:40 AM	5	11	0	0	0	0	24	4	0	0	0	0	0	0	0	52	1	1	0	0	98	1667
8:45 AM	5	13	0	0	0	0	26	4	0	0	0	0	0	0	0	55	0	2	0	0	105	1662
8:50 AM	2	14	0	0	0	0	18	3	0	0	0	0	0	0	0	41	0	1	0	1	80	1590
8:55 AM	4	17	0	0	0	0	22	3	0	0	0	0	0	0	0	46	0	0	0	1	93	1509
Peak 15-Min Flowrates	Northbound					Southbound					Eastbound					Westbound					Total	
	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*		
All Vehicles	24	308	0	0	0	0	488	112	0	0	0	0	0	0	0	980	0	68	0	8	1988	
Heavy Trucks	4	0	0	0	0	0	16	4	0	0	0	0	0	0	0	12	0	0	0	0	36	
Pedestrians	0	0	0	0	0	0	0	0	0	0	20	0	0	0	0	0	0	0	0	0	20	
Bicycles	0	0	0	0	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	0	3	
Railroad																						
Stopped Buses																						

Comments: N/A

ITM Peak Hour Summary

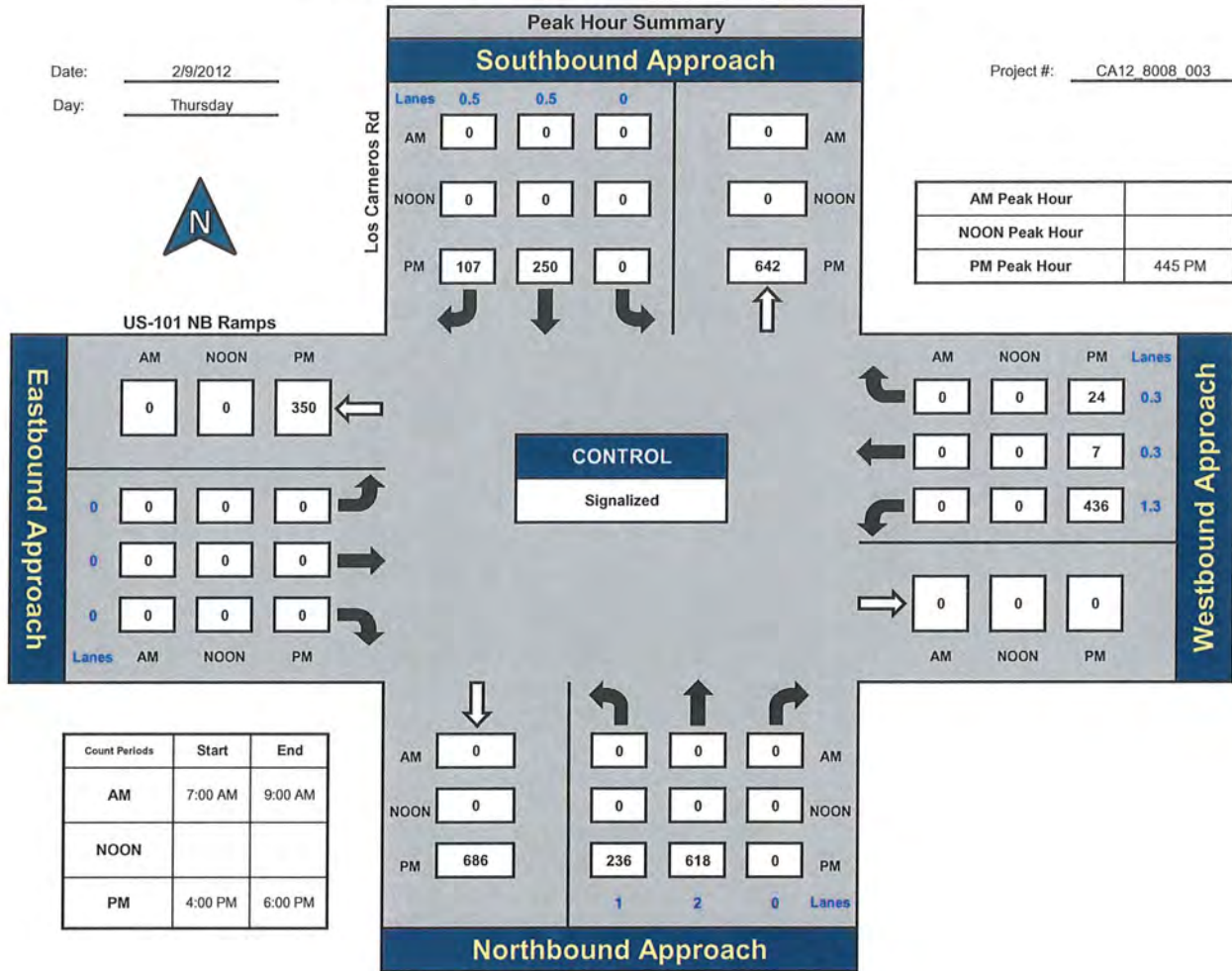


Prepared by:
National Data & Surveying Services

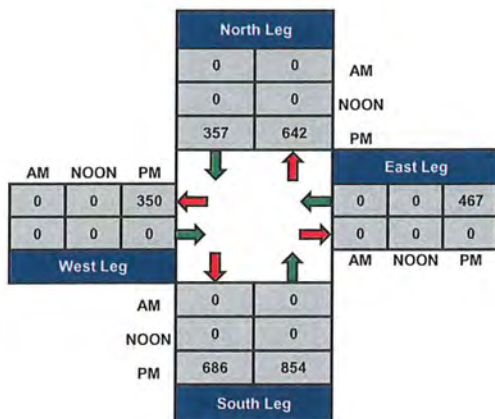
Los Carneros Rd and US-101 NB Ramps, City of Goleta

Date: 2/9/2012
Day: Thursday

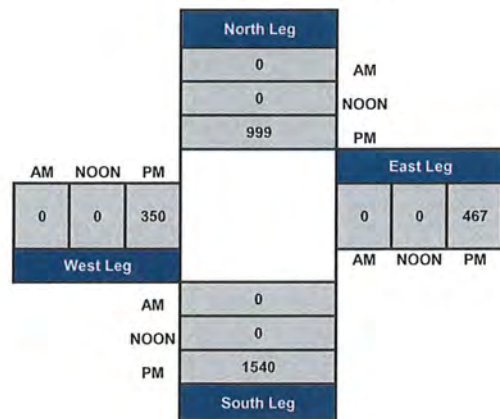
Project #: CA12_8008_003



Total Ins & Outs



Total Volume Per Leg

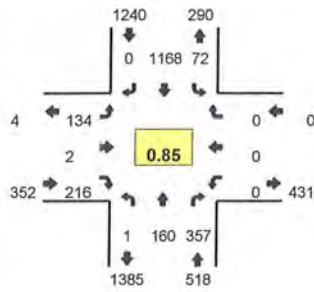


Type of peak hour being reported: Intersection Peak

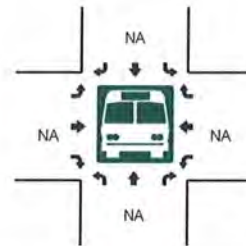
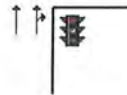
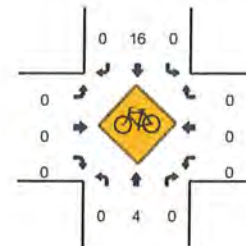
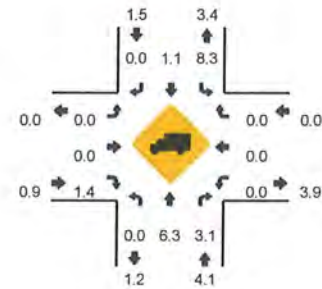
Method for determining peak hour: Total Entering Volume

LOCATION: Los Carneros Rd -- US 101 SB Ramps
 CITY/STATE: Goleta, CA

QC JOB #: 10938621
 DATE: Wed, May 22 2013



Peak-Hour: 7:40 AM -- 8:40 AM
 Peak 15-Min: 7:50 AM -- 8:05 AM



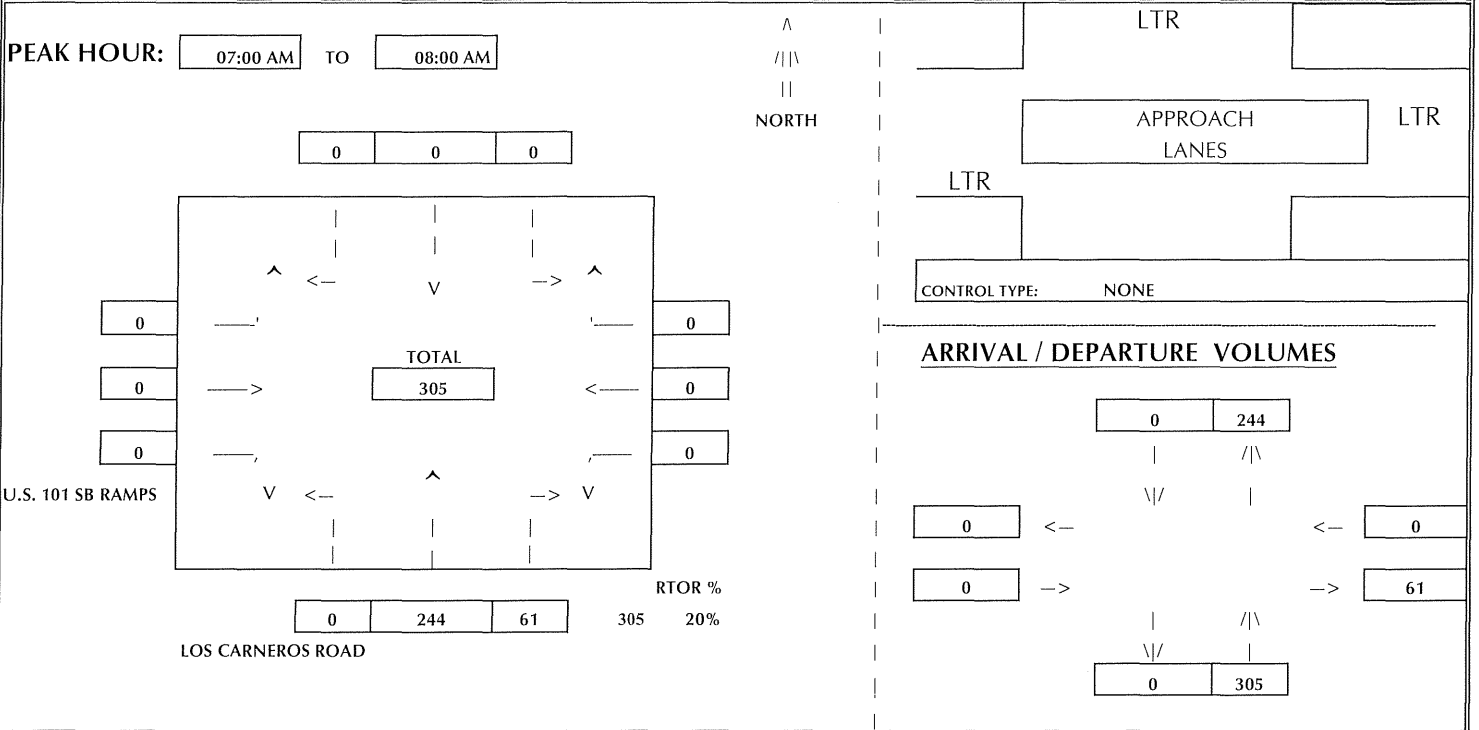
5-Min Count Period	Los Carneros Rd (Northbound)					Los Carneros Rd (Southbound)					US 101 SB Ramps (Eastbound)					US 101 SB Ramps (Westbound)					Total	Hourly Totals
	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*		
7:00 AM	0	8	22	1	5	5	63	0	0	0	3	0	2	0	16	0	0	0	0	0	125	
7:05 AM	0	8	13	1	2	1	55	0	0	0	4	0	8	0	13	0	0	0	0	0	105	
7:10 AM	0	7	20	0	2	2	52	0	0	0	2	1	0	0	9	0	0	0	0	0	95	
7:15 AM	0	12	15	0	13	4	60	0	0	0	6	0	4	0	11	0	0	0	0	0	125	
7:20 AM	0	11	21	0	5	2	56	0	0	0	7	0	8	0	18	0	0	0	0	0	128	
7:25 AM	0	2	28	0	3	6	53	0	0	0	7	0	6	0	16	0	0	0	0	0	121	
7:30 AM	0	10	34	0	6	9	60	0	0	0	7	0	0	0	14	0	0	0	0	0	140	
7:35 AM	0	8	24	0	5	8	73	0	0	0	3	0	5	0	10	0	0	0	0	0	136	
7:40 AM	0	12	32	0	14	13	83	0	0	0	8	0	6	1	15	0	0	0	0	0	184	
7:45 AM	0	9	28	0	11	8	78	0	0	0	7	1	7	0	21	0	0	0	0	0	170	
7:50 AM	0	17	22	0	8	7	97	0	0	0	15	0	14	0	10	0	0	0	0	0	190	
7:55 AM	0	14	17	1	11	11	134	0	0	0	14	0	15	1	13	0	0	0	0	0	231	1750
8:00 AM	0	12	6	0	22	7	103	0	0	0	20	0	10	1	15	0	0	0	0	0	196	1821
8:05 AM	0	12	13	0	7	6	86	0	0	0	10	0	3	0	8	0	0	0	0	0	145	1861
8:10 AM	0	22	37	0	6	3	114	0	0	0	14	0	4	0	10	0	0	0	0	0	210	1976
8:15 AM	0	16	18	0	12	4	101	0	0	0	10	0	10	0	7	0	0	0	0	0	178	2029
8:20 AM	0	13	14	0	0	3	86	0	0	0	14	0	7	0	7	0	0	0	0	0	144	2045
8:25 AM	0	16	21	0	6	3	111	0	0	0	8	1	2	0	8	0	0	0	0	0	176	2100
8:30 AM	0	11	21	0	2	3	87	0	0	0	5	0	2	0	7	0	0	0	0	0	138	2098
8:35 AM	0	6	27	0	2	4	88	0	0	0	5	0	7	1	8	0	0	0	0	0	148	2110
8:40 AM	0	11	26	0	4	5	64	0	0	0	3	0	1	1	12	0	0	0	0	0	127	2053
8:45 AM	0	16	39	0	1	2	71	0	0	0	4	0	0	0	7	0	0	0	0	0	140	2023
8:50 AM	0	16	32	0	1	5	54	0	0	0	3	0	1	0	5	0	0	0	0	0	117	1950
8:55 AM	0	10	11	0	4	3	73	0	0	0	8	0	2	2	13	0	0	0	0	0	126	1845
Peak 15-Min Flowrates	Northbound					Southbound					Eastbound					Westbound					Total	
	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*		
All Vehicles	0	172	180	4	164	100	1336	0	0	0	196	0	156	8	152	0	0	0	0	0	2468	
Heavy Trucks	0	12	4			12	12	0			0	0	4			0	0	0			44	
Pedestrians		0					0					24					0				24	
Bicycles	0	1	0			0	4	0			0	0	0			0	0	0			5	
Railroad																						
Stopped Buses																						

Comments: N/A

ASSOCIATED TRANSPORTATION ENGINEERS

INTERSECTION TURNING MOVEMENT SUMMARY

PROJECT: HERITAGE RIDGE **PROJECT #:** 12089.01 **COUNT DATE:** 07-17-15 **FILE NAME:** *3 AM_RTOR
N-S Approach: LOS CARNEROS ROAD **COUNT TIME:** 07:30 AM TO 8:30
E-W Approach: U.S. 101 SB RAMPS **CITY:** GOLETA **WEATHER:** SUNNY



TIME PERIOD			NORTHBOUND		SOUTHBOUND			EASTBOUND			WESTBOUND			TOTAL VOLUMES
From	--	To	RIGHT	RTOR	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
COUNT DATA														
07:30 AM	--	07:45 AM	59	23										82
07:45 AM	--	08:00 AM	123	45										168
08:00 AM	--	08:15 AM	188	51										239
08:15 AM	--	08:30 AM	244	61										305
08:30 AM	--	08:45 AM												0
08:45 AM	--	09:00 AM												0
09:00 AM	--	09:15 AM												0
09:15 AM	--	09:30 AM												0
TOTAL BY PERIOD														
07:30 AM	--	07:45 AM	0	59	23	0	0	0	0	0	0	0	0	82
07:45 AM	--	08:00 AM	0	64	22	0	0	0	0	0	0	0	0	86
08:00 AM	--	08:15 AM	0	65	6	0	0	0	0	0	0	0	0	71
08:15 AM	--	08:30 AM	0	56	10	0	0	0	0	0	0	0	0	66
HOURLY TOTALS														
07:00 AM	--	08:00 AM	0	244	61	0	0	0	0	0	0	0	0	305

ITM Peak Hour Summary

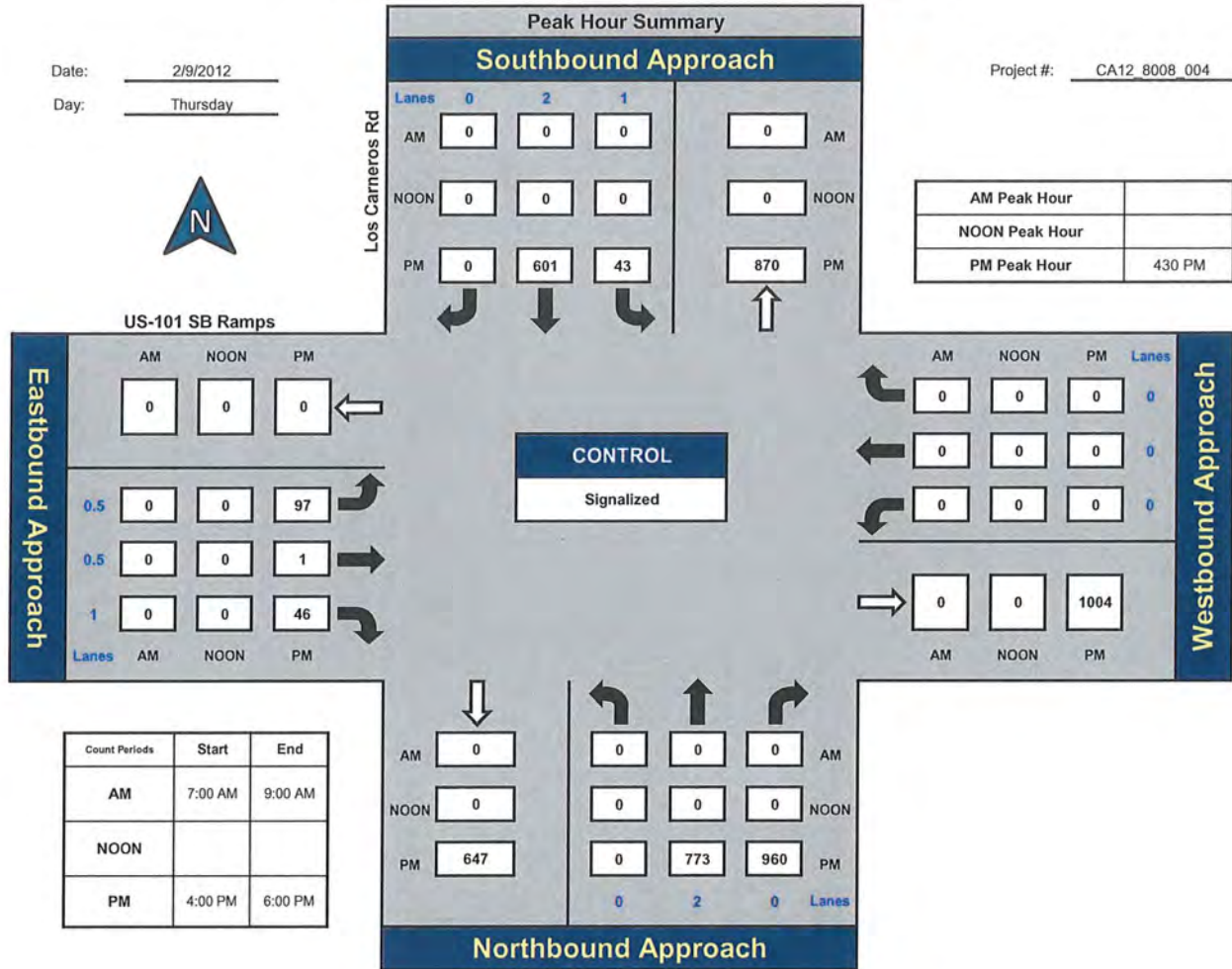


Prepared by:
National Data & Surveying Services

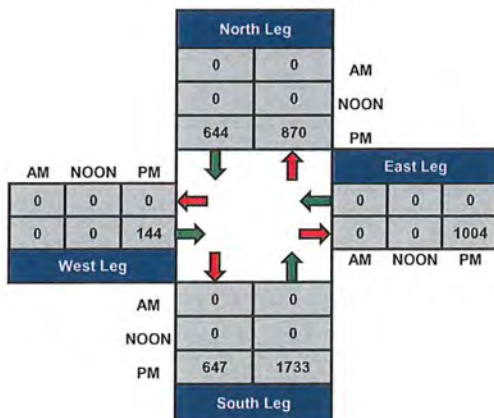
Los Carneros Rd and US-101 SB Ramps, City of Goleta

Date: 2/9/2012
Day: Thursday

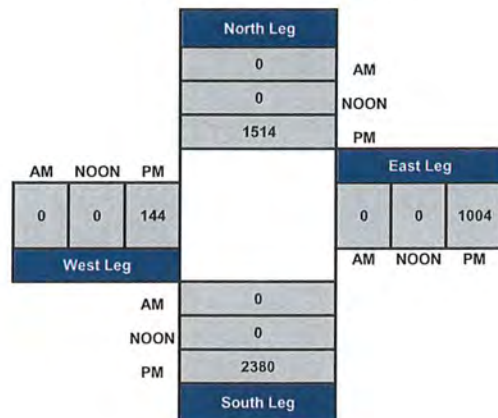
Project #: CA12_8008_004



Total Ins & Outs



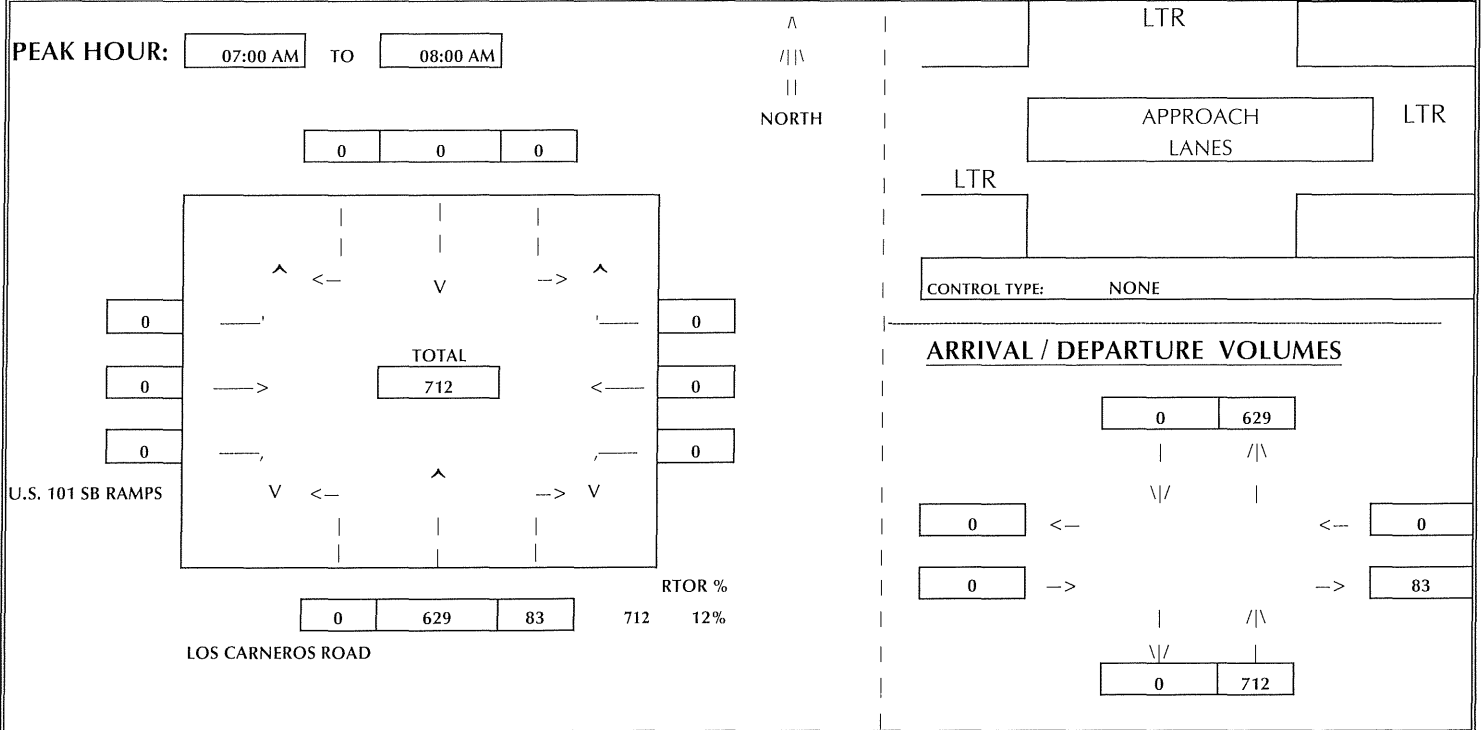
Total Volume Per Leg



ASSOCIATED TRANSPORTATION ENGINEERS

INTERSECTION TURNING MOVEMENT SUMMARY

PROJECT: HERITAGE RIDGE **PROJECT #:** 12089.01 **COUNT DATE:** 07-16-15 **FILE NAME:** #3 PM_RTOR
N-S Approach: LOS CARNEROS ROAD **COUNT TIME:** 07:30 AM TO 8:30
E-W Approach: U.S. 101 SB RAMPS **CITY:** GOLETA **WEATHER:** SUNNY



TIME PERIOD		NORTHBOUND		SOUTHBOUND			EASTBOUND			WESTBOUND			TOTAL
From	To	RIGHT	RTOR	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	VOLUMES

COUNT DATA													
04:30 PM	--	04:45 PM	139	12									151
04:45 PM	--	05:00 PM	323	42									365
05:00 PM	--	05:15 PM	478	65									543
05:15 PM	--	05:30 PM	629	83									712
05:30 PM	--	05:45 PM											0
05:45 PM	--	06:00 PM											0

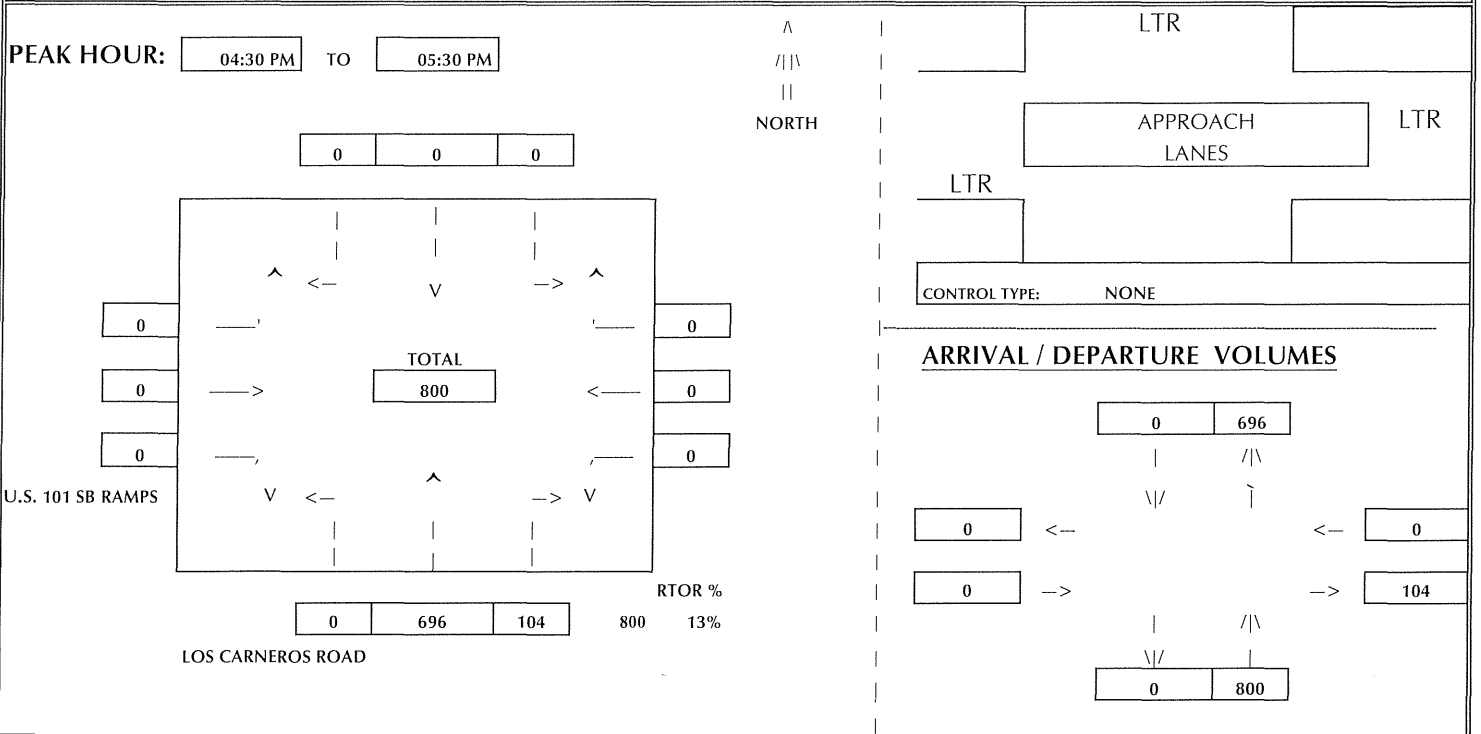
TOTAL BY PERIOD													
04:30 PM	--	04:45 PM	0	139	12	0	0	0	0	0	0	0	151
04:45 PM	--	05:00 PM	0	184	30	0	0	0	0	0	0	0	214
05:00 PM	--	05:15 PM	0	155	23	0	0	0	0	0	0	0	178
05:15 PM	--	05:30 PM	0	151	18	0	0	0	0	0	0	0	169

HOURLY TOTALS													
07:00 AM	--	08:00 AM	0	629	83	0	0	0	0	0	0	0	712

ASSOCIATED TRANSPORTATION ENGINEERS

INTERSECTION TURNING MOVEMENT SUMMARY

PROJECT: HERITAGE RIDGE **PROJECT #:** 12089.01 **COUNT DATE:** 10-29-15 **FILE NAME:** PM_RTOR
N-S Approach: LOS CARNEROS ROAD **COUNT TIME:** 04:30 AM TO 8:30
E-W Approach: U.S. 101 SB RAMPS **CITY:** GOLETA **WEATHER:** SUNNY



TIME PERIOD			NORTHBOUND		SOUTHBOUND			EASTBOUND			WESTBOUND			TOTAL VOLUMES	
			RIGHT	RTOR	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right		
COUNT DATA															
04:30 PM	---	04:45 PM	162	19										181	
04:45 PM	---	05:00 PM	300	38										338	
05:00 PM	---	05:15 PM	505	65										570	
05:15 PM	---	05:30 PM	696	104										800	
05:30 PM	---	05:45 PM												0	
05:45 PM	---	06:00 PM												0	
TOTAL BY PERIOD															
04:30 PM	---	04:45 PM	0	162	19	0	0	0	0	0	0	0	0	0	181
04:45 PM	---	05:00 PM	0	138	19	0	0	0	0	0	0	0	0	0	157
05:00 PM	---	05:15 PM	0	205	27	0	0	0	0	0	0	0	0	0	232
05:15 PM	---	05:30 PM	0	191	39	0	0	0	0	0	0	0	0	0	230
HOURLY TOTALS															
04:30 PM	---	05:30 PM	0	696	104	0	0	0	0	0	0	0	0	0	800

ITM Peak Hour Summary

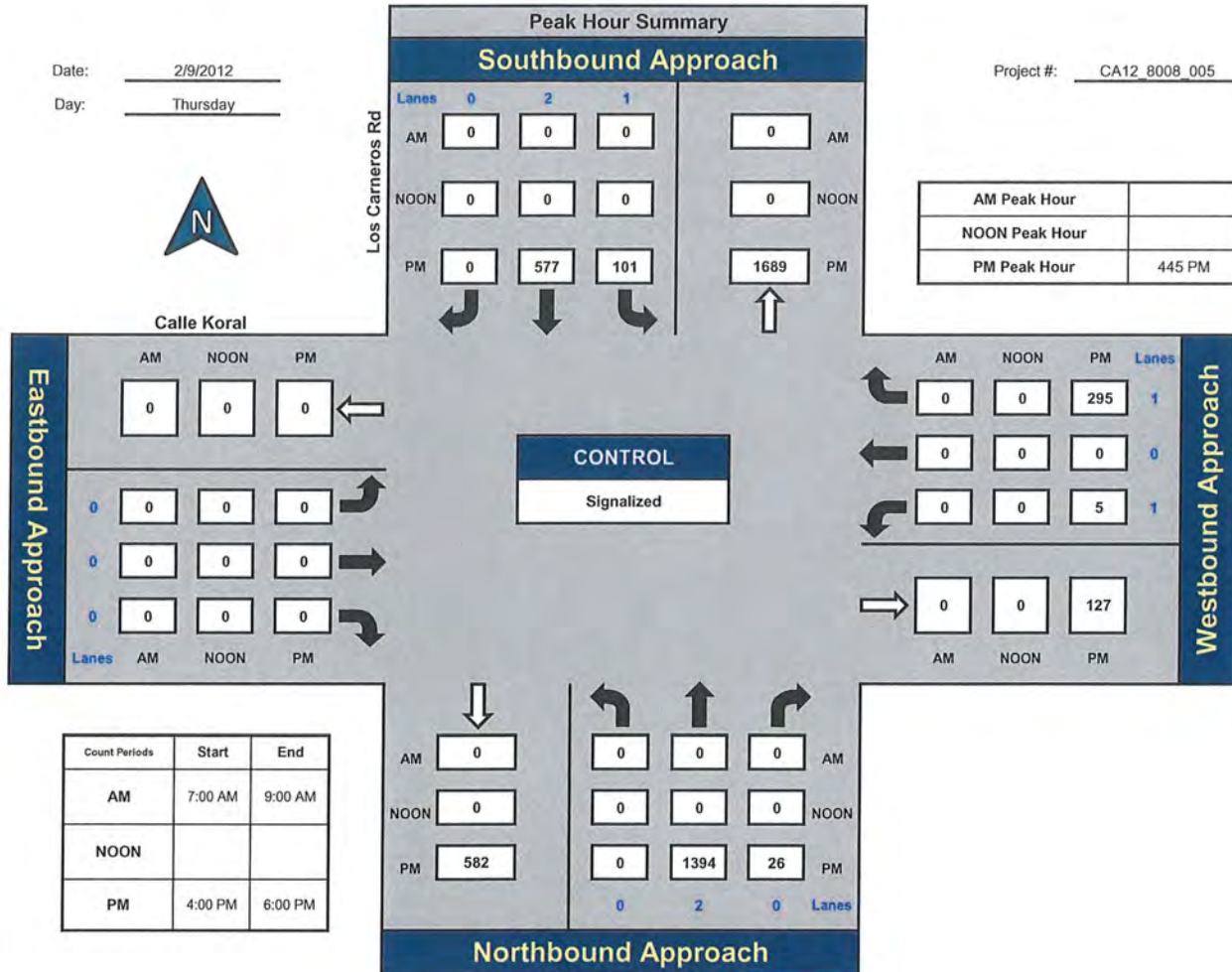


Prepared by:
National Data & Surveying Services

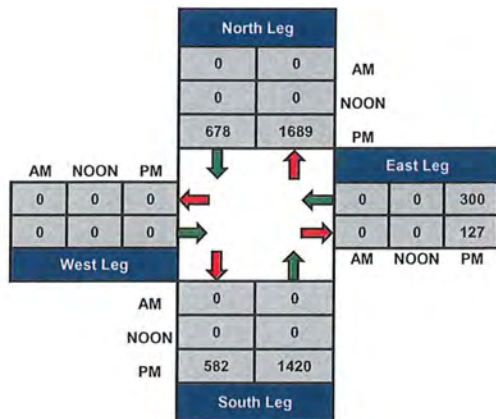
Los Carneros Rd and Calle Koral, City of Goleta

Date: 2/9/2012
Day: Thursday

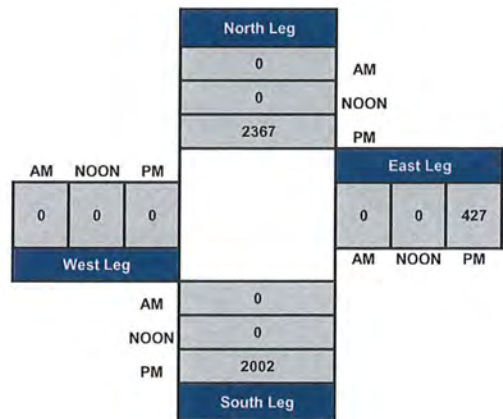
Project #: CA12_8008_005



Total Ins & Outs



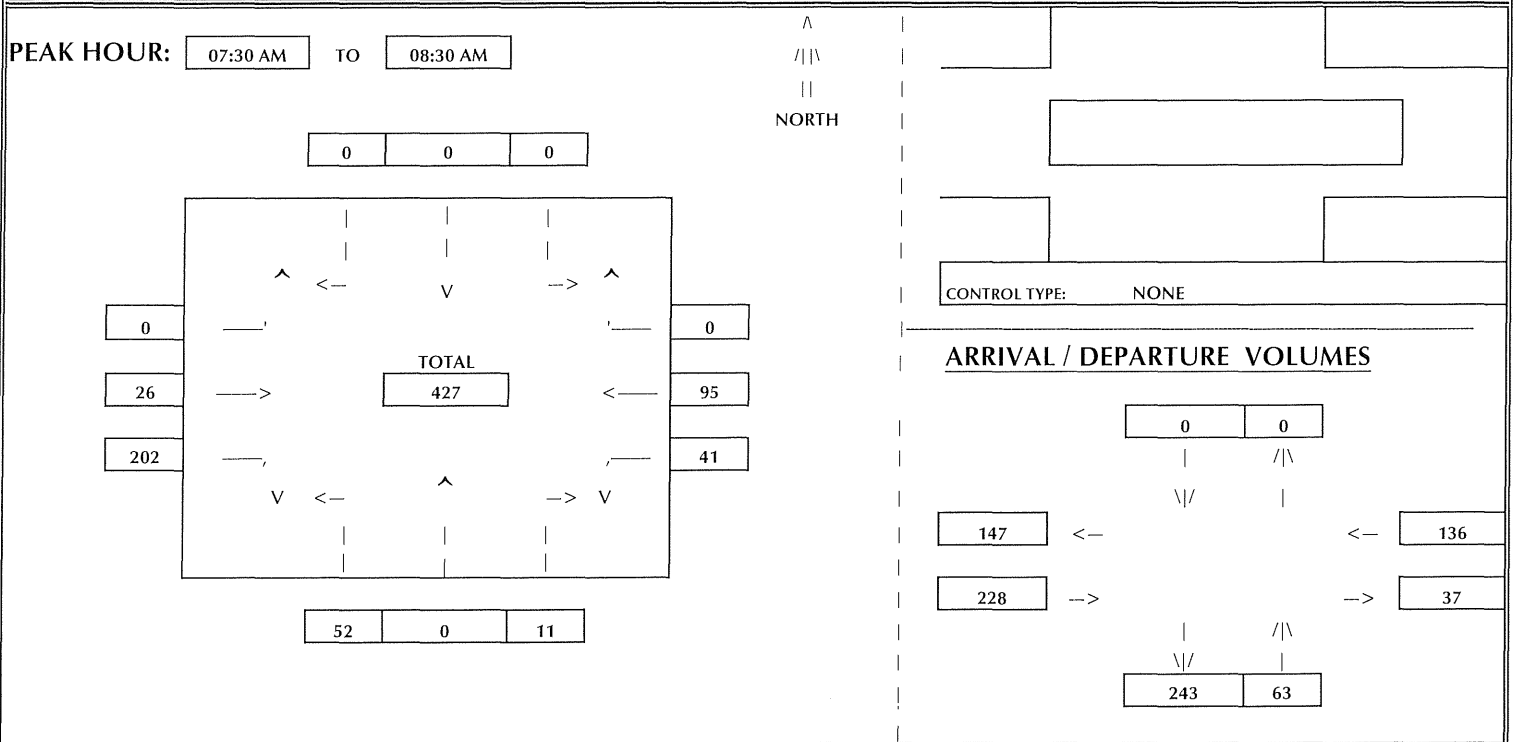
Total Volume Per Leg



ASSOCIATED TRANSPORTATION ENGINEERS

INTERSECTION TURNING MOVEMENT SUMMARY

PROJECT: WILLOW SPRINGS NORTH **PROJECT #:** 12089.01 **COUNT DATE:** 02-13-2014 **FILE NAME:** 05_AM
N-S Approach: LOS CARNEROS WAY **COUNT TIME:** 07:00 AM TO 9:00
E-W Approach: CALLE KORAL **CITY:** GOLETA **WEATHER:** SUNNY



TIME PERIOD		NORTHBOUND			SOUTHBOUND			EASTBOUND			WESTBOUND			TOTAL
From	To	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	VOLUMES

COUNT DATA

07:00 AM	---	07:15 AM	12	0	0	0	0	0	0	2	42	8	7	0	71
07:15 AM	---	07:30 AM	28	0	0	0	0	0	0	4	75	15	20	0	142
07:30 AM	---	07:45 AM	45	0	0	0	0	0	0	7	120	21	44	0	237
07:45 AM	---	08:00 AM	62	0	2	0	0	0	0	13	185	31	68	0	361
08:00 AM	---	08:15 AM	77	0	3	0	0	0	0	20	243	47	89	0	479
08:15 AM	---	08:30 AM	80	0	11	0	0	0	0	30	277	56	115	0	569
08:30 AM	---	08:45 AM	91	0	13	0	0	0	0	36	315	61	124	0	640
08:45 AM	---	09:00 AM	96	0	14	0	0	0	0	38	347	67	149	0	711

TOTAL BY PERIOD

07:00 AM	---	07:15 AM	12	0	0	0	0	0	0	2	42	8	7	0	71
07:15 AM	---	07:30 AM	16	0	0	0	0	0	0	2	33	7	13	0	71
07:30 AM	---	07:45 AM	17	0	0	0	0	0	0	3	45	6	24	0	95
07:45 AM	---	08:00 AM	17	0	2	0	0	0	0	6	65	10	24	0	124
08:00 AM	---	08:15 AM	15	0	1	0	0	0	0	7	58	16	21	0	118
08:15 AM	---	08:30 AM	3	0	8	0	0	0	0	10	34	9	26	0	90
08:30 AM	---	08:45 AM	11	0	2	0	0	0	0	6	38	5	9	0	71
08:45 AM	---	09:00 AM	5	0	1	0	0	0	0	2	32	6	25	0	71

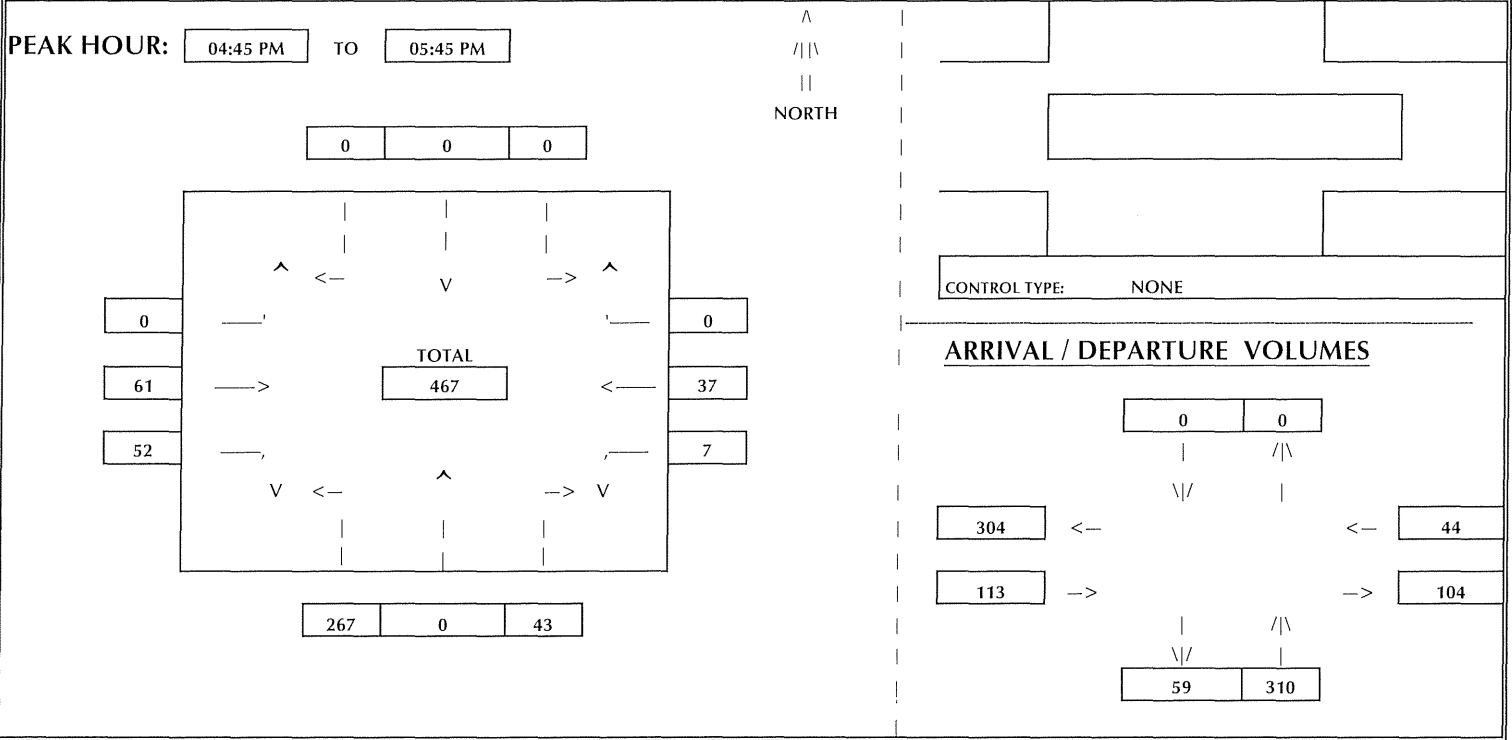
HOURLY TOTALS

07:00 AM	---	08:00 AM	62	0	2	0	0	0	0	13	185	31	68	0	361
07:15 AM	---	08:15 AM	65	0	3	0	0	0	0	18	201	39	82	0	408
07:30 AM	---	08:30 AM	52	0	11	0	0	0	0	26	202	41	95	0	427
07:45 AM	---	08:45 AM	46	0	13	0	0	0	0	29	195	40	80	0	403
08:00 AM	---	09:00 AM	34	0	12	0	0	0	0	25	162	36	81	0	350

ASSOCIATED TRANSPORTATION ENGINEERS

INTERSECTION TURNING MOVEMENT SUMMARY

PROJECT: WILLOW SPRINGS NORTH **PROJECT #:** 12089.01 **COUNT DATE:** 02-13-2014 **FILE NAME:** 05_PM
N-S Approach: LOS CARNEROS WAY **COUNT TIME:** 04:00 PM TO 6:00
E-W Approach: CALLE KORAL **CITY:** GOLETA **WEATHER:** SUNNY



TIME PERIOD		NORTHBOUND			SOUTHBOUND			EASTBOUND			WESTBOUND			TOTAL VOLUMES
From	To	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
COUNT DATA														
04:00 PM	04:15 PM	43	0	5	0	0	0	0	11	11	3	9	0	82
04:15 PM	04:30 PM	93	0	12	0	0	0	0	18	35	7	11	0	176
04:30 PM	04:45 PM	156	0	14	0	0	0	0	26	44	7	16	0	263
04:45 PM	05:00 PM	220	0	21	0	0	0	0	42	55	8	28	0	374
05:00 PM	05:15 PM	293	0	31	0	0	0	0	51	73	10	34	0	492
05:15 PM	05:30 PM	360	0	48	0	0	0	0	71	83	12	40	0	614
05:30 PM	05:45 PM	423	0	57	0	0	0	0	87	96	14	53	0	730
05:45 PM	06:00 PM	462	0	68	0	0	0	0	101	107	18	62	0	818

TOTAL BY PERIOD														
04:00 PM	04:15 PM	43	0	5	0	0	0	0	11	11	3	9	0	82
04:15 PM	04:30 PM	50	0	7	0	0	0	0	7	24	4	2	0	94
04:30 PM	04:45 PM	63	0	2	0	0	0	0	8	9	0	5	0	87
04:45 PM	05:00 PM	64	0	7	0	0	0	0	16	11	1	12	0	111
05:00 PM	05:15 PM	73	0	10	0	0	0	0	9	18	2	6	0	118
05:15 PM	05:30 PM	67	0	17	0	0	0	0	20	10	2	6	0	122
05:30 PM	05:45 PM	63	0	9	0	0	0	0	16	13	2	13	0	116
05:45 PM	06:00 PM	39	0	11	0	0	0	0	14	11	4	9	0	88

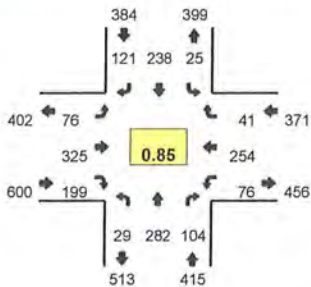
HOURLY TOTALS														
04:00 PM	05:00 PM	220	0	21	0	0	0	0	42	55	8	28	0	374
04:15 PM	05:15 PM	250	0	26	0	0	0	0	40	62	7	25	0	410
04:30 PM	05:30 PM	267	0	36	0	0	0	0	53	48	5	29	0	438
04:45 PM	05:45 PM	267	0	43	0	0	0	0	61	52	7	37	0	467
05:00 PM	06:00 PM	242	0	47	0	0	0	0	59	52	10	34	0	444

Type of peak hour being reported: Intersection Peak

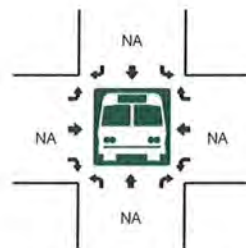
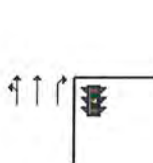
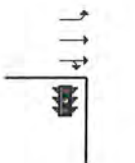
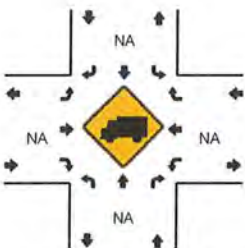
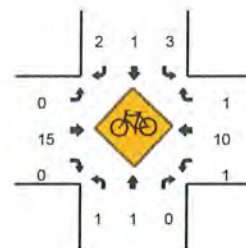
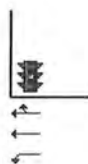
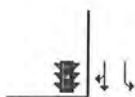
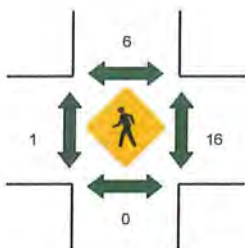
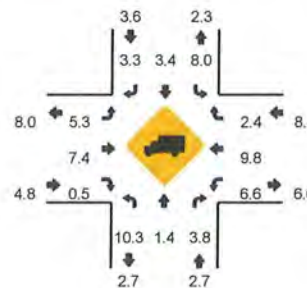
Method for determining peak hour: Total Entering Volume

LOCATION: Los Carneros Rd -- Hollister Ave
CITY/STATE: Goleta, CA

QC JOB #: 10938617
DATE: Wed, May 22 2013



Peak-Hour: 7:50 AM -- 8:50 AM
Peak 15-Min: 7:50 AM -- 8:05 AM



R* = RTOR

5-Min Count Period Beginning At	Los Carneros Rd (Northbound)				Los Carneros Rd (Southbound)				Hollister Ave (Eastbound)				Hollister Ave (Westbound)				Total	Hourly Totals					
	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left			Thru	Right	U	R*	
7:00 AM	1	13	0	0	2	0	18	6	0	0	5	19	0	0	6	3	15	3	0	0	91		
7:05 AM	1	18	1	0	0	2	10	10	0	0	4	21	4	0	5	2	18	0	0	2	98		
7:10 AM	2	18	1	0	2	0	12	3	0	0	1	21	5	0	3	1	21	1	0	0	91		
7:15 AM	3	20	3	0	2	1	27	4	0	0	3	18	10	0	2	2	5	3	0	2	105		
7:20 AM	3	16	2	0	1	0	16	7	0	2	6	17	4	0	7	2	23	3	0	1	110		
7:25 AM	3	33	3	0	0	1	15	4	0	1	4	30	4	0	8	6	14	1	0	1	128		
7:30 AM	1	32	2	0	0	0	18	9	0	1	3	28	5	0	5	5	9	0	0	1	119		
7:35 AM	1	29	2	0	1	3	14	10	0	0	4	25	2	0	7	3	18	2	0	0	121		
7:40 AM	0	27	6	0	0	1	9	7	0	0	8	28	8	0	2	9	18	3	0	0	126		
7:45 AM	2	28	1	0	3	3	19	7	0	0	5	25	20	0	7	5	14	4	0	0	143		
7:50 AM	3	24	9	0	1	0	14	8	0	0	12	34	18	0	2	9	25	1	0	1	161		
7:55 AM	7	19	3	0	3	5	32	10	0	0	10	41	20	0	8	11	25	4	1	1	200	1493	
8:00 AM	3	26	13	0	2	2	24	15	0	0	6	21	10	0	11	3	17	4	0	0	157	1559	
8:05 AM	3	20	6	0	5	0	13	9	0	1	10	32	17	0	2	6	24	5	0	1	154	1615	
8:10 AM	1	29	10	1	0	3	17	5	0	3	5	23	1	0	4	9	21	2	1	1	136	1660	
8:15 AM	2	19	5	0	6	0	27	20	0	0	7	35	14	0	2	5	15	0	0	1	158	1713	
8:20 AM	1	23	2	0	7	1	7	7	0	0	4	32	7	0	8	3	19	2	0	3	126	1729	
8:25 AM	1	24	4	0	2	2	31	11	0	0	3	20	4	0	8	4	21	1	0	0	136	1737	
8:30 AM	0	19	3	1	3	4	17	9	0	2	6	29	8	0	7	6	27	2	0	3	148	1764	
8:35 AM	3	28	8	0	0	2	23	5	0	0	4	16	10	0	3	4	16	1	0	0	123	1766	
8:40 AM	3	16	1	0	4	2	17	7	0	1	1	23	10	0	6	5	26	4	0	0	126	1766	
8:45 AM	0	35	7	0	0	4	16	8	0	0	8	19	11	0	8	9	18	4	0	0	147	1770	
8:50 AM	4	28	8	0	0	3	23	16	0	0	4	23	7	0	6	4	21	4	0	2	153	1762	
8:55 AM	5	18	5	0	2	0	18	14	0	0	3	19	7	0	8	7	18	0	0	1	125	1687	
Peak 15-Min Flowrates	Northbound				Southbound				Eastbound				Westbound				Total						
All Vehicles	52	276	100	0	24	28	280	132	0	0	112	384	192	0	84	92			268	36	4	8	2072
Heavy Trucks	8	4	8			4	8	4			4	20	4			8	20	4			96		
Pedestrians		0					8					4					16				28		
Bicycles	1	0	0			0	1	0			0	7	0			0	6	1			16		
Railroad																							
Stopped Buses																							

Comments: N/A

ITM Peak Hour Summary



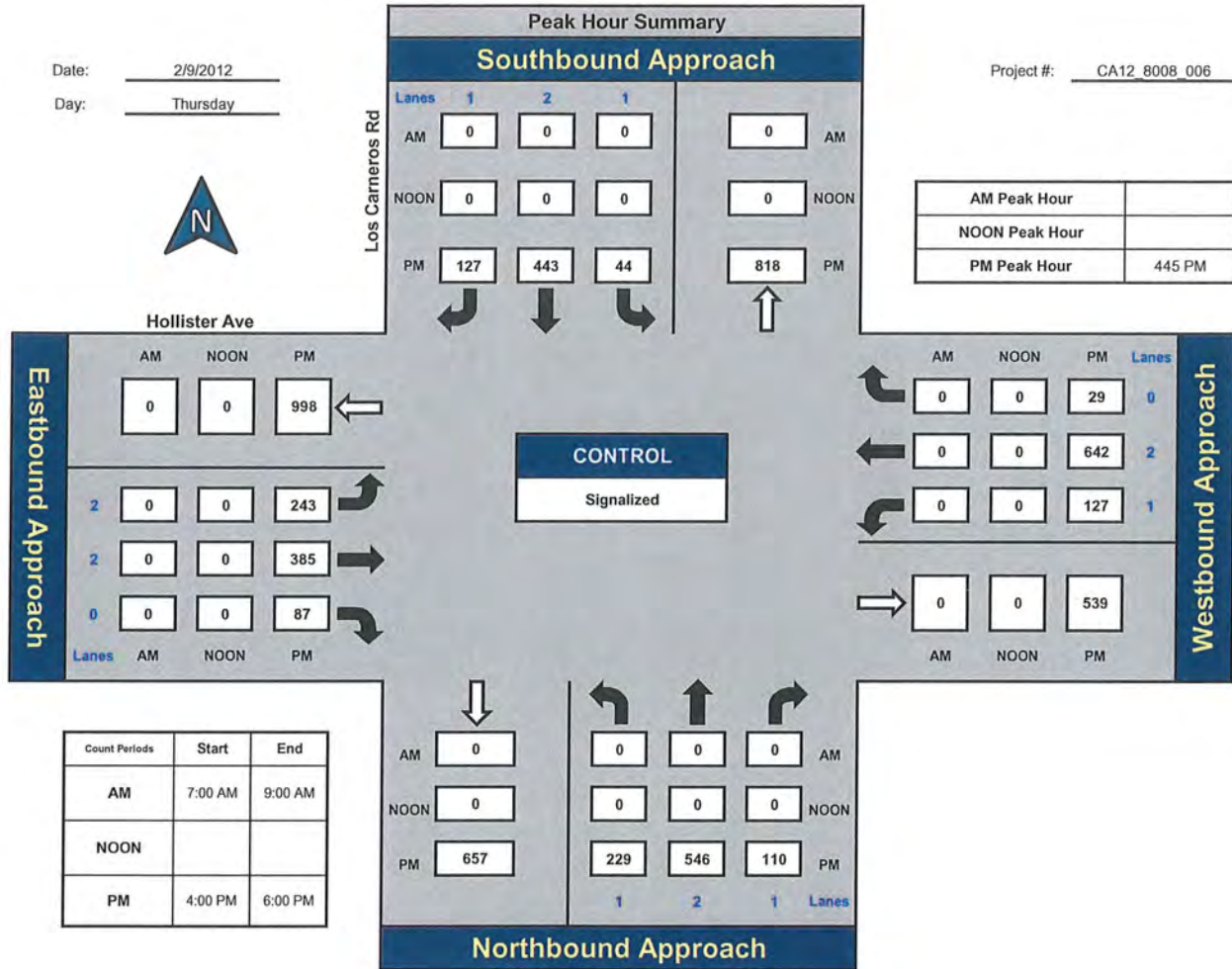
National Data & Surveying Services

Los Carneros Rd and Hollister Ave, City of Goleta

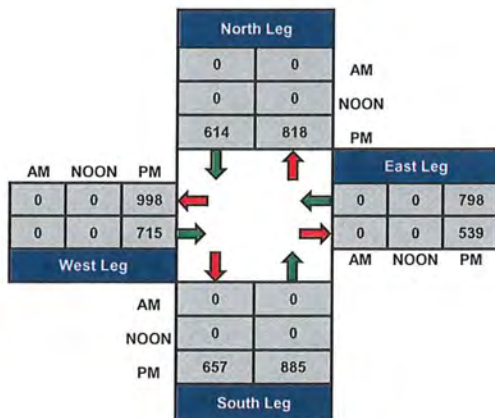
Date: 2/9/2012

Day: Thursday

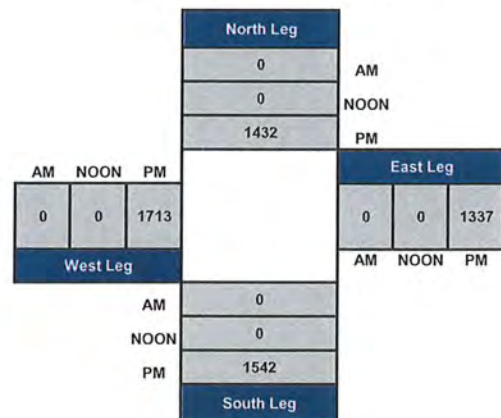
Project #: CA12_8008_006



Total Ins & Outs



Total Volume Per Leg

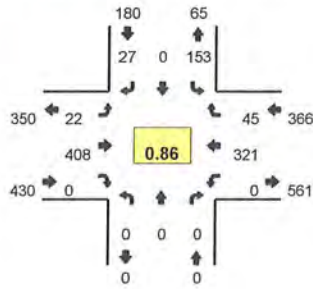


Type of peak hour being reported: Intersection Peak

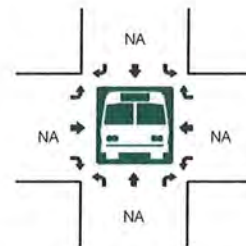
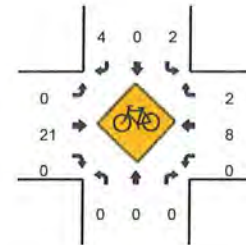
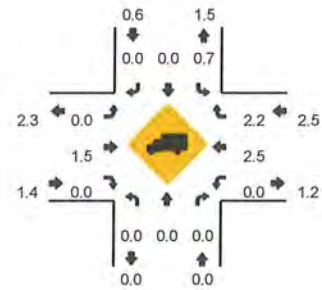
Method for determining peak hour: Total Entering Volume

LOCATION: Los Carneros Way -- Hollister Ave
CITY/STATE: Goleta, CA

QC JOB #: 10938819
DATE: Wed, Apr 03 2013



Peak-Hour: 7:40 AM -- 8:40 AM
Peak 15-Min: 7:50 AM -- 8:05 AM



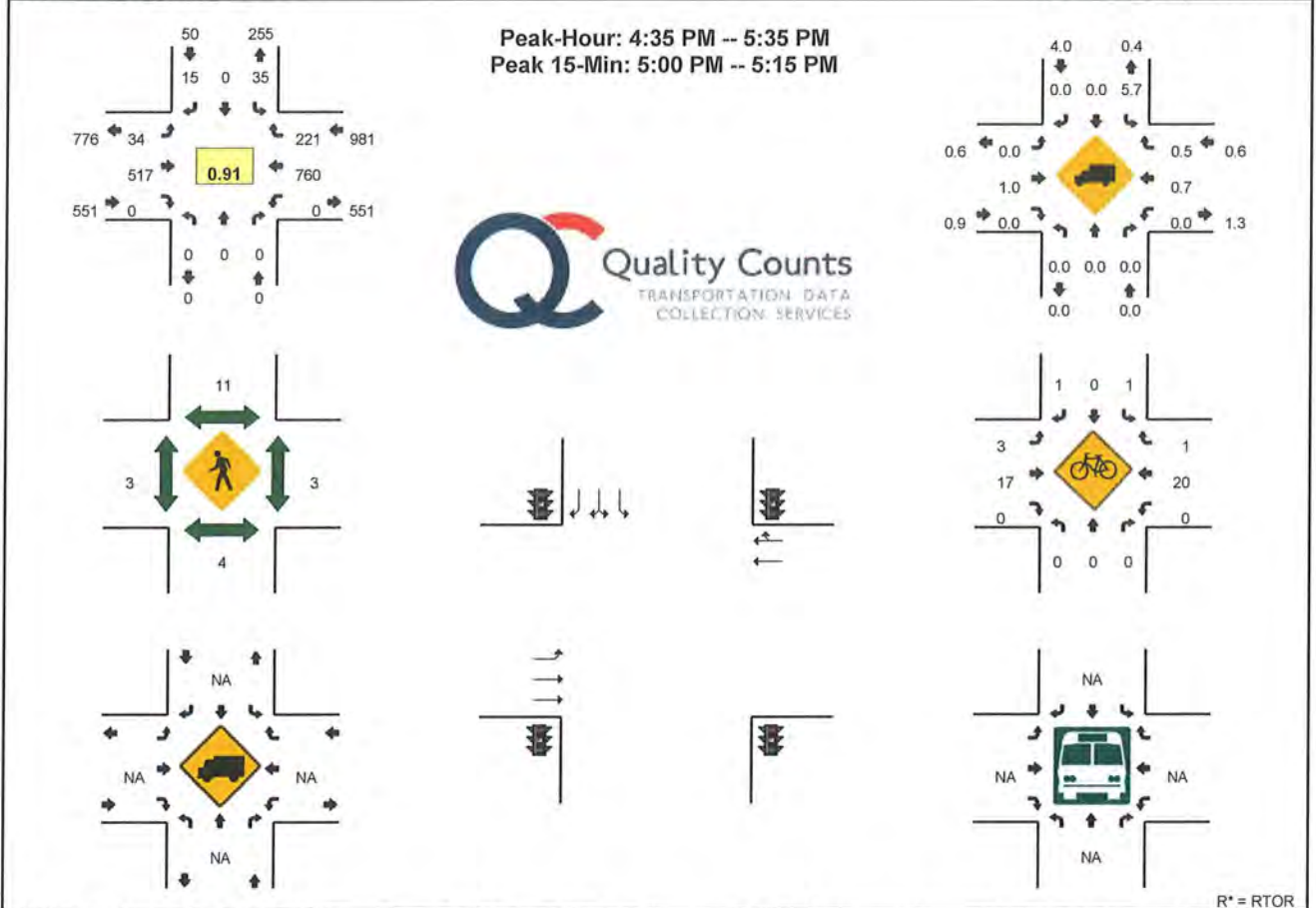
5-Min Count Period Beginning At	Los Carneros Way (Northbound)					Los Carneros Way (Southbound)					Hollister Ave (Eastbound)					Hollister Ave (Westbound)					Total	Hourly Totals
	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*		
7:00 AM	0	0	0	0	0	10	0	0	0	1	0	33	0	0	0	0	18	3	0	0	65	
7:05 AM	0	0	0	0	0	4	0	0	0	1	1	21	0	0	0	0	20	3	0	0	50	
7:10 AM	0	0	0	0	0	10	0	1	0	1	1	19	0	0	0	0	18	1	0	2	53	
7:15 AM	0	0	0	0	0	12	0	1	0	0	0	27	0	1	0	0	22	2	0	0	65	
7:20 AM	0	0	0	0	0	16	0	0	0	0	0	22	0	0	0	0	23	3	0	1	65	
7:25 AM	0	0	0	0	0	13	0	0	0	2	1	19	0	0	0	0	23	4	0	0	62	
7:30 AM	0	0	0	0	0	8	0	2	0	2	1	30	0	0	0	0	16	1	0	0	60	
7:35 AM	0	0	0	0	0	12	0	0	0	0	1	25	0	0	0	0	27	5	0	0	70	
7:40 AM	0	0	0	0	0	10	0	1	0	2	0	28	0	0	0	0	28	3	0	1	73	
7:45 AM	0	0	0	0	0	12	0	1	0	2	1	24	0	0	0	0	23	2	0	0	65	
7:50 AM	0	0	0	0	0	10	0	2	0	1	0	50	0	0	0	0	35	4	0	1	103	
7:55 AM	0	0	0	0	0	22	0	0	0	2	3	41	0	1	0	0	24	3	0	2	98	829
8:00 AM	0	0	0	0	0	8	0	2	0	4	4	41	0	0	0	0	21	2	0	0	82	846
8:05 AM	0	0	0	0	0	9	0	1	0	3	2	39	0	0	0	0	32	3	0	0	89	885
8:10 AM	0	0	0	0	0	14	0	1	0	0	1	33	0	1	0	0	32	5	0	0	87	919
8:15 AM	0	0	0	0	0	15	0	0	0	0	2	24	0	0	0	0	27	3	0	1	72	926
8:20 AM	0	0	0	0	0	12	0	1	0	2	1	33	0	0	0	0	23	4	0	1	77	938
8:25 AM	0	0	0	0	0	14	0	0	0	0	3	39	0	0	0	0	24	2	0	0	82	958
8:30 AM	0	0	0	0	0	18	0	0	0	0	0	29	0	0	0	0	22	3	0	1	73	971
8:35 AM	0	0	0	0	0	9	0	0	0	2	3	27	0	0	0	0	30	4	0	0	75	976
8:40 AM	0	0	0	0	0	6	0	0	0	1	0	23	0	0	0	0	24	5	0	0	59	962
8:45 AM	0	0	0	0	0	11	0	1	0	1	0	37	0	0	0	0	23	4	0	0	77	974
8:50 AM	0	0	0	0	0	6	0	0	0	3	2	26	0	0	0	0	35	9	0	1	82	953
8:55 AM	0	0	0	0	0	5	0	0	0	0	0	24	0	0	0	0	26	3	0	0	58	913
Peak 15-Min Flowrates	Northbound					Southbound					Eastbound					Westbound					Total	
Beginning At	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*		
All Vehicles	0	0	0	0	0	160	0	16	0	28	28	528	0	4	0	0	320	36	0	12	1132	
Heavy Trucks	0	0	0	0	0	0	0	0	0	0	0	8	0	0	0	0	12	4	0	0	24	
Pedestrians	0	0	0	0	0	0	4	0	0	0	0	0	0	0	0	0	0	0	0	0	4	
Bicycles	0	0	0	0	0	0	0	3	0	0	0	6	0	0	0	0	4	0	0	0	13	
Railroad	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Stopped Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	

Comments: N/A

Type of peak hour being reported: Intersection Peak

Method for determining peak hour: Total Entering Volume

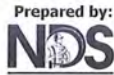
LOCATION: Los Carneros Way -- Hollister Ave
 CITY/STATE: Goleta, CA
 QC JOB #: 10938820
 DATE: Wed, Apr 03 2013



5-Min Count Period	Los Carneros Way (Northbound)					Los Carneros Way (Southbound)					Hollister Ave (Eastbound)					Hollister Ave (Westbound)					Total	Hourly Totals
	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*		
4:00 PM	0	0	0	0	0	5	0	1	0	0	1	28	0	0	0	0	47	8	0	0	90	
4:05 PM	0	0	0	0	0	2	0	0	0	1	3	45	0	0	0	0	56	18	0	0	125	
4:10 PM	0	0	0	0	0	5	0	0	0	2	3	35	0	0	0	0	43	5	0	4	97	
4:15 PM	0	0	0	0	0	3	0	1	0	2	0	35	0	0	0	0	48	12	0	3	104	
4:20 PM	0	0	0	0	0	2	0	0	0	0	2	36	0	0	0	0	45	11	0	1	97	
4:25 PM	0	0	0	0	0	2	0	0	0	0	2	30	0	0	0	0	35	12	0	2	83	
4:30 PM	0	0	0	0	0	5	0	0	0	1	2	51	0	0	0	0	38	16	0	1	114	
4:35 PM	0	0	0	0	0	3	0	1	0	1	3	38	0	0	0	0	71	22	0	3	142	
4:40 PM	0	0	0	0	0	3	0	1	0	3	1	46	0	0	0	0	61	17	0	2	134	
4:45 PM	0	0	0	0	0	2	0	0	0	0	2	52	0	0	0	0	59	12	0	1	128	
4:50 PM	0	0	0	0	0	0	0	0	0	2	2	29	0	0	0	0	64	13	0	0	110	
4:55 PM	0	0	0	0	0	2	0	0	0	1	1	31	0	0	0	0	68	18	0	1	122	
5:00 PM	0	0	0	0	0	5	0	0	0	0	5	42	0	0	0	0	66	20	0	1	139	
5:05 PM	0	0	0	0	0	4	0	0	0	0	3	66	0	0	0	0	67	17	0	4	161	
5:10 PM	0	0	0	0	0	3	0	0	0	1	2	37	0	1	0	0	73	14	0	3	134	
5:15 PM	0	0	0	0	0	6	0	2	0	0	1	45	0	0	0	0	65	7	0	2	128	
5:20 PM	0	0	0	0	0	3	0	0	0	1	2	52	0	0	0	0	55	22	0	2	137	
5:25 PM	0	0	0	0	0	1	0	0	0	1	4	44	0	0	0	0	52	13	0	2	117	
5:30 PM	0	0	0	0	0	2	0	0	1	1	7	35	0	0	0	0	59	24	0	1	130	
5:35 PM	0	0	0	0	0	3	0	0	0	4	3	50	0	0	0	0	32	7	0	2	101	
5:40 PM	0	0	0	0	0	3	0	0	0	1	2	37	0	0	0	0	49	12	0	3	107	
5:45 PM	0	0	0	0	0	2	0	0	0	0	3	47	0	0	0	0	44	12	0	2	110	
5:50 PM	0	0	0	0	0	1	0	0	0	3	1	23	0	0	0	0	50	14	0	0	92	
5:55 PM	0	0	0	0	0	5	0	1	0	0	1	37	0	0	0	0	48	6	0	3	101	
Peak 15-Min Flowrates	Northbound					Southbound					Eastbound					Westbound					Total	
	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*		
All Vehicles	0	0	0	0	0	48	0	0	0	4	40	580	0	4	0	0	824	204	0	32	1736	
Heavy Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	4	4	0	8	8	
Pedestrians	0	0	0	0	0	0	12	0	0	0	0	4	0	0	0	0	0	0	0	0	20	
Bicycles	0	0	0	0	0	0	0	0	0	0	2	4	0	0	0	0	2	0	0	0	8	
Railroad	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Stopped Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	

Comments: N/A

ITM Peak Hour Summary

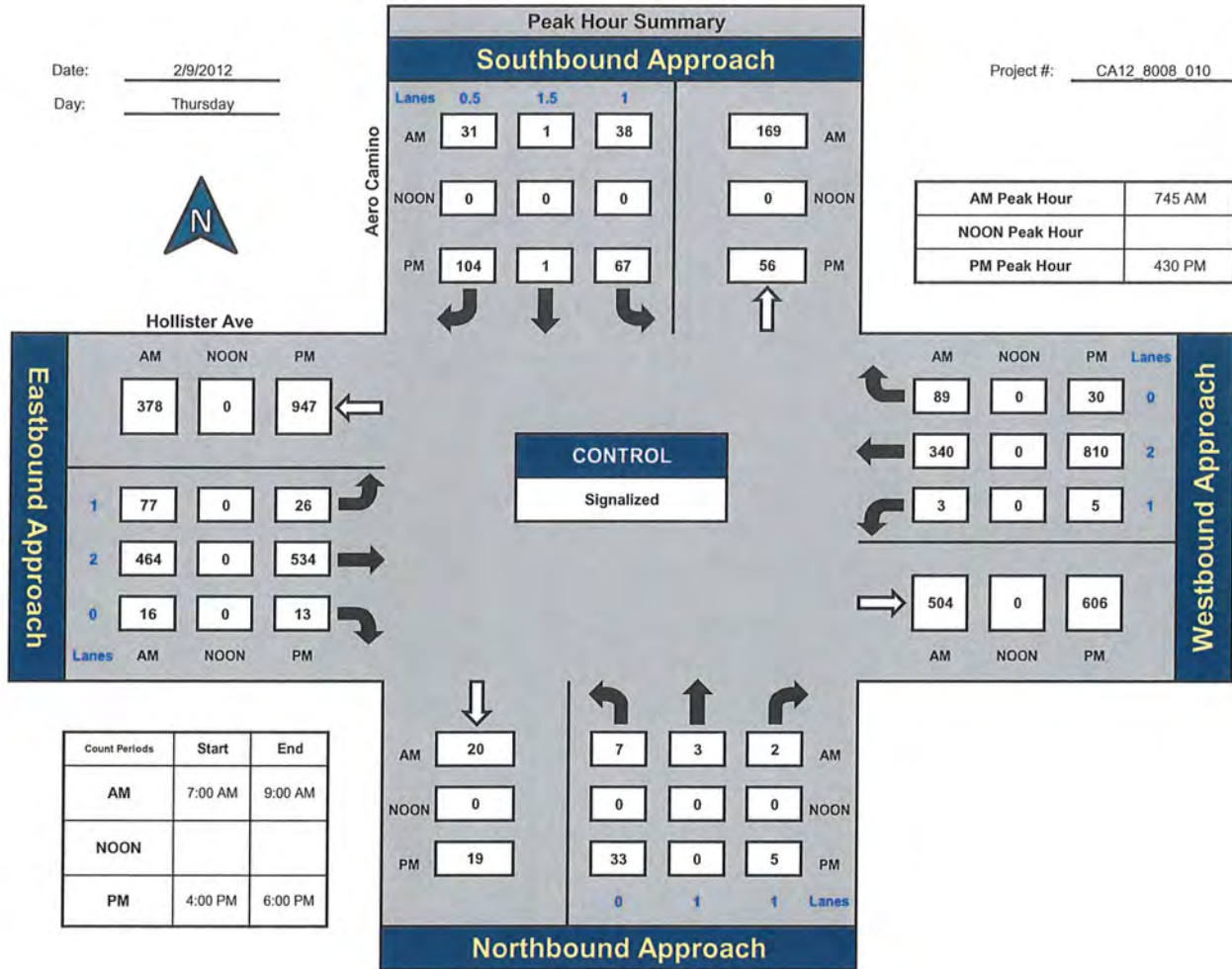


Prepared by:
National Data & Surveying Services

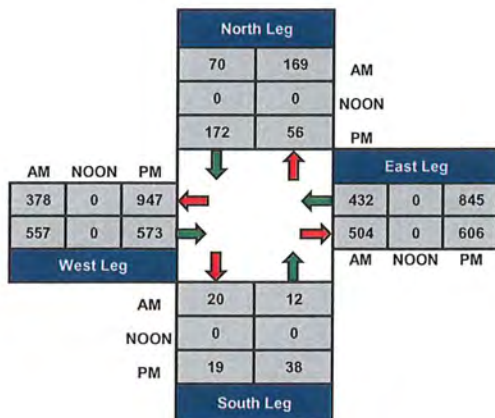
Aero Camino and Hollister Ave, City of Goleta

Date: 2/9/2012
Day: Thursday

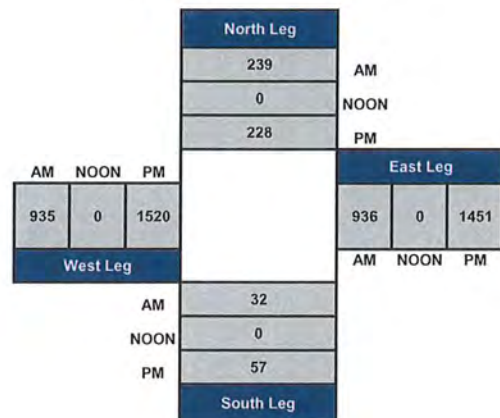
Project #: CA12_8008_010



Total Ins & Outs



Total Volume Per Leg



ITM Peak Hour Summary



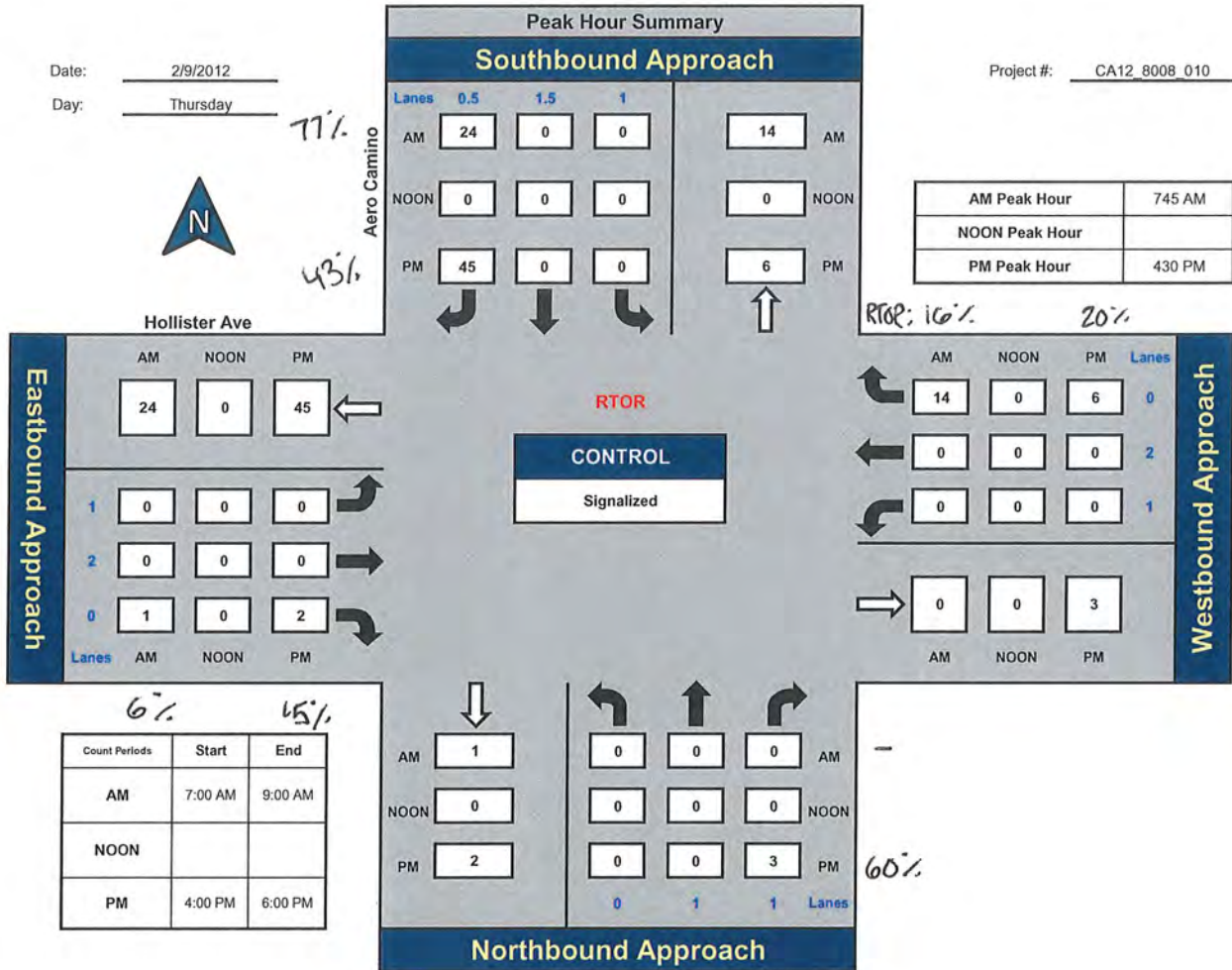
Prepared by:
National Data & Surveying Services

Aero Camino and Hollister Ave, City of Goleta

Date: 2/9/2012

Day: Thursday

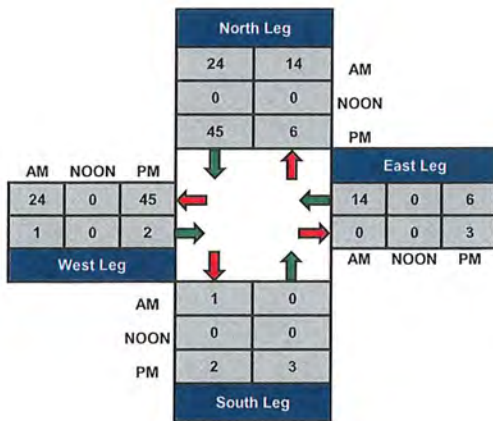
Project #: CA12_8008_010



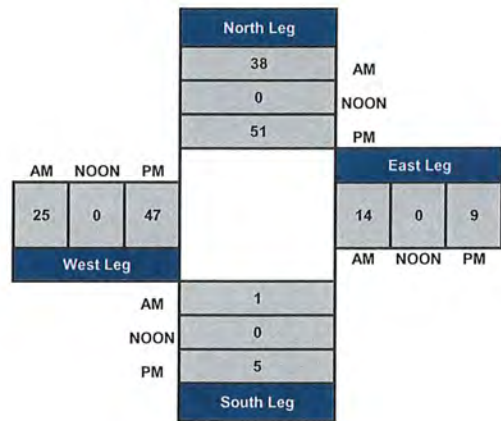
AM Peak Hour	745 AM
NOON Peak Hour	
PM Peak Hour	430 PM

Count Periods	Start	End
AM	7:00 AM	9:00 AM
NOON		
PM	4:00 PM	6:00 PM

Total Ins & Outs



Total Volume Per Leg



INTERSECTION LEVEL OF SERVICE CALCULATION WORKSHEETS

- Reference 1 Los Carneros Road/Calle Real**
- Reference 2 U.S. 101 NB Ramps/Los Carneros Road**
- Reference 3 U.S. 101 SB Ramps/Los Carneros Road**
- Reference 4 Los Carneros Road/Calle Koral**
- Reference 5 Calle Koral/Los Carneros Way**
- Reference 6 Hollister Avenue/Los Carneros Road**
- Reference 7 Hollister Avenue/Los Carneros Way**
- Reference 8 Hollister Avenue/Aero Camino**

ROUNDBOUT REPORT

General Information

Analyst *MMF*
 Agency or Co. *ATE*
 Date Performed *04/04/2013*
 Time Period *AM PEAK*
 Peak Hour Factor *1.00*

Site Information

Intersection *01_EX_AM*
 E/W Street Name *CALLE REAL*
 N/S Street Name
 Analysis Year *EXISTING*
 Project ID *#12089.01 HERITAGE RIDGE PROJECT*

Project Description:

Volume Adjustment and Site Characteristics

	EB				WB				NB				SB			
	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R	U
Number of Lanes (N)	0	0	0		0	0	0		0	1	0		0	1	0	
Lane Assignment					LR				TR				LT			
Right-Turn Bypass	None				None				None				None			
Conflicting Lanes	1				1				1				1			
Volume (V), veh/h	0	0	0	0	241	0	14	0	0	118	162	0	20	197	0	0
Heavy Veh. Adj. (f _{HV}), %	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4
Pedestrians Crossing	0				0				0				0			

Critical and Follow-Up Headway Adjustment

	EB			WB			NB			SB		
	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass
Critical Headway (sec)	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929
Follow-Up Headway (sec)	3.1858	3.1858		3.1858	3.1858		3.1858	3.1858		3.1858	3.1858	

Flow Computations

	EB			WB			NB			SB		
	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass
Circulating Flow (V _c), pc/h	477			123			21			251		
Exiting Flow (V _{ex}), pc/h	189			0			137			456		
Entry Flow (V _e), pc/h		217			265			291			226	
Entry Volume veh/h					255			280			217	

Capacity and v/c Ratios

	EB			WB			NB			SB		
	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass
Capacity (c _{PCE}), pc/h		0			1000			1107			879	
Capacity (c), veh/h		0			961			1064			846	
v/c Ratio (X)					0.27			0.26			0.26	

Delay and Level of Service

	EB			WB			NB			SB		
	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass
Lane Control Delay (d), s/veh					6.4			5.9			7.0	
Lane LOS		F			A			A			A	
Lane 95% Queue					1.1			1.1			1.0	
Approach Delay, s/veh				6.42			5.90			7.00		
Approach LOS, s/veh				A			A			A		
Intersection Delay, s/veh	6.39											
Intersection LOS	A											

ROUNDBABOUT REPORT

General Information

Analyst *MMF*
 Agency or Co. *ATE*
 Date Performed *04/04/2013*
 Time Period *AM PEAK*
 Peak Hour Factor *1.00*

Site Information

Intersection *01_EX+PR_AM*
 E/W Street Name *CALLE REAL*
 N/S Street Name
 Analysis Year *EXISTING+PROJECT*
 Project ID *#12089.01 HERITAGE RIDGE PROJECT*

Project Description:

Volume Adjustment and Site Characteristics

	EB				WB				NB				SB			
	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R	U
Number of Lanes (N)	0	0	0		0	0	0		0	1	0		0	1	0	
Lane Assignment					LR				TR				LT			
Right-Turn Bypass	None				None				None				None			
Conflicting Lanes	1				1				1				1			
Volume (V), veh/h	0	0	0	0	243	0	14	0	0	125	169	0	20	199	0	0
Heavy Veh. Adj. (f _{HV}), %	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4
Pedestrians Crossing	0				0				0				0			

Critical and Follow-Up Headway Adjustment

	EB			WB			NB			SB		
	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass
Critical Headway (sec)	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929
Follow-Up Headway (sec)	3.1858	3.1858		3.1858	3.1858		3.1858	3.1858		3.1858	3.1858	

Flow Computations

	EB			WB			NB			SB		
	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass
Circulating Flow (V _c), pc/h	481			130			21			253		
Exiting Flow (V _{ex}), pc/h	197			0			145			460		
Entry Flow (V _e), pc/h		219			267			306			228	
Entry Volume veh/h					257			294			219	

Capacity and v/c Ratios

	EB			WB			NB			SB		
	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass
Capacity (c _{PCE}), pc/h		0			992			1107			878	
Capacity (c), veh/h		0			954			1064			844	
v/c Ratio (X)					0.27			0.28			0.26	

Delay and Level of Service

	EB			WB			NB			SB		
	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass
Lane Control Delay (d), s/veh					6.5			6.1			7.1	
Lane LOS		F			A			A			A	
Lane 95% Queue					1.1			1.1			1.0	
Approach Delay, s/veh				6.50			6.05			7.05		
Approach LOS, s/veh				A			A			A		
Intersection Delay, s/veh	6.49											
Intersection LOS	A											

ROUNDBOUT REPORT

General Information

Analyst *MMF*
 Agency or Co. *ATE*
 Date Performed *04/04/2013*
 Time Period *PM PEAK*
 Peak Hour Factor *1.00*

Site Information

Intersection *01_EX_PM*
 E/W Street Name *CALLE REAL*
 N/S Street Name
 Analysis Year *EXISTING*
 Project ID *#12089.01 HERITAGE RIDGE PROJECT*

Project Description:

Volume Adjustment and Site Characteristics

	EB				WB				NB				SB			
	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R	U
Number of Lanes (N)	0	0	0		0	0	0		0	1	0		0	1	0	
Lane Assignment					<i>LR</i>				<i>TR</i>				<i>LT</i>			
Right-Turn Bypass	<i>None</i>				<i>None</i>				<i>None</i>				<i>None</i>			
Conflicting Lanes	<i>1</i>				<i>1</i>				<i>1</i>				<i>1</i>			
Volume (V), veh/h	0	0	0	0	319	0	24	0	0	209	396	0	12	86	0	0
Heavy Veh. Adj. (f_{HV}), %	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4
Pedestrians Crossing	<i>0</i>				<i>0</i>				<i>0</i>				<i>0</i>			

Critical and Follow-Up Headway Adjustment

	EB			WB			NB			SB		
	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass
Critical Headway (sec)	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929
Follow-Up Headway (sec)	3.1858	3.1858		3.1858	3.1858		3.1858	3.1858		3.1858	3.1858	

Flow Computations

	EB			WB			NB			SB		
	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass
Circulating Flow (V_c), pc/h	433			217			12			332		
Exiting Flow (V_{ex}), pc/h	424			0			242			421		
Entry Flow (V_e), pc/h		98			357			629			102	
Entry Volume veh/h					343			605			98	

Capacity and v/c Ratios

	EB			WB			NB			SB		
	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass
Capacity (c_{PCE}), pc/h		0			909			1116			811	
Capacity (c), veh/h		0			874			1073			780	
v/c Ratio (X)					0.39			0.56			0.13	

Delay and Level of Service

	EB			WB			NB			SB		
	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass
Lane Control Delay (d), s/veh					8.7			10.4			5.9	
Lane LOS		<i>F</i>			<i>A</i>			<i>B</i>			<i>A</i>	
Lane 95% Queue					1.9			3.6			0.4	
Approach Delay, s/veh				8.72			10.42			5.91		
Approach LOS, s/veh				<i>A</i>			<i>B</i>			<i>A</i>		
Intersection Delay, s/veh	9.44											
Intersection LOS	<i>A</i>											

ROUNDBOUT REPORT

General Information

Analyst *MMF*
 Agency or Co. *ATE*
 Date Performed *04/04/2013*
 Time Period *PM PEAK*
 Peak Hour Factor *1.00*

Site Information

Intersection *01_EX+PR_PM*
 E/W Street Name *CALLE REAL*
 N/S Street Name
 Analysis Year *EXISTING+PROJECT*
 Project ID *#12089.01 HERITAGE RIDGE PROJECT*

Project Description:

Volume Adjustment and Site Characteristics

	EB				WB				NB				SB			
	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R	U
Number of Lanes (N)	0	0	0		0	0	0		0	1	0		0	1	0	
Lane Assignment					LR				TR				LT			
Right-Turn Bypass	None				None				None				None			
Conflicting Lanes	1				1				1				1			
Volume (V), veh/h	0	0	0	0	325	0	24	0	0	212	399	0	12	92	0	0
Heavy Veh. Adj. (f _{HV}), %	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4
Pedestrians Crossing	0				0				0				0			

Critical and Follow-Up Headway Adjustment

	EB			WB			NB			SB		
	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass
Critical Headway (sec)	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929
Follow-Up Headway (sec)	3.1858	3.1858		3.1858	3.1858		3.1858	3.1858		3.1858	3.1858	

Flow Computations

	EB			WB			NB			SB		
	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass
Circulating Flow (V _c), pc/h	446			220			12			338		
Exiting Flow (V _{ex}), pc/h	427			0			245			434		
Entry Flow (V _e), pc/h		104			363			635			108	
Entry Volume veh/h					349			611			104	

Capacity and v/c Ratios

	EB			WB			NB			SB		
	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass
Capacity (c _{PCE}), pc/h		0			906			1116			806	
Capacity (c), veh/h		0			872			1073			775	
v/c Ratio (X)					0.40			0.57			0.13	

Delay and Level of Service

	EB			WB			NB			SB		
	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass
Lane Control Delay (d), s/veh					8.9			10.5			6.0	
Lane LOS		F			A			B			A	
Lane 95% Queue					1.9			3.7			0.5	
Approach Delay, s/veh				8.86			10.53			6.03		
Approach LOS, s/veh				A			B			A		
Intersection Delay, s/veh	9.54											
Intersection LOS	A											

ROUNDBOUT REPORT

General Information

Analyst *MMF*
 Agency or Co. *ATE*
 Date Performed *04/04/2013*
 Time Period *AM PEAK*
 Peak Hour Factor *1.00*

Site Information

Intersection *01_CU_AM*
 E/W Street Name *CALLE REAL*
 N/S Street Name *LOS CARNEROS*
 Analysis Year *CUMULATIVE*
 Project ID *#12089.01 HERITAGE RIDGE PROJECT*

Project Description:

Volume Adjustment and Site Characteristics

	EB				WB				NB				SB			
	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R	U
Number of Lanes (N)	0	0	0		0	0	0		0	1	0		0	1	0	
Lane Assignment					LR				TR				LT			
Right-Turn Bypass	None				None				None				None			
Conflicting Lanes	1				1				1				1			
Volume (V), veh/h	0	0	0	0	274	0	14	0	0	143	217	0	25	245	0	0
Heavy Veh. Adj. (f _{HV}), %	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4
Pedestrians Crossing	0				0				0				0			

Critical and Follow-Up Headway Adjustment

	EB			WB			NB			SB		
	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass
Critical Headway (sec)	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929
Follow-Up Headway (sec)	3.1858	3.1858		3.1858	3.1858		3.1858	3.1858		3.1858	3.1858	

Flow Computations

	EB			WB			NB			SB		
	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass
Circulating Flow (V _c), pc/h	566			149			26			285		
Exiting Flow (V _{ex}), pc/h	252			0			163			540		
Entry Flow (V _e), pc/h		270		300				374			281	
Entry Volume veh/h				288				360			270	

Capacity and v/c Ratios

	EB			WB			NB			SB		
	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass
Capacity (c _{PCE}), pc/h		0		974			1101			850		
Capacity (c), veh/h		0		936			1059			817		
v/c Ratio (X)				0.31			0.34			0.33		

Delay and Level of Service

	EB			WB			NB			SB		
	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass
Lane Control Delay (d), s/veh					7.1			6.8			8.2	
Lane LOS		F			A			A			A	
Lane 95% Queue					1.3			1.5			1.5	
Approach Delay, s/veh				7.09			6.84			8.22		
Approach LOS, s/veh				A			A			A		
Intersection Delay, s/veh	7.32											
Intersection LOS	A											

ROUNDBOUT REPORT

General Information

Analyst *MMF*
 Agency or Co. *ATE*
 Date Performed *04/04/2013*
 Time Period *AM PEAK*
 Peak Hour Factor *1.00*

Site Information

Intersection *01_CU+PR_AM*
 E/W Street Name *CALLE REAL*
 N/S Street Name
 Analysis Year *CUMULATIVE+PROJECT*
 Project ID *#12089.01 HERITAGE RIDGE PROJECT*

Project Description:

Volume Adjustment and Site Characteristics

	EB				WB				NB				SB			
	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R	U
Number of Lanes (N)	0	0	0		0	0	0		0	1	0		0	1	0	
Lane Assignment					LR				TR				LT			
Right-Turn Bypass	None				None				None				None			
Conflicting Lanes	1				1				1				1			
Volume (V), veh/h	0	0	0	0	276	0	14	0	0	152	228	0	25	247	0	0
Heavy Veh. Adj. (f _{HV}), %	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4
Pedestrians Crossing	0				0				0				0			

Critical and Follow-Up Headway Adjustment

	EB			WB			NB			SB		
	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass
Critical Headway (sec)	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929
Follow-Up Headway (sec)	3.1858	3.1858		3.1858	3.1858		3.1858	3.1858		3.1858	3.1858	

Flow Computations

	EB			WB			NB			SB		
	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass
Circulating Flow (V _c), pc/h	570			158			26			287		
Exiting Flow (V _{ex}), pc/h	263			0			173			544		
Entry Flow (V _e), pc/h		272			302			395			283	
Entry Volume veh/h					290			380			272	

Capacity and v/c Ratios

	EB			WB			NB			SB		
	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass
Capacity (c _{PCE}), pc/h		0			965			1101			848	
Capacity (c), veh/h		0			928			1059			815	
v/c Ratio (X)					0.31			0.36			0.33	

Delay and Level of Service

	EB			WB			NB			SB		
	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass
Lane Control Delay (d), s/veh					7.2			7.1			8.3	
Lane LOS		F			A			A			A	
Lane 95% Queue					1.3			1.6			1.5	
Approach Delay, s/veh				7.20			7.08			8.28		
Approach LOS, s/veh				A			A			A		
Intersection Delay, s/veh	7.47											
Intersection LOS	A											

ROUNDBOUT REPORT

General Information

Analyst *MMF*
 Agency or Co. *ATE*
 Date Performed *04/04/2013*
 Time Period *PM PEAK*
 Peak Hour Factor *1.00*

Site Information

Intersection *01_CU_PM*
 E/W Street Name *CALLE REAL*
 N/S Street Name *LOS CARNEROS*
 Analysis Year *CUMULATIVE*
 Project ID *#12089.01 HERITAGE RIDGE PROJECT*

Project Description:

Volume Adjustment and Site Characteristics

	EB				WB				NB				SB			
	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R	U
Number of Lanes (N)	0	0	0		0	0	0		0	1	0		0	1	0	
Lane Assignment					LR				TR				LT			
Right-Turn Bypass	None				None				None				None			
Conflicting Lanes	1				1				1				1			
Volume (V), veh/h	0	0	0	0	424	0	27	0	0	260	435	0	13	140	0	0
Heavy Veh. Adj. (f _{HV}), %	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4
Pedestrians Crossing	0				0				0				0			

Critical and Follow-Up Headway Adjustment

	EB			WB			NB			SB		
	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass
Critical Headway (sec)	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929
Follow-Up Headway (sec)	3.1858	3.1858		3.1858	3.1858		3.1858	3.1858		3.1858	3.1858	

Flow Computations

	EB			WB			NB			SB		
	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass
Circulating Flow (V _c), pc/h	601			270			14			441		
Exiting Flow (V _{ex}), pc/h	466			0			298			587		
Entry Flow (V _e), pc/h		144		469			723			159		
Entry Volume veh/h				451			695			153		

Capacity and v/c Ratios

	EB			WB			NB			SB		
	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass
Capacity (c _{PCE}), pc/h		0		862			1115			727		
Capacity (c), veh/h		0		829			1072			699		
v/c Ratio (X)				0.54			0.65			0.22		

Delay and Level of Service

	EB			WB			NB			SB		
	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass
Lane Control Delay (d), s/veh					12.1			12.6			7.7	
Lane LOS		F			B			B			A	
Lane 95% Queue					3.3			5.0			0.8	
Approach Delay, s/veh				12.12			12.57			7.68		
Approach LOS, s/veh				B			B			A		
Intersection Delay, s/veh	11.84											
Intersection LOS	B											

ROUNDBOUT REPORT

General Information

Analyst *MMF*
 Agency or Co. *ATE*
 Date Performed *04/04/2013*
 Time Period *PM PEAK*
 Peak Hour Factor *1.00*

Site Information

Intersection *01_CU+PR_PM*
 E/W Street Name *CALLE REAL*
 N/S Street Name *LOS CARNEROS*
 Analysis Year *CUMULATIVE+PROJECT*
 Project ID *#12089.01 HERITAGE RIDGE PROJECT*

Project Description:

Volume Adjustment and Site Characteristics

	EB				WB				NB				SB			
	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R	U
Number of Lanes (N)	0	0	0		0	0	0		0	1	0		0	1	0	
Lane Assignment					LR				TR				LT			
Right-Turn Bypass	None				None				None				None			
Conflicting Lanes	1				1				1				1			
Volume (V), veh/h	0	0	0	0	430	0	27	0	0	263	442	0	13	146	0	0
Heavy Veh. Adj. (f _{HV}), %	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4
Pedestrians Crossing	0				0				0				0			

Critical and Follow-Up Headway Adjustment

	EB			WB			NB			SB		
	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass
Critical Headway (sec)	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929	5.1929
Follow-Up Headway (sec)	3.1858	3.1858		3.1858	3.1858		3.1858	3.1858		3.1858	3.1858	

Flow Computations

	EB			WB			NB			SB		
	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass
Circulating Flow (V _c), pc/h	613			274			14			447		
Exiting Flow (V _{ex}), pc/h	473			0			302			599		
Entry Flow (V _e), pc/h		153			475			733			165	
Entry Volume veh/h					457			705			159	

Capacity and v/c Ratios

	EB			WB			NB			SB		
	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass
Capacity (c _{PCE}), pc/h		0			860			1115			723	
Capacity (c), veh/h		0			827			1072			695	
v/c Ratio (X)					0.55			0.66			0.23	

Delay and Level of Service

	EB			WB			NB			SB		
	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass
Lane Control Delay (d), s/veh					12.3			12.8			7.8	
Lane LOS		F			B			B			A	
Lane 95% Queue					3.4			5.2			0.9	
Approach Delay, s/veh				12.35			12.84			7.85		
Approach LOS, s/veh				B			B			A		
Intersection Delay, s/veh	12.07											
Intersection LOS	B											

#12089.01 HERITAGE RIDGE PROJECT

REF: 02 AM

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE: 05/22/2013
 TIME PERIOD: A.M. PEAK HOUR
 N/S STREET: LOS CARNEROS ROAD
 E/W STREET: U.S. 101 NB RAMPS
 CONTROL TYPE: SIGNAL

TRAFFIC VOLUME SUMMARY

VOLUMES	NORTH BOUND			SOUTH BOUND			EAST BOUND			WEST BOUND		
	L	T	R	L	T	R	L	T	R	L	T	R
(A) EXISTING:	21	276	0	0	423	110	0	0	0	805	2	53
(B) PROJECT-ADDED:	7	14	0	0	4	0	0	0	0	13	0	0
(C) CUMULATIVE:	34	313	0	0	412	107	0	0	0	1233	2	53

GEOMETRICS

LANE GEOMETRICS	NORTH BOUND		SOUTH BOUND		EAST BOUND		WEST BOUND	
	L	TT	T	TR	L	TR	L	LTR

TRAFFIC SCENARIOS

SCENARIO 1 = EXISTING VOLUMES (A)
 SCENARIO 2 = EXISTING + PROJECT VOLUMES (A+B)
 SCENARIO 3 = CUMULATIVE (C)
 SCENARIO 4 = CUMULATIVE + PROJECT VOLUMES (B+C)

LEVEL OF SERVICE CALCULATIONS

MOVE- MENTS	# OF LANES	CAPACITY	SCENARIO VOLUMES				SCENARIO V/C RATIOS					
			1	2	3	4	1	2	3	4		
NBL	1	1600	21	28	34	41	0.013 *	0.018 *	0.021 *	0.026 *		
NBT	2	3200	276	290	313	327	0.086	0.091	0.098	0.102		
NBR (a)	0	0	0	0	0	0	-	-	-	-		
SBL	0	0	0	0	0	0	-	-	-	-		
SBT	2	3200	423	427	412	416	0.165 *	0.167 *	0.161 *	0.162 *		
SBR (b)	0	0	106	106	103	103	-	-	-	-		
EBL	0	0	0	0	0	0	-	-	-	-		
EBT	0	0	0	0	0	0	-	-	-	-		
EBR (c)	0	0	0	0	0	0	-	-	-	-		
WBL	0	0	805	818	1233	1246	-	-	-	-		
WBT	2	3200	2	2	2	2	0.265 *	0.269 *	0.398 *	0.403 *		
WBR (d)	0	0	40	40	40	40	-	-	-	-		
LOST TIME:							0.100 *	0.100 *	0.100 *	0.100 *		
TOTAL INTERSECTION CAPACITY UTILIZATION:							0.543	0.554	0.680	0.691		
SCENARIO LEVEL OF SERVICE:							A	A	B	B		

NOTES:

RTOR: (a) 0%
 (b) 4%
 (c) 0%
 (d) 25%

Printed: 10/12/15

#12089.01 HERITAGE RIDGE PROJECT

REF: 02 PM

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE: 02/09/2012

TIME PERIOD: P.M. PEAK HOUR

N/S STREET: LOS CARNEROS ROAD

E/W STREET: U.S. 101 NB RAMPS

CONTROL TYPE: SIGNAL

TRAFFIC VOLUME SUMMARY

VOLUMES	NORTH BOUND			SOUTH BOUND			EAST BOUND			WEST BOUND		
	L	T	R	L	T	R	L	T	R	L	T	R
(A) EXISTING:	236	618	0	0	250	107	0	0	0	436	7	24
(B) PROJECT-ADDED:	3	6	0	0	12	0	0	0	0	49	0	0
(C) CUMULATIVE:	236	667	0	0	436	128	0	0	0	493	11	33

GEOMETRICS

LANE GEOMETRICS	NORTH BOUND		SOUTH BOUND		EAST BOUND		WEST BOUND	
	L	TT	T	TR	L	TR	L	LTR

TRAFFIC SCENARIOS

- SCENARIO 1 = EXISTING VOLUMES (A)
- SCENARIO 2 = EXISTING + PROJECT VOLUMES (A+B)
- SCENARIO 3 = CUMULATIVE (C)
- SCENARIO 4 = CUMULATIVE + PROJECT VOLUMES (B+C)

LEVEL OF SERVICE CALCULATIONS

MOVE- MENTS	# OF LANES	CAPACITY	SCENARIO VOLUMES				SCENARIO V/C RATIOS					
			1	2	3	4	1	2	3	4		
NBL	1	1600	236	239	236	239	0.148 *	0.149 *	0.148 *	0.149 *		
NBT	2	3200	618	624	667	673	0.193	0.195	0.208	0.210		
NBR (a)	0	0	0	0	0	0	-	-	-	-		
SBL	0	0	0	0	0	0	-	-	-	-		
SBT	2	3200	250	262	436	448	0.100 *	0.104 *	0.163 *	0.166 *		
SBR (b)	0	0	71	71	84	84	-	-	-	-		
EBL	0	0	0	0	0	0	-	-	-	-		
EBT	0	0	0	0	0	0	-	-	-	-		
EBR (c)	0	0	0	0	0	0	-	-	-	-		
WBL	0	0	436	485	493	542	-	-	-	-		
WBT	2	3200	7	7	11	11	0.141 *	0.157 *	0.161 *	0.177 *		
WBR (d)	0	0	9	9	12	12	-	-	-	-		
LOST TIME:							0.100 *	0.100 *	0.100 *	0.100 *		
TOTAL INTERSECTION CAPACITY UTILIZATION:							0.489	0.510	0.572	0.592		
SCENARIO LEVEL OF SERVICE:							A	A	A	A		

NOTES:

- RTOR: (a)
- (b) 34%
- (c)
- (d) 63%

Printed: 10/12/15

#12089.01 HERITAGE RIDGE PROJECT

REF: 03AM

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE: 05/22/2013
 TIME PERIOD: A.M. PEAK HOUR
 N/S STREET: LOS CARNEROS ROAD
 E/W STREET: U.S. 101 SB RAMPS
 CONTROL TYPE: SIGNAL

ASSUMES COMPLETION OF LOS CARNEROS BRIDGE PROJECT

TRAFFIC VOLUME SUMMARY

VOLUMES	NORTH BOUND			SOUTH BOUND			EAST BOUND			WEST BOUND		
	L	T	R	L	T	R	L	T	R	L	T	R
(A) EXISTING:	0	160	357	72	1168	0	134	2	216	0	0	0
(B) PROJECT-ADDED:	0	21	56	0	17	0	0	0	2	0	0	0
(C) CUMULATIVE:	0	213	475	83	1562	0	134	2	245	0	0	0

GEOMETRICS

LANE GEOMETRICS	NORTH BOUND		SOUTH BOUND		EAST BOUND		WEST BOUND	
	TT	R	L	TT	LT	R	LT	R

TRAFFIC SCENARIOS

SCENARIO 1 = EXISTING VOLUMES (A)
 SCENARIO 2 = EXISTING + PROJECT VOLUMES (A+B)
 SCENARIO 3 = CUMULATIVE (C)
 SCENARIO 4 = CUMULATIVE + PROJECT VOLUMES (B+C)

LEVEL OF SERVICE CALCULATIONS

MOVE-MENTS	# OF LANES	CAPACITY	SCENARIO VOLUMES				SCENARIO V/C RATIOS					
			1	2	3	4	1	2	3	4		
NBL	0	0	0	0	0	0	-	-	-	-		
NBT	2	3200	160	181	213	234	0.050	0.057	0.067	0.073		
NBR (a)	1	1600	286	330	380	425	0.179	0.206	0.238	0.266		
SBL	1	1600	72	72	83	83	0.045	0.045	0.052	0.052		
SBT	2	3200	1168	1185	1562	1579	0.365 *	0.370 *	0.488 *	0.493 *		
SBR (b)	0	0	0	0	0	0	-	-	-	-		
EBL	0	0	134	134	134	134	-	-	-	-		
EBT	1	1600	2	2	2	2	0.085 *	0.085 *	0.085 *	0.085 *		
EBR (c)	1	1600	50	50	56	57	0.031	0.031	0.035	0.036		
WBL	0	0	0	0	0	0	-	-	-	-		
WBT	0	0	0	0	0	0	-	-	-	-		
WBR (d)	0	0	0	0	0	0	-	-	-	-		
LOST TIME:							0.100 *	0.100 *	0.100 *	0.100 *		
TOTAL INTERSECTION CAPACITY UTILIZATION:							0.550	0.555	0.673	0.678		
SCENARIO LEVEL OF SERVICE:							A	A	B	B		

NOTES:

RTOR: (a) 20%
 (b)
 (c) 77%
 (d)

Printed: 10/12/15

#12089.01 HERITAGE RIDGE PROJECT

REF: 03PM

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE: 02/09/2012

TIME PERIOD: P.M. PEAK HOUR

N/S STREET: LOS CARNEROS ROAD

ASSUMES COMPLETION OF LOS CARNEROS BRIDGE PROJECT

E/W STREET: U.S. 101 SB RAMPS

CONTROL TYPE: SIGNAL

TRAFFIC VOLUME SUMMARY

VOLUMES	NORTH BOUND			SOUTH BOUND			EAST BOUND			WEST BOUND		
	L	T	R	L	T	R	L	T	R	L	T	R
(A) EXISTING:	0	773	960	43	601	0	97	1	46	0	0	0
(B) PROJECT-ADDED:	0	9	24	0	61	0	0	0	6	0	0	0
(C) CUMULATIVE:	0	790	1157	48	881	0	113	1	75	0	0	0

GEOMETRICS

LANE GEOMETRICS	NORTH BOUND		SOUTH BOUND		EAST BOUND		WEST BOUND	
	TT	R	L	TT	LT	R	LT	R

TRAFFIC SCENARIOS

SCENARIO 1 = EXISTING VOLUMES (A)
 SCENARIO 2 = EXISTING + PROJECT VOLUMES (A+B)
 SCENARIO 3 = CUMULATIVE (C)
 SCENARIO 4 = CUMULATIVE + PROJECT VOLUMES (B+C)

LEVEL OF SERVICE CALCULATIONS

MOVE- MENTS	# OF LANES	CAPACITY	SCENARIO VOLUMES				SCENARIO V/C RATIOS					
			1	2	3	4	1	2	3	4		
NBL	0	0	0	0	0	0	-	-	-	-		
NBT	2	3200	773	782	790	799	0.242	0.244	0.247	0.250		
NBR (a)	1	1600	845	866	1018	1039	0.528 *	0.541 *	0.636 *	0.649 *		
SBL	1	1600	43	43	48	48	0.027 *	0.027 *	0.030 *	0.030 *		
SBT	2	3200	601	662	881	942	0.188	0.207	0.275	0.294		
SBR (b)	0	0	0	0	0	0	-	-	-	-		
EBL	0	0	97	97	113	113	-	-	-	-		
EBT	1	1600	1	1	1	1	0.061 *	0.061 *	0.071 *	0.071 *		
EBR (c)	1	1600	9	10	15	16	0.006	0.006	0.009	0.010		
WBL	0	0	0	0	0	0	-	-	-	-		
WBT	0	0	0	0	0	0	-	-	-	-		
WBR (d)	0	0	0	0	0	0	-	-	-	-		
LOST TIME:							0.100 *	0.100 *	0.100 *	0.100 *		
TOTAL INTERSECTION CAPACITY UTILIZATION:							0.716	0.729	0.837	0.850		
SCENARIO LEVEL OF SERVICE:							C	C	D	D		

NOTES:

RTOR: (a) 12%
 (b)
 (c) 80%
 (d)

Printed: 10/12/15

#12089.01 HERITAGE RIDGE PROJECT

REF: 03PM_MIT

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE: 02/09/2012

TIME PERIOD: P.M. PEAK HOUR

N/S STREET: LOS CARNEROS ROAD

ASSUMES FREE NB RIGHT-TURN LANE

E/W STREET: U.S. 101 SB RAMPS

CONTROL TYPE: SIGNAL

TRAFFIC VOLUME SUMMARY

VOLUMES	NORTH BOUND			SOUTH BOUND			EAST BOUND			WEST BOUND		
	L	T	R	L	T	R	L	T	R	L	T	R
(A) EXISTING:	0	773	960	43	601	0	97	1	46	0	0	0
(B) PROJECT-ADDED:	0	9	24	0	61	0	0	0	6	0	0	0
(C) CUMULATIVE:	0	790	1157	48	881	0	113	1	75	0	0	0

GEOMETRICS

LANE GEOMETRICS	NORTH BOUND		SOUTH BOUND		EAST BOUND		WEST BOUND	
	TT	R	L	TT	LT	R	LT	R

TRAFFIC SCENARIOS

- SCENARIO 1 = EXISTING VOLUMES (A)
- SCENARIO 2 = EXISTING + PROJECT VOLUMES (A+B)
- SCENARIO 3 = CUMULATIVE (C)
- SCENARIO 4 = CUMULATIVE + PROJECT VOLUMES (B+C)

LEVEL OF SERVICE CALCULATIONS

MOVE-MENTS	# OF LANES	CAPACITY	SCENARIO VOLUMES				SCENARIO V/C RATIOS					
			1	2	3	4	1	2	3	4		
NBL	0	0	0	0	0	0	-	-	-	-		
NBT	2	3200	773	782	790	799	0.242 *	0.244 *	0.247 *	0.250 *		
NBR (a)	1	1600	0	0	0	0	0.000	0.000	0.000	0.000		
SBL	1	1600	43	43	48	48	0.027 *	0.027 *	0.030 *	0.030 *		
SBT	2	3200	601	662	881	942	0.188	0.207	0.275	0.294		
SBR (b)	0	0	0	0	0	0	-	-	-	-		
EBL	0	0	97	97	113	113	-	-	-	-		
EBT	1	1600	1	1	1	1	0.061 *	0.061 *	0.071 *	0.071 *		
EBR (c)	1	1600	9	10	15	16	0.006	0.006	0.009	0.010		
WBL	0	0	0	0	0	0	-	-	-	-		
WBT	0	0	0	0	0	0	-	-	-	-		
WBR (d)	0	0	0	0	0	0	-	-	-	-		
LOST TIME:							0.100 *	0.100 *	0.100 *	0.100 *		
TOTAL INTERSECTION CAPACITY UTILIZATION:							0.430	0.432	0.448	0.451		
SCENARIO LEVEL OF SERVICE:							A	A	A	A		

NOTES:

- RTOR: (a) FREE RIGHT-TURN LANE
- (b)
- (c) 80%
- (d)

Printed: 10/12/15

#12089.01 HERITAGE RIDGE PROJECT

REF: 04AM_EX

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE: 05/22/2013
 TIME PERIOD: A.M. PEAK HOUR
 N/S STREET: LOS CARNEROS ROAD
 E/W STREET: CALLE KORAL
 CONTROL TYPE: SIGNAL

TRAFFIC VOLUME SUMMARY

VOLUMES	NORTH BOUND			SOUTH BOUND			EAST BOUND			WEST BOUND		
	L	T	R	L	T	R	L	T	R	L	T	R
(A) EXISTING:	0	399	4	247	1137	0	0	0	0	13	0	120
(B) PROJECT-ADDED:	0	0	5	19	0	0	0	0	0	24	0	77

GEOMETRICS

LANE GEOMETRICS	NORTH BOUND TT TR	SOUTH BOUND L TT	EAST BOUND	WEST BOUND L R
-----------------	----------------------	---------------------	------------	-------------------

TRAFFIC SCENARIOS

SCENARIO 1 = EXISTING VOLUMES (A)
 SCENARIO 2 = EXISTING + PROJECT VOLUMES (A+B)

LEVEL OF SERVICE CALCULATIONS

MOVE- MENTS	# OF LANES	CAPACITY	SCENARIO VOLUMES				SCENARIO V/C RATIOS					
			1	2	3	4	1	2	3	4		
NBL	0	0	0	0	0	0	-	-				
NBT	3	4800	399	399	0	0	0.084	0.085				
NBR (a)	0	0	4	9	0	5	-	-				
SBL	1	1600	247	266	0	19	0.154	0.166				
SBT	2	3200	1137	1137	0	0	0.355 *	0.355 *				
SBR (b)	0	0	0	0	0	0	-	-				
EBL	0	0	0	0	0	0	-	-				
EBT	0	0	0	0	0	0	-	-				
EBR (c)	0	0	0	0	0	0	-	-				
WBL	1	1600	13	37	0	24	0.008	0.023				
WBT	0	0	0	0	0	0	-	-				
WBR (d)	1	1600	4	6	0	2	0.003 *	0.004 *				
LOST TIME:							0.100 *	0.100 *				
TOTAL INTERSECTION CAPACITY UTILIZATION:							0.458	0.459				
SCENARIO LEVEL OF SERVICE:							A	A				

NOTES:

RTOR: (a)
 (b) 0%
 (c)
 (d) 97%

Printed: 10/12/15

#12089.01 HERITAGE RIDGE PROJECT

REF: 04AM_EX+PR

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE: 05/22/2013

TIME PERIOD: A.M. PEAK HOUR

EX+PR LOS WITH WESTERN LEG

N/S STREET: LOS CARNEROS ROAD

E/W STREET: CALLE KORAL

CONTROL TYPE: SIGNAL

TRAFFIC VOLUME SUMMARY

VOLUMES	NORTH BOUND			SOUTH BOUND			EAST BOUND			WEST BOUND		
	L	T	R	L	T	R	L	T	R	L	T	R
(A) EXISTING:	57	399	4	247	1137	94	69	26	69	13	29	120
(B) PROJECT-ADDED:	0	0	5	19	0	0	0	0	0	24	0	77

GEOMETRICS

LANE GEOMETRICS	NORTH BOUND			SOUTH BOUND			EAST BOUND		WEST BOUND	
	L	TT	TR	L	T	TR	L	TR	LT	R

TRAFFIC SCENARIOS

SCENARIO 1 = EXISTING VOLUMES (A)
 SCENARIO 2 = EXISTING + PROJECT VOLUMES (A+B)

LEVEL OF SERVICE CALCULATIONS

MOVE- MENTS	# OF LANES	CAPACITY	SCENARIO VOLUMES				SCENARIO V/C RATIOS					
			1	2	3	4	1	2	3	4		
NBL	1	1600	57	57	0	0	0.036 *	0.036 *				
NBT	3	4800	399	399	0	0	0.084	0.085				
NBR (a)	0	0	4	9	0	5	-	-				
SBL	1	1600	247	266	0	19	0.154	0.166				
SBT	2	3200	1137	1137	0	0	0.385 *	0.385 *				
SBR (b)	0	0	94	94	0	0	-	-				
EBL	1	1600	69	69	0	0	0.043 *	0.043 *				
EBT	1	1600	26	26	0	0	0.059	0.059				
EBR (c)	0	0	69	69	0	0	-	-				
WBL	0	0	13	37	0	24	-	-				
WBT	1	1600	29	29	0	0	0.026 *	0.041 *				
WBR (d)	1	1600	4	6	0	2	0.003	0.004				
LOST TIME:							0.100 *	0.100 *				
TOTAL INTERSECTION CAPACITY UTILIZATION:							0.590	0.605				
SCENARIO LEVEL OF SERVICE:							A	B				

NOTES:

RTOR: (a)
 (b) 0%
 (c)
 (d) 97%

Printed: 10/12/15

#12089.01 HERITAGE RIDGE PROJECT

REF: 04AM_CU

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE: 05/22/2013
 TIME PERIOD: A.M. PEAK HOUR
 N/S STREET: LOS CARNEROS ROAD
 E/W STREET: CALLE KORAL
 CONTROL TYPE: SIGNAL

TRAFFIC VOLUME SUMMARY

VOLUMES	NORTH BOUND			SOUTH BOUND			EAST BOUND			WEST BOUND		
	L	T	R	L	T	R	L	T	R	L	T	R
(A) CUMULATIVE:	57	407	7	279	1434	94	69	26	69	39	29	212
(B) PROJECT-ADDED:	0	0	5	19	0	0	0	0	0	24	0	77

GEOMETRICS

LANE GEOMETRICS	NORTH BOUND			SOUTH BOUND			EAST BOUND			WEST BOUND		
	L	TT	TR	L	T	TR	L	T	TR	L	T	R

TRAFFIC SCENARIOS

SCENARIO 1 = CUMULATIVE VOLUMES (A)
 SCENARIO 2 = CUMULATIVE + PROJECT VOLUMES (A+B)

LEVEL OF SERVICE CALCULATIONS

MOVE- MENTS	# OF LANES	CAPACITY	SCENARIO VOLUMES				SCENARIO V/C RATIOS					
			1	2	3	4	1	2	3	4		
NBL	1	1600	57	57	0	0	0.036 *	0.036 *				
NBT	3	4800	407	407	0	0	0.086	0.087				
NBR (a)	0	0	7	12	0	5	-	-				
SBL	1	1600	279	298	0	19	0.174	0.186				
SBT	2	3200	1434	1434	0	0	0.478 *	0.478 *				
SBR (b)	0	0	94	94	0	0	-	-				
EBL	1	1600	69	69	0	0	0.043 *	0.043 *				
EBT	1	1600	26	26	0	0	0.059	0.059				
EBR (c)	0	0	69	69	0	0	-	-				
WBL	0	0	39	63	0	24	-	-				
WBT	1	1600	29	29	0	0	0.043 *	0.058 *				
WBR (d)	1	1600	6	9	0	2	0.004	0.006				
LOST TIME:							0.100 *	0.100 *				
TOTAL INTERSECTION CAPACITY UTILIZATION:							0.700	0.715				
SCENARIO LEVEL OF SERVICE:							B	C				

NOTES:

- RTOR: (a)
- (b) 0%
- (c)
- (d) 97%

Printed: 10/12/15

#12089.01 HERITAGE RIDGE PROJECT

REF: 04PM_EX

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE: 02/09/2012
 TIME PERIOD: P.M. PEAK HOUR
 N/S STREET: LOS CARNEROS ROAD
 E/W STREET: CALLE KORAL
 CONTROL TYPE: SIGNAL

TRAFFIC VOLUME SUMMARY

VOLUMES	NORTH BOUND			SOUTH BOUND			EAST BOUND			WEST BOUND		
	L	T	R	L	T	R	L	T	R	L	T	R
(A) EXISTING:	0	1394	26	101	577	0	0	0	0	5	0	295
(B) PROJECT-ADDED:	0	0	21	67	0	0	0	0	0	10	0	33

GEOMETRICS

LANE GEOMETRICS	NORTH BOUND		SOUTH BOUND		EAST BOUND		WEST BOUND	
	TT	TR	L	TT	L	TR	L	R

TRAFFIC SCENARIOS

SCENARIO 1 = EXISTING VOLUMES (A)
 SCENARIO 2 = EXISTING + PROJECT VOLUMES (A+B)

LEVEL OF SERVICE CALCULATIONS

MOVE- MENTS	# OF LANES	CAPACITY	SCENARIO VOLUMES				SCENARIO V/C RATIOS					
			1	2	3	4	1	2	3	4		
NBL	0	0	0	0	0	0	-	-				
NBT	3	4800	1394	1394	0	0	0.296 *	0.300 *				
NBR (a)	0	0	26	47	0	21	-	-				
SBL	1	1600	101	168	0	67	0.063 *	0.105 *				
SBT	2	3200	577	577	0	0	0.180	0.180				
SBR (b)	0	0	0	0	0	0	-	-				
EBL	0	0	0	0	0	0	-	-				
EBT	0	0	0	0	0	0	-	-				
EBR (c)	0	0	0	0	0	0	-	-				
WBL	1	1600	5	15	0	10	0.003	0.009				
WBT	0	0	0	0	0	0	-	-				
WBR (d)	1	1600	83	92	0	9	0.052 *	0.058 *				
LOST TIME:							0.100 *	0.100 *				
TOTAL INTERSECTION CAPACITY UTILIZATION:							0.511	0.563				
SCENARIO LEVEL OF SERVICE:							A	A				

NOTES:

RTOR: (a)
 (b) 4%
 (c)
 (d) 72%

Printed: 10/12/15

#12089.01 HERITAGE RIDGE PROJECT

REF: 04PM_EX+PR

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE: 02/09/2012

TIME PERIOD: P.M. PEAK HOUR

EX+PR LOS WITH WESTERN LEG

N/S STREET: LOS CARNEROS ROAD

E/W STREET: CALLE KORAL

CONTROL TYPE: SIGNAL

TRAFFIC VOLUME SUMMARY

VOLUMES	NORTH BOUND			SOUTH BOUND			EAST BOUND			WEST BOUND		
	L	T	R	L	T	R	L	T	R	L	T	R
(A) EXISTING:	63	1394	26	101	577	60	95	11	65	5	32	295
(B) PROJECT-ADDED:	0	0	21	67	0	0	0	0	0	10	0	33

GEOMETRICS

LANE GEOMETRICS	NORTH BOUND			SOUTH BOUND			EAST BOUND			WEST BOUND		
	L	TT	TR	L	T	TR	L	T	TR	L	T	R

TRAFFIC SCENARIOS

SCENARIO 1 = EXISTING VOLUMES (A)
 SCENARIO 2 = EXISTING + PROJECT VOLUMES (A+B)

LEVEL OF SERVICE CALCULATIONS

MOVE- MENTS	# OF LANES	CAPACITY	SCENARIO VOLUMES				SCENARIO V/C RATIOS					
			1	2	3	4	1	2	3	4		
NBL	1	1600	63	63	0	0	0.039	0.039				
NBT	3	4800	1394	1394	0	0	0.296 *	0.300 *				
NBR (a)	0	0	26	47	0	21	-	-				
SBL	1	1600	101	168	0	67	0.063 *	0.105 *				
SBT	2	3200	577	577	0	0	0.198	0.198				
SBR (b)	0	0	58	58	0	0	-	-				
EBL	1	1600	95	95	0	0	0.059 *	0.059 *				
EBT	1	1600	11	11	0	0	0.048	0.048				
EBR (c)	0	0	65	65	0	0	-	-				
WBL	0	0	5	15	0	10	-	-				
WBT	1	1600	32	32	0	0	0.023	0.029				
WBR (d)	1	1600	83	92	0	9	0.052 *	0.058 *				
LOST TIME:							0.100 *	0.100 *				
TOTAL INTERSECTION CAPACITY UTILIZATION:							0.570	0.622				
SCENARIO LEVEL OF SERVICE:							A	B				

NOTES:

- RTOR: (a)
- (b) 4%
- (c)
- (d) 72%

Printed: 10/12/15

#12089.01 HERITAGE RIDGE PROJECT

REF: 04PM_CU

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE: 02/09/2012
 TIME PERIOD: P.M. PEAK HOUR
 N/S STREET: LOS CARNEROS ROAD
 E/W STREET: CALLE KORAL
 CONTROL TYPE: SIGNAL

TRAFFIC VOLUME SUMMARY

VOLUMES	NORTH BOUND			SOUTH BOUND			EAST BOUND			WEST BOUND		
	L	T	R	L	T	R	L	T	R	L	T	R
(A) CUMULATIVE:	63	1489	34	190	706	60	95	11	65	7	32	363
(B) PROJECT-ADDED:	0	0	21	67	0	0	0	0	0	10	0	33

GEOMETRICS

LANE GEOMETRICS	NORTH BOUND			SOUTH BOUND			EAST BOUND			WEST BOUND		
	L	TT	TR	L	T	TR	L	T	TR	L	T	R

TRAFFIC SCENARIOS

SCENARIO 1 = CUMULATIVE VOLUMES (A)
 SCENARIO 2 = CUMULATIVE + PROJECT VOLUMES (A+B)

LEVEL OF SERVICE CALCULATIONS

MOVE-MENTS	# OF LANES	CAPACITY	SCENARIO VOLUMES				SCENARIO V/C RATIOS					
			1	2	3	4	1	2	3	4		
NBL	1	1600	63	63	0	0	0.039	0.039				
NBT	3	4800	1489	1489	0	0	0.317 *	0.322 *				
NBR (a)	0	0	34	55	0	21	-	-				
SBL	1	1600	190	257	0	67	0.119 *	0.161 *				
SBT	2	3200	706	706	0	0	0.239	0.239				
SBR (b)	0	0	58	58	0	0	-	-				
EBL	1	1600	95	95	0	0	0.059 *	0.059 *				
EBT	1	1600	11	11	0	0	0.048	0.048				
EBR (c)	0	0	65	65	0	0	-	-				
WBL	0	0	7	17	0	10	-	-				
WBT	1	1600	32	32	0	0	0.024	0.031				
WBR (d)	1	1600	102	111	0	9	0.064 *	0.069 *				
LOST TIME:							0.100 *	0.100 *				
TOTAL INTERSECTION CAPACITY UTILIZATION:							0.659	0.711				
SCENARIO LEVEL OF SERVICE:							B	C				

NOTES:

RTOR: (a)
 (b) 4%
 (c)
 (d) 72%

Printed: 10/12/15

HCS 2010 Two-Way Stop Control Summary Report

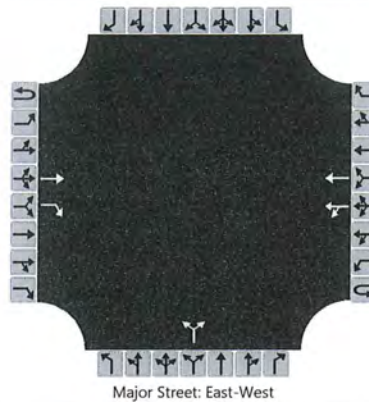
General Information

Analyst	MMF
Agency/Co.	ATE
Date Performed	10/12/2015
Analysis Year	2015
Time Analyzed	A.M. PEAK HOUR
Intersection Orientation	East-West
Project Description	HERITAGE RIDGE PROJECT (#12089.01)

Site Information

Intersection	05AM_EX
Jurisdiction	CITY OF GOLETA
East/West Street	CALLE KORAL
North/South Street	LOS CARNEROS WAY
Peak Hour Factor	1.00
Analysis Time Period (hrs)	1.00

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Priority																
Number of Lanes	0	0	1	1	0	0	2	0		0	0	0		0	0	0
Configuration			T	R		LT	T				LR					
Volume (veh/h)			26	202		41	95			52		11				
Percent Heavy Vehicles						3				3		3				
Proportion Time Blocked																
Right Turn Channelized	No				No				No				No			
Median Type	Undivided															
Median Storage																

Delay, Queue Length, and Level of Service

Flow Rate (veh/h)					89							63				
Capacity					1330							959				
v/c Ratio					0.07							0.07				
95% Queue Length					0.1							0.2				
Control Delay (s/veh)					7.8							9.0				
Level of Service (LOS)					A							A				
Approach Delay (s/veh)					2.4				9.0							
Approach LOS					A				A							

AVD = 8.3 sec. / LOS A

HCS 2010 Two-Way Stop Control Summary Report

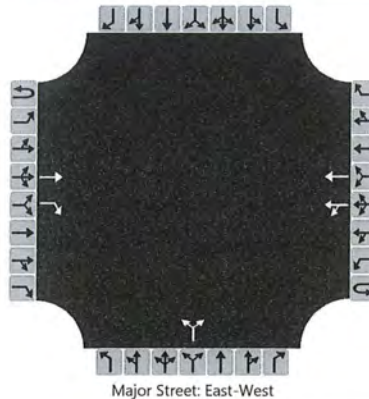
General Information

Analyst	MMF
Agency/Co.	ATE
Date Performed	10/12/2015
Analysis Year	2015
Time Analyzed	A.M. PEAK HOUR
Intersection Orientation	East-West
Project Description	HERITAGE RIDGE PROJECT (#12089.01)

Site Information

Intersection	05AM_EX w/ DIVERSION
Jurisdiction	CITY OF GOLETA
East/West Street	CALLE KORAL
North/South Street	LOS CARNEROS WAY
Peak Hour Factor	1.00
Analysis Time Period (hrs)	1.00

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	1	0	0	2	0		0	0	0		0	0	0
Configuration			T	R		LT	T				LR					
Volume (veh/h)			36	171		41	107			40		11				
Percent Heavy Vehicles						3				3		3				
Proportion Time Blocked																
Right Turn Channelized	No				No				No				No			
Median Type	Undivided															
Median Storage																

Delay, Queue Length, and Level of Service

Flow Rate (veh/h)					95							51				
Capacity					1354							987				
v/c Ratio					0.07							0.05				
95% Queue Length					0.1							0.2				
Control Delay (s/veh)					7.7							8.8				
Level of Service (LOS)					A							A				
Approach Delay (s/veh)					2.2				8.8							
Approach LOS					A				A							

AWD = 8.1 sec./LOS A

HCS 2010 Two-Way Stop Control Summary Report

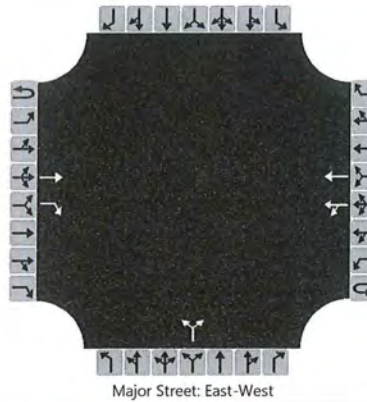
General Information

Analyst	MMF
Agency/Co.	ATE
Date Performed	10/12/2015
Analysis Year	2015
Time Analyzed	A.M. PEAK HOUR
Intersection Orientation	East-West
Project Description	HERITAGE RIDGE PROJECT (#12089.01)

Site Information

Intersection	05AM_EX+PR
Jurisdiction	CITY OF GOLETA
East/West Street	CALLE KORAL
North/South Street	LOS CARNEROS WAY
Peak Hour Factor	1.00
Analysis Time Period (hrs)	1.00

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Priority																
Number of Lanes	0	0	1	1	0	0	2	0		0	0	0		0	0	0
Configuration			T	R		LT	T				LR					
Volume (veh/h)			81	171		66	208			40		18				
Percent Heavy Vehicles						3				3		3				
Proportion Time Blocked																
Right Turn Channelized	No				No				No				No			
Median Type	Undivided															
Median Storage																

Delay, Queue Length, and Level of Service

Flow Rate (veh/h)					170						58					
Capacity					1303						893					
v/c Ratio					0.13						0.06					
95% Queue Length					0.2						0.2					
Control Delay (s/veh)					7.9						9.3					
Level of Service (LOS)					A						A					
Approach Delay (s/veh)					2.0				9.3							
Approach LOS					A				A							

AWD = 8.3 sec. / LOS A

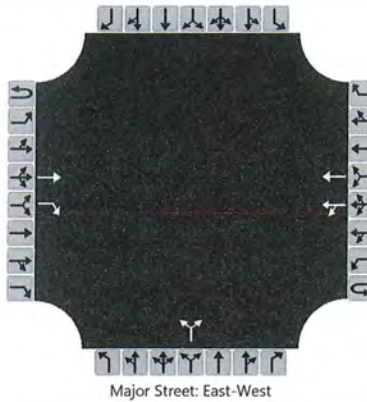
HCS 2010 Two-Way Stop Control Summary Report

General Information

Site Information

Analyst	MMF	Intersection	05PM_EX
Agency/Co.	ATE	Jurisdiction	CITY OF GOLETA
Date Performed	10/12/2015	East/West Street	CALLE KORAL
Analysis Year	2015	North/South Street	LOS CARNEROS WAY
Time Analyzed	P.M. PEAK HOUR	Peak Hour Factor	1.00
Intersection Orientation	East-West	Analysis Time Period (hrs)	1.00
Project Description	HERITAGE RIDGE PROJECT (#12089.01)		

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Priority																
Number of Lanes	0	0	1	1	0	0	2	0		0	0	0		0	0	0
Configuration			T	R		LT	T				LR					
Volume (veh/h)			61	52		7	37			267		43				
Percent Heavy Vehicles						3				3		3				
Proportion Time Blocked																
Right Turn Channelized	No				No				No				No			
Median Type	Undivided															
Median Storage																

Delay, Queue Length, and Level of Service

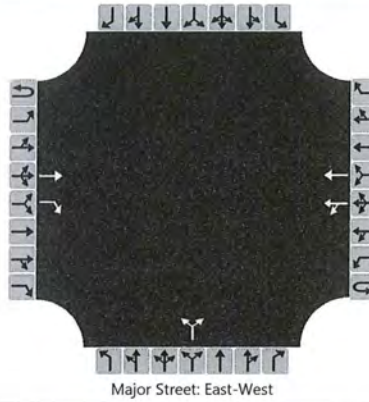
Flow Rate (veh/h)					26								310			
Capacity					1467								1031			
v/c Ratio					0.02								0.30			
95% Queue Length					0.0								1.3			
Control Delay (s/veh)					7.5								10.0			
Level of Service (LOS)					A								A			
Approach Delay (s/veh)					1.2				10.0							
Approach LOS					A				A							

AWD = 9.8 sec. / LOS A

HCS 2010 Two-Way Stop Control Summary Report

General Information		Site Information	
Analyst	MMF	Intersection	05PM_EX w/ DIVERSION
Agency/Co.	ATE	Jurisdiction	CITY OF GOLETA
Date Performed	10/12/2015	East/West Street	CALLE KORAL
Analysis Year	2015	North/South Street	LOS CARNEROS WAY
Time Analyzed	P.M. PEAK HOUR	Peak Hour Factor	1.00
Intersection Orientation	East-West	Analysis Time Period (hrs)	1.00
Project Description	HERITAGE RIDGE PROJECT (#12089.01)		

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6	7	8	9		10	11	12	
Priority																
Number of Lanes	0	0	1	1	0	0	2	0	0	0	0		0	0	0	
Configuration			T	R		LT	T				LR					
Volume (veh/h)			71	42		7	79			225		43				
Percent Heavy Vehicles						3				3		3				
Proportion Time Blocked																
Right Turn Channelized	No				No				No				No			
Median Type	Undivided															
Median Storage																

Delay, Queue Length, and Level of Service

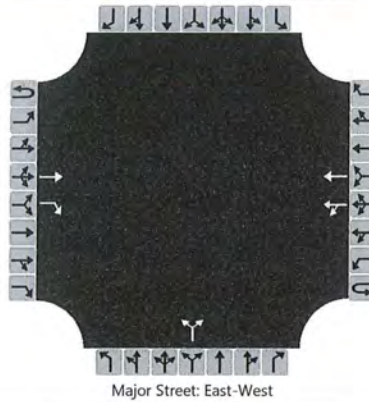
Flow Rate (veh/h)					47						268					
Capacity					1467						1014					
v/c Ratio					0.03						0.26					
95% Queue Length					0.0						1.1					
Control Delay (s/veh)					7.5						9.8					
Level of Service (LOS)					A						A					
Approach Delay (s/veh)					0.6				9.8							
Approach LOS					A				A							

AWD = 9.5 sec. / LOS A

HCS 2010 Two-Way Stop Control Summary Report

General Information		Site Information	
Analyst	MMF	Intersection	05PM_EX+PR
Agency/Co.	ATE	Jurisdiction	CITY OF GOLETA
Date Performed	10/12/2015	East/West Street	CALLE KORAL
Analysis Year	2015	North/South Street	LOS CARNEROS WAY
Time Analyzed	P.M. PEAK HOUR	Peak Hour Factor	1.00
Intersection Orientation	East-West	Analysis Time Period (hrs)	1.00
Project Description	HERITAGE RIDGE PROJECT (#12089.01)		

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6	7	8	9		10	11	12	
Priority																
Number of Lanes	0	0	1	1	0	0	2	0	0	0	0		0	0	0	
Configuration			T	R		LT	T			LR						
Volume (veh/h)			159	42		18	122			225		65				
Percent Heavy Vehicles						3				3		3				
Proportion Time Blocked																
Right Turn Channelized	No				No				No				No			
Median Type	Undivided															
Median Storage																

Delay, Queue Length, and Level of Service

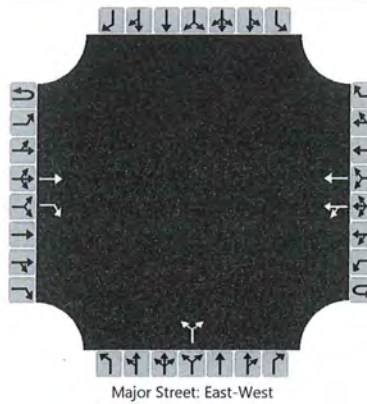
Flow Rate (veh/h)					79					290						
Capacity					1361					901						
v/c Ratio					0.06					0.32						
95% Queue Length					0.0					1.4						
Control Delay (s/veh)					7.7					10.9						
Level of Service (LOS)					A					B						
Approach Delay (s/veh)					1.0				10.9							
Approach LOS					A				B							

AWD = 10.2 sec. / LOS B

HCS 2010 Two-Way Stop Control Summary Report

General Information		Site Information	
Analyst	MMF	Intersection	05AM_CU
Agency/Co.	ATE	Jurisdiction	CITY OF GOLETA
Date Performed	10/12/2015	East/West Street	CALLE KORAL
Analysis Year	2015	North/South Street	LOS CARNEROS WAY
Time Analyzed		Peak Hour Factor	1.00
Intersection Orientation	East-West	Analysis Time Period (hrs)	1.00
Project Description	HERITAGE RIDGE PROJECT (#12089.01)		

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Priority																
Number of Lanes	0	0	1	1	0	0	2	0		0	0	0		0	0	0
Configuration			T	R		LT	T				LR					
Volume (veh/h)			103	209		41	95			185		63				
Percent Heavy Vehicles						3				3		3				
Proportion Time Blocked																
Right Turn Channelized	No				No				No				No			
Median Type	Undivided															
Median Storage																

Delay, Queue Length, and Level of Service

Flow Rate (veh/h)					89						248					
Capacity					1238						950					
v/c Ratio					0.07						0.26					
95% Queue Length					0.1						1.1					
Control Delay (s/veh)					8.0						10.1					
Level of Service (LOS)					A						B					
Approach Delay (s/veh)					2.5				10.1							
Approach LOS					A				B							

AWD = 9.5 sec. / LOS A

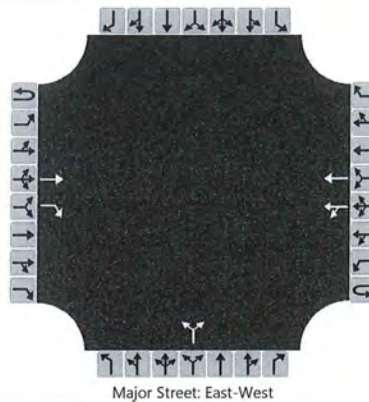
HCS 2010 Two-Way Stop Control Summary Report

General Information

Site Information

Analyst	MMF	Intersection	05AM_CU+PR
Agency/Co.	ATE	Jurisdiction	CITY OF GOLETA
Date Performed	10/12/2015	East/West Street	CALLE KORAL
Analysis Year	2015	North/South Street	LOS CARNEROS WAY
Time Analyzed		Peak Hour Factor	1.00
Intersection Orientation	East-West	Analysis Time Period (hrs)	1.00
Project Description	HERITAGE RIDGE PROJECT (#12089.01)		

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Priority																
Number of Lanes	0	0	1	1	0	0	2	0		0	0	0		0	0	0
Configuration			T	R		LT	T				LR					
Volume (veh/h)			127	209		66	196			185		70				
Percent Heavy Vehicles						3				3		3				
Proportion Time Blocked																
Right Turn Channelized	No				No				No				No			
Median Type	Undivided															
Median Storage																

Delay, Queue Length, and Level of Service

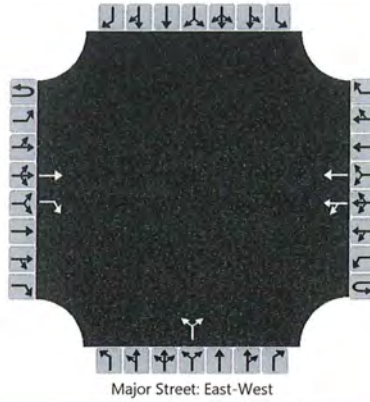
Flow Rate (veh/h)					164					255						
Capacity					1213					798						
v/c Ratio					0.14					0.32						
95% Queue Length					0.2					1.4						
Control Delay (s/veh)					8.1					11.6						
Level of Service (LOS)					A					B						
Approach Delay (s/veh)					2.2				11.6							
Approach LOS					A				B							

AWD = 10.2 sec. / LOS B

HCS 2010 Two-Way Stop Control Summary Report

General Information		Site Information	
Analyst	MMF	Intersection	05PM_CU
Agency/Co.	ATE	Jurisdiction	CITY OF GOLETA
Date Performed	10/12/2015	East/West Street	CALLE KORAL
Analysis Year	2015	North/South Street	LOS CARNEROS WAY
Time Analyzed		Peak Hour Factor	1.00
Intersection Orientation	East-West	Analysis Time Period (hrs)	1.00
Project Description	HERITAGE RIDGE PROJECT (#12089.01)		

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Priority																
Number of Lanes	0	0	1	1	0	0	2	0		0	0	0		0	0	0
Configuration			T	R		LT	T				LR					
Volume (veh/h)			163	72		7	37			365		73				
Percent Heavy Vehicles						3				3		3				
Proportion Time Blocked																
Right Turn Channelized	No				No				No				No			
Median Type	Undivided															
Median Storage																

Delay, Queue Length, and Level of Service

Flow Rate (veh/h)						26					438					
Capacity						1322					921					
v/c Ratio						0.02					0.48					
95% Queue Length						0.0					2.7					
Control Delay (s/veh)						7.7					12.4					
Level of Service (LOS)						A					B					
Approach Delay (s/veh)					1.2				12.4							
Approach LOS					A				B							

AWD = 12.1 sec. / LOS B

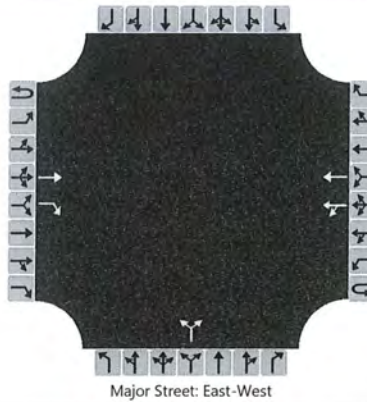
HCS 2010 Two-Way Stop Control Summary Report

General Information

Site Information

Analyst	MMF	Intersection	05PM_CU+PR
Agency/Co.	ATE	Jurisdiction	CITY OF GOLETA
Date Performed	10/12/2015	East/West Street	CALLE KORAL
Analysis Year	2015	North/South Street	LOS CARNEROS WAY
Time Analyzed		Peak Hour Factor	1.00
Intersection Orientation	East-West	Analysis Time Period (hrs)	1.00
Project Description	HERITAGE RIDGE PROJECT (#12089.01)		

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Priority																
Number of Lanes	0	0	1	1	0	0	2	0		0	0	0		0	0	0
Configuration			T	R		LT	T				LR					
Volume (veh/h)			251	72		18	80			365		95				
Percent Heavy Vehicles						3				3		3				
Proportion Time Blocked																
Right Turn Channelized	No				No				No				No			
Median Type	Undivided															
Median Storage																

Delay, Queue Length, and Level of Service

Flow Rate (veh/h)					58						460					
Capacity					1227						794					
v/c Ratio					0.05						0.58					
95% Queue Length					0.0						4.0					
Control Delay (s/veh)					8.0						15.7					
Level of Service (LOS)					A						C					
Approach Delay (s/veh)					1.5				15.7							
Approach LOS					A				C							

AWD = 14.8 sec. / LOS B

#12089.01 HERITAGE RIDGE PROJECT

REF: 06AM

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE: 05/22/2013
 TIME PERIOD: A.M. PEAK HOUR
 N/S STREET: LOS CARNEROS ROAD
 E/W STREET: HOLLISTER AVENUE
 CONTROL TYPE: SIGNAL

TRAFFIC VOLUME SUMMARY

VOLUMES	NORTH BOUND			SOUTH BOUND			EAST BOUND			WEST BOUND		
	L	T	R	L	T	R	L	T	R	L	T	R
(A) EXISTING:	29	282	104	25	238	121	76	325	199	76	254	41
(B) PROJECT-ADDED:	0	2	3	0	10	14	3	4	0	11	14	0
(C) CUMULATIVE:	66	361	129	40	429	361	77	401	258	76	307	41

GEOMETRICS

LANE GEOMETRICS	NORTH BOUND			SOUTH BOUND			EAST BOUND			WEST BOUND		
	LL	T	R	L	TT	R	LL	T	TR	L	T	TR

TRAFFIC SCENARIOS

SCENARIO 1 = EXISTING VOLUMES (A)
 SCENARIO 2 = EXISTING + PROJECT VOLUMES (A+B)
 SCENARIO 3 = CUMULATIVE (C)
 SCENARIO 4 = CUMULATIVE + PROJECT VOLUMES (B+C)

LEVEL OF SERVICE CALCULATIONS

MOVE-MENTS	# OF LANES	CAPACITY	SCENARIO VOLUMES				SCENARIO V/C RATIOS			
			1	2	3	4	1	2	3	4
NBL	2	3200	29	29	66	66	0.009	0.009	0.021 *	0.021 *
NBT	2	3200	282	284	361	363	0.088 *	0.089 *	0.113	0.113
NBR (a)	1	1600	71	73	88	90	0.044	0.046	0.055	0.056
SBL	1	1600	25	25	40	40	0.016 *	0.016 *	0.025	0.025
SBT	2	3200	238	248	429	439	0.074	0.078	0.134	0.137
SBR (b)	1	1600	114	127	339	353	0.071	0.079	0.212 *	0.221 *
EBL	2	3200	76	79	77	80	0.024	0.025	0.024	0.025
EBT	2	3200	325	329	401	405	0.142 *	0.143 *	0.178 *	0.179 *
EBR (c)	0	0	129	129	168	168	-	-	-	-
WBL	1	1600	76	87	76	87	0.048 *	0.054 *	0.048 *	0.054 *
WBT	2	3200	254	268	307	321	0.089	0.093	0.105	0.110
WBR (d)	0	0	30	30	30	30	-	-	-	-
LOST TIME:							0.100 *	0.100 *	0.100 *	0.100 *
TOTAL INTERSECTION CAPACITY UTILIZATION:							0.394	0.402	0.559	0.575
SCENARIO LEVEL OF SERVICE:							A	A	A	A

NOTES:

RTOR: (a) 32%
 (b) 6%
 (c) 35%
 (d) 27%

Printed: 10/12/15

#12089.01 HERITAGE RIDGE PROJECT

REF: 06PM

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE: 02/09/2012
 TIME PERIOD: P.M. PEAK HOUR
 N/S STREET: LOS CARNEROS ROAD
 E/W STREET: HOLLISTER AVENUE
 CONTROL TYPE: SIGNAL

TRAFFIC VOLUME SUMMARY

VOLUMES	NORTH BOUND			SOUTH BOUND			EAST BOUND			WEST BOUND		
	L	T	R	L	T	R	L	T	R	L	T	R
(A) EXISTING:	229	546	110	44	443	127	243	385	87	127	642	29
(B) PROJECT-ADDED:	0	9	9	0	4	6	12	13	0	5	6	0
(C) CUMULATIVE:	273	707	142	61	518	193	324	505	117	132	664	34

GEOMETRICS

LANE GEOMETRICS	NORTH BOUND			SOUTH BOUND			EAST BOUND			WEST BOUND		
	LL	T	R	L	TT	R	LL	T	TR	L	T	TR

TRAFFIC SCENARIOS

SCENARIO 1 = EXISTING VOLUMES (A)
 SCENARIO 2 = EXISTING + PROJECT VOLUMES (A+B)
 SCENARIO 3 = CUMULATIVE (C)
 SCENARIO 4 = CUMULATIVE + PROJECT VOLUMES (B+C)

LEVEL OF SERVICE CALCULATIONS

MOVE- MENTS	# OF LANES	CAPACITY	SCENARIO VOLUMES				SCENARIO V/C RATIOS					
			1	2	3	4	1	2	3	4		
NBL	2	3200	229	229	273	273	0.072 *	0.072 *	0.085	0.085		
NBT	2	3200	546	555	707	716	0.171	0.173	0.221 *	0.224 *		
NBR (a)	1	1600	50	54	64	68	0.031	0.034	0.040	0.043		
SBL	1	1600	44	44	61	61	0.028	0.028	0.038 *	0.038 *		
SBT	2	3200	443	447	518	522	0.138 *	0.140 *	0.162	0.163		
SBR (b)	1	1600	64	67	97	100	0.040	0.042	0.061	0.063		
EBL	2	3200	243	255	324	336	0.076 *	0.080 *	0.101	0.105		
EBT	2	3200	385	398	505	518	0.134	0.138	0.177 *	0.181 *		
EBR (c)	0	0	44	44	60	60	-	-	-	-		
WBL	1	1600	127	132	132	137	0.079	0.083	0.083 *	0.086 *		
WBT	2	3200	642	648	664	670	0.207 *	0.209 *	0.215	0.217		
WBR (d)	0	0	21	21	24	24	-	-	-	-		
LOST TIME:							0.100 *	0.100 *	0.100 *	0.100 *		
TOTAL INTERSECTION CAPACITY UTILIZATION:							0.593	0.601	0.619	0.629		
SCENARIO LEVEL OF SERVICE:							A	A	B	B		

NOTES:

RTOR: (a) 55%
 (b) 50%
 (c) 49%
 (d) 28%

Printed: 10/12/15

#12089.01 HERITAGE RIDGE PROJECT

REF: 07AM_EX

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE: 04/03/2013
 TIME PERIOD: A.M. PEAK HOUR
 N/S STREET: LOS CARNEROS WAY
 E/W STREET: HOLLISTER AVENUE
 CONTROL TYPE: SIGNAL

TRAFFIC VOLUME SUMMARY

VOLUMES	NORTH BOUND			SOUTH BOUND			EAST BOUND			WEST BOUND		
	L	T	R	L	T	R	L	T	R	L	T	R
(A) EXISTING:	0	0	0	153	0	27	22	408	0	0	321	45

GEOMETRICS

LANE GEOMETRICS	NORTH BOUND	SOUTH BOUND	EAST BOUND	WEST BOUND
		L LR R	L TT	T TR

TRAFFIC SCENARIOS

SCENARIO 1 = EXISTING VOLUMES (A)

LEVEL OF SERVICE CALCULATIONS

MOVE-MENTS	# OF LANES	CAPACITY	SCENARIO VOLUMES				SCENARIO V/C RATIOS					
			1	2	3	4	1	2	3	4		
NBL	0	0	0					-				
NBT	0	0	0					-				
NBR (a)	0	0	0					-				
SBL	0	0	153					-				
SBT	2	3200	0					0.051 *				
SBR (b)	0	0	9					-				
EBL	1	1600	22					0.014 *				
EBT	2	3200	408					0.128				
EBR (c)	0	0	0					-				
WBL	0	0	0					-				
WBT	2	3200	321					0.112 *				
WBR (d)	0	0	38					-				
LOST TIME:								0.100 *				
TOTAL INTERSECTION CAPACITY UTILIZATION:								0.277				
SCENARIO LEVEL OF SERVICE:								A				

NOTES:

RTOR: (a)
 (b) 67%
 (c)
 (d) 16%

Printed: 10/13/15

#12089.01 HERITAGE RIDGE PROJECT

REF: 07AM

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE: 04/03/2013
 TIME PERIOD: A.M. PEAK HOUR
 N/S STREET: LOS CARNEROS WAY
 E/W STREET: HOLLISTER AVENUE
 CONTROL TYPE: SIGNAL

TRAFFIC VOLUME SUMMARY

VOLUMES	NORTH BOUND			SOUTH BOUND			EAST BOUND			WEST BOUND		
	L	T	R	L	T	R	L	T	R	L	T	R
(A) EXISTING:	0	0	0	122	0	27	22	408	0	0	321	33
(B) PROJECT-ADDED:	0	0	0	0	0	25	7	0	0	0	0	0
(C) CUMULATIVE:	0	0	0	174	0	44	172	423	0	0	357	111

GEOMETRICS

LANE GEOMETRICS	NORTH BOUND			SOUTH BOUND			EAST BOUND		WEST BOUND	
	L	LR	R	L	LR	R	L	TT	T	TR

TRAFFIC SCENARIOS

SCENARIO 1 = EXISTING VOLUMES (A)
 SCENARIO 2 = EXISTING + PROJECT VOLUMES (A+B)
 SCENARIO 3 = CUMULATIVE (C)
 SCENARIO 4 = CUMULATIVE + PROJECT VOLUMES (B+C)

LEVEL OF SERVICE CALCULATIONS

MOVE-MENTS	# OF LANES	CAPACITY	SCENARIO VOLUMES				SCENARIO V/C RATIOS					
			1	2	3	4	1	2	3	4		
NBL	0	0	0	0	0	0	-	-	-	-		
NBT	0	0	0	0	0	0	-	-	-	-		
NBR (a)	0	0	0	0	0	0	-	-	-	-		
SBL	0	0	122	122	174	174	-	-	-	-		
SBT	2	3200	0	0	0	0	0.041 *	0.043 *	0.059 *	0.062 *		
SBR (b)	0	0	9	17	15	23	-	-	-	-		
EBL	1	1600	22	29	172	179	0.014 *	0.018 *	0.108 *	0.112 *		
EBT	2	3200	408	408	423	423	0.128	0.128	0.132	0.132		
EBR (c)	0	0	0	0	0	0	-	-	-	-		
WBL	0	0	0	0	0	0	-	-	-	-		
WBT	2	3200	321	321	357	357	0.109 *	0.109 *	0.141 *	0.141 *		
WBR (d)	0	0	28	28	93	93	-	-	-	-		
LOST TIME:							0.100 *	0.100 *	0.100 *	0.100 *		
TOTAL INTERSECTION CAPACITY UTILIZATION:							0.264	0.270	0.408	0.415		
SCENARIO LEVEL OF SERVICE:							A	A	A	A		

NOTES:

RTOR: (a)
 (b) 67%
 (c)
 (d) 16%

Printed: 10/12/15

#12089.01 HERITAGE RIDGE PROJECT

REF: 07PM_EX

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE: 04/03/2013
 TIME PERIOD: P.M. PEAK HOUR
 N/S STREET: LOS CARNEROS WAY
 E/W STREET: HOLLISTER AVENUE
 CONTROL TYPE: SIGNAL

TRAFFIC VOLUME SUMMARY

VOLUMES	NORTH BOUND			SOUTH BOUND			EAST BOUND			WEST BOUND		
	L	T	R	L	T	R	L	T	R	L	T	R
(A) EXISTING:	0	0	0	35	0	15	34	517	0	0	760	221

GEOMETRICS

LANE GEOMETRICS	NORTH BOUND			SOUTH BOUND			EAST BOUND			WEST BOUND		
	L	T	R	L	LR	R	L	TT	R	L	T	TR

TRAFFIC SCENARIOS

SCENARIO 1 = EXISTING VOLUMES (A)

LEVEL OF SERVICE CALCULATIONS

MOVE- MENTS	# OF LANES	CAPACITY	SCENARIO VOLUMES				SCENARIO V/C RATIOS					
			1	2	3	4	1	2	3	4		
NBL	0	0	0					-				
NBT	0	0	0					-				
NBR (a)	0	0	0					-				
SBL	0	0	35					-				
SBT	2	3200	0					0.012 *				
SBR (b)	0	0	4					-				
EBL	1	1600	34					0.021 *				
EBT	2	3200	517					0.162				
EBR (c)	0	0	0					-				
WBL	0	0	0					-				
WBT	2	3200	760					0.300 *				
WBR (d)	0	0	199					-				
LOST TIME:								0.100 *				
TOTAL INTERSECTION CAPACITY UTILIZATION:								0.433				
SCENARIO LEVEL OF SERVICE:								A				

NOTES:

- RTOR: (a)
- (b) 73%
- (c)
- (d) 10%

Printed: 10/13/15

#12089.01 HERITAGE RIDGE PROJECT

REF: 07PM

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE: 04/03/2013
 TIME PERIOD: P.M. PEAK HOUR
 N/S STREET: LOS CARNEROS WAY
 E/W STREET: HOLLISTER AVENUE
 CONTROL TYPE: SIGNAL

TRAFFIC VOLUME SUMMARY

VOLUMES	NORTH BOUND			SOUTH BOUND			EAST BOUND			WEST BOUND		
	L	T	R	L	T	R	L	T	R	L	T	R
(A) EXISTING:	0	0	0	25	0	15	34	517	0	0	760	179
(B) PROJECT-ADDED:	0	0	0	0	0	11	22	0	0	0	0	0
(C) CUMULATIVE:	0	0	0	35	0	45	120	575	0	0	760	332

GEOMETRICS

LANE GEOMETRICS	NORTH BOUND	SOUTH BOUND	EAST BOUND	WEST BOUND
		L LR R	L TT	T TR

TRAFFIC SCENARIOS

SCENARIO 1 = EXISTING VOLUMES (A)
 SCENARIO 2 = EXISTING + PROJECT VOLUMES (A+B)
 SCENARIO 3 = CUMULATIVE (C)
 SCENARIO 4 = CUMULATIVE + PROJECT VOLUMES (B+C)

LEVEL OF SERVICE CALCULATIONS

MOVE- MENTS	# OF LANES	CAPACITY	SCENARIO VOLUMES				SCENARIO V/C RATIOS					
			1	2	3	4	1	2	3	4		
NBL	0	0	0	0	0	0	-	-	-	-		
NBT	0	0	0	0	0	0	-	-	-	-		
NBR (a)	0	0	0	0	0	0	-	-	-	-		
SBL	0	0	25	25	35	35	-	-	-	-		
SBT	2	3200	0	0	0	0	0.009 *	0.010 *	0.015 *	0.016 *		
SBR (b)	0	0	4	7	12	15	-	-	-	-		
EBL	1	1600	34	56	120	142	0.021 *	0.035 *	0.075 *	0.089 *		
EBT	2	3200	517	517	575	575	0.162	0.162	0.180	0.180		
EBR (c)	0	0	0	0	0	0	-	-	-	-		
WBL	0	0	0	0	0	0	-	-	-	-		
WBT	2	3200	760	760	760	760	0.288 *	0.288 *	0.331 *	0.331 *		
WBR (d)	0	0	161	161	299	299	-	-	-	-		
LOST TIME:							0.100 *	0.100 *	0.100 *	0.100 *		
TOTAL INTERSECTION CAPACITY UTILIZATION:							0.418	0.433	0.521	0.536		
SCENARIO LEVEL OF SERVICE:							A	A	A	A		

NOTES:

RTOR: (a)
 (b) 73%
 (c)
 (d) 10%

Printed: 10/12/15

#12089.01 HERITAGE RIDGE PROJECT

REF: 08AM_EX

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE: 02/09/2012
 TIME PERIOD: A.M. PEAK HOUR
 N/S STREET: AERO CAMINO
 E/W STREET: HOLLISTER AVENUE
 CONTROL TYPE: SIGNAL

TRAFFIC VOLUME SUMMARY

VOLUMES	NORTH BOUND			SOUTH BOUND			EAST BOUND			WEST BOUND		
	L	T	R	L	T	R	L	T	R	L	T	R
(A) EXISTING:	7	3	2	38	1	31	77	464	16	3	340	89

GEOMETRICS

LANE GEOMETRICS	NORTH BOUND		SOUTH BOUND		EAST BOUND		WEST BOUND	
	L	T	L	T	L	T	L	T

TRAFFIC SCENARIOS

SCENARIO 1 = EXISTING VOLUMES (A)

LEVEL OF SERVICE CALCULATIONS

MOVE-MENTS	# OF LANES	CAPACITY	SCENARIO VOLUMES				SCENARIO V/C RATIOS					
			1	2	3	4	1	2	3	4		
NBL	0	0	7					-				
NBT	1	1600	3					0.008 *				
NBR (a)	0	0	2					-				
SBL	0	0	38					-				
SBT	1	1600	1					0.024 *				
SBR (b)	1	1600	7					0.004				
EBL	1	1600	77					0.048 *				
EBT	2	3200	464					0.150				
EBR (c)	0	0	15					-				
WBL	1	1600	3					0.002				
WBT	2	3200	340					0.130 *				
WBR (d)	0	0	75					-				
LOST TIME:								0.100 *				
TOTAL INTERSECTION CAPACITY UTILIZATION:								0.310				
SCENARIO LEVEL OF SERVICE:								A				

NOTES:

RTOR: (a) 0%
 (b) 77%
 (c) 6%
 (d) 16%

Printed: 10/13/15

#12089.01 HERITAGE RIDGE PROJECT

REF: 08AM

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE: 02/09/2012
 TIME PERIOD: A.M. PEAK HOUR
 N/S STREET: AERO CAMINO
 E/W STREET: HOLLISTER AVENUE
 CONTROL TYPE: SIGNAL

TRAFFIC VOLUME SUMMARY

VOLUMES	NORTH BOUND			SOUTH BOUND			EAST BOUND			WEST BOUND		
	L	T	R	L	T	R	L	T	R	L	T	R
(A) EXISTING:	7	3	2	38	1	19	46	464	16	3	340	89
(B) PROJECT-ADDED:	0	0	0	14	0	0	0	0	0	0	0	3
(C) CUMULATIVE:	7	3	2	46	1	38	81	505	16	3	409	96

GEOMETRICS

LANE GEOMETRICS	NORTH BOUND			SOUTH BOUND		EAST BOUND			WEST BOUND		
	LTR			LT	R	L	T	TR	L T R		

TRAFFIC SCENARIOS

SCENARIO 1 = EXISTING VOLUMES (A)
 SCENARIO 2 = EXISTING + PROJECT VOLUMES (A+B)
 SCENARIO 3 = CUMULATIVE (C)
 SCENARIO 4 = CUMULATIVE + PROJECT VOLUMES (B+C)

LEVEL OF SERVICE CALCULATIONS

MOVE- MENTS	# OF LANES	CAPACITY	SCENARIO VOLUMES				SCENARIO V/C RATIOS					
			1	2	3	4	1	2	3	4		
NBL	0	0	7	7	7	7	-	-	-	-		
NBT	1	1600	3	3	3	3	0.008 *	0.008 *	0.008 *	0.008 *		
NBR (a)	0	0	2	2	2	2	-	-	-	-		
SBL	0	0	38	52	46	60	-	-	-	-		
SBT	1	1600	1	1	1	1	0.024 *	0.033 *	0.029 *	0.038 *		
SBR (b)	1	1600	4	4	9	9	0.003	0.003	0.006	0.006		
EBL	1	1600	46	46	81	81	0.029 *	0.029 *	0.051 *	0.051 *		
EBT	2	3200	464	464	505	505	0.150	0.150	0.163	0.163		
EBR (c)	0	0	15	15	15	15	-	-	-	-		
WBL	1	1600	3	3	3	3	0.002	0.002	0.002	0.002		
WBT	2	3200	340	340	409	409	0.130 *	0.130 *	0.153 *	0.154 *		
WBR (d)	0	0	75	77	81	83	-	-	-	-		
LOST TIME:							0.100 *	0.100 *	0.100 *	0.100 *		
TOTAL INTERSECTION CAPACITY UTILIZATION:							0.291	0.300	0.341	0.351		
SCENARIO LEVEL OF SERVICE:							A	A	A	A		

NOTES:

RTOR: (a) 0%
 (b) 77%
 (c) 6%
 (d) 16%

Printed: 10/12/15

#12089.01 HERITAGE RIDGE PROJECT

REF: 08PM_EX

INTERSECTION CAPACITY UTILIZATION WORKSHEET

COUNT DATE: 02/09/2012
 TIME PERIOD: P.M. PEAK HOUR
 N/S STREET: AERO CAMINO
 E/W STREET: HOLLISTER AVENUE
 CONTROL TYPE: SIGNAL

TRAFFIC VOLUME SUMMARY

VOLUMES	NORTH BOUND			SOUTH BOUND			EAST BOUND			WEST BOUND		
	L	T	R	L	T	R	L	T	R	L	T	R
(A) EXISTING:	33	0	5	67	1	104	26	534	13	5	810	30

GEOMETRICS

LANE GEOMETRICS	NORTH BOUND			SOUTH BOUND		EAST BOUND			WEST BOUND		
	L	T	R	L	R	L	T	TR	L	T	TR

TRAFFIC SCENARIOS

SCENARIO 1 = EXISTING VOLUMES (A)

LEVEL OF SERVICE CALCULATIONS

MOVE-MENTS	# OF LANES	CAPACITY	SCENARIO VOLUMES				SCENARIO V/C RATIOS						
			1	2	3	4	1	2	3	4			
NBL	0	0	33	33	0	0	-						
NBT	1	1600	0	0	0	0	0.022 *						
NBR (a)	0	0	2	2	0	0	-						
SBL	0	0	67	67	0	0	-						
SBT	1	1600	1	1	0	0	0.043 *						
SBR (b)	1	1600	59	59	0	0	0.037						
EBL	1	1600	26	26	0	0	0.016 *						
EBT	2	3200	534	534	0	0	0.170						
EBR (c)	0	0	11	11	0	0	-						
WBL	1	1600	5	5	0	0	0.003						
WBT	2	3200	810	810	0	0	0.261 *						
WBR (d)	0	0	24	24	0	0	-						
LOST TIME:							0.100 *						
TOTAL INTERSECTION CAPACITY UTILIZATION:							0.442						
SCENARIO LEVEL OF SERVICE:							A						

NOTES:

RTOR: (a) 60%
 (b) 43%
 (c) 15%
 (d) 20%

Printed: 10/30/15

#12089.01 HERITAGE RIDGE PROJECT
 INTERSECTION CAPACITY UTILIZATION WORKSHEET
 COUNT DATE: 02/09/2012
 TIME PERIOD: P.M. PEAK HOUR
 N/S STREET: AERO CAMINO
 E/W STREET: HOLLISTER AVENUE
 CONTROL TYPE: SIGNAL

REF: 08PM

TRAFFIC VOLUME SUMMARY

VOLUMES	NORTH BOUND			SOUTH BOUND			EAST BOUND			WEST BOUND		
	L	T	R	L	T	R	L	T	R	L	T	R
(A) EXISTING:	33	0	5	67	1	62	16	534	13	5	810	30
(B) PROJECT-ADDED:	0	0	0	6	0	0	0	0	0	0	0	13
(C) CUMULATIVE:	33	0	5	81	1	144	31	589	13	5	863	44

GEOMETRICS

LANE GEOMETRICS	NORTH BOUND		SOUTH BOUND		EAST BOUND		WEST BOUND	
	LTR		LT R		L T TR		L T TR	

TRAFFIC SCENARIOS

SCENARIO 1 = EXISTING VOLUMES (A)
 SCENARIO 2 = EXISTING + PROJECT VOLUMES (A+B)
 SCENARIO 3 = CUMULATIVE (C)
 SCENARIO 4 = CUMULATIVE + PROJECT VOLUMES (B+C)

LEVEL OF SERVICE CALCULATIONS

MOVE- MENTS	# OF LANES	CAPACITY	SCENARIO VOLUMES				SCENARIO V/C RATIOS					
			1	2	3	4	1	2	3	4		
NBL	0	0	33	33	33	33	-	-	-	-		
NBT	1	1600	0	0	0	0	0.022 *	0.022 *	0.022 *	0.022 *		
NBR (a)	0	0	2	2	2	2	-	-	-	-		
SBL	0	0	67	73	81	87	-	-	-	-		
SBT	1	1600	1	1	1	1	0.043 *	0.046 *	0.051 *	0.055 *		
SBR (b)	1	1600	35	35	82	82	0.022	0.022	0.051	0.051		
EBL	1	1600	16	16	31	31	0.010 *	0.010 *	0.019 *	0.019 *		
EBT	2	3200	534	534	589	589	0.170	0.170	0.188	0.188		
EBR (c)	0	0	11	11	11	11	-	-	-	-		
WBL	1	1600	5	5	5	5	0.003	0.003	0.003	0.003		
WBT	2	3200	810	810	863	863	0.261 *	0.264 *	0.281 *	0.284 *		
WBR (d)	0	0	24	34	35	46	-	-	-	-		
LOST TIME:							0.100 *	0.100 *	0.100 *	0.100 *		
TOTAL INTERSECTION CAPACITY UTILIZATION:							0.436	0.442	0.473	0.480		
SCENARIO LEVEL OF SERVICE:							A	A	A	A		

NOTES:

RTOR: (a) 60%
 (b) 43%
 (c) 15%
 (d) 20%

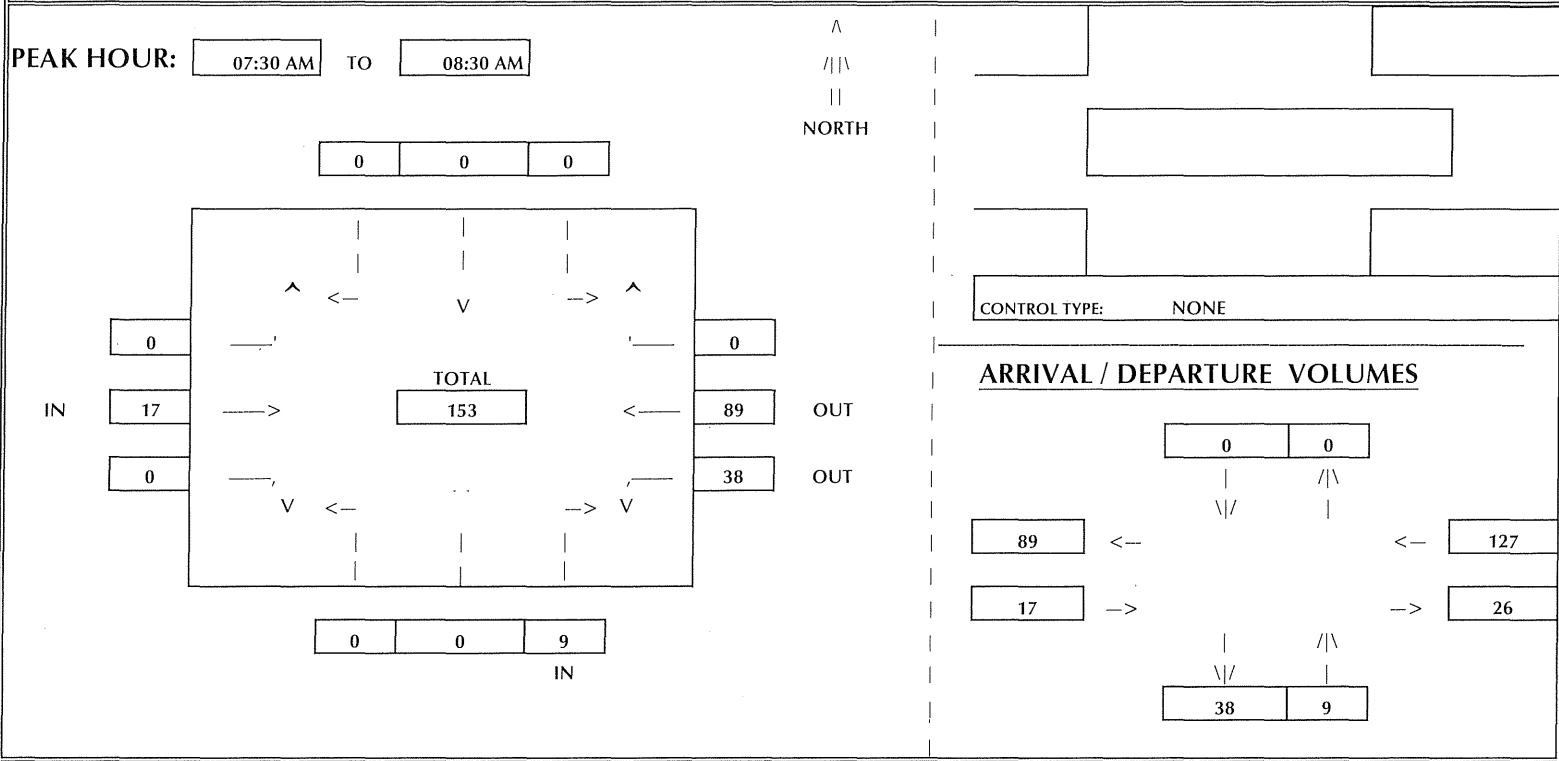
Printed: 10/13/15

WILLOWSPRINGS DRIVEWAY COUNT DATA

ASSOCIATED TRANSPORTATION ENGINEERS

INTERSECTION TURNING MOVEMENT SUMMARY

PROJECT: WILLOW SPRINGS II **PROJECT #:** 08098 **COUNT DATE:** 9-24-08 **FILE NAME:** AM_TGEN
N-S Approach: IN/OUT **COUNT TIME:** 07:00 AM TO 9:00
E-W Approach: **CITY:** GOLETA **WEATHER:** SUNNY



TIME PERIOD		NORTHBOUND			SOUTHBOUND			EASTBOUND			WESTBOUND			TOTAL
From	To	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	VOLUMES

COUNT DATA

07:00 AM	---	07:15 AM	0	0	2	0	0	0	0	3	0	2	13	0	20
07:15 AM	---	07:30 AM	0	0	3	0	0	0	0	8	0	7	33	0	51
07:30 AM	---	07:45 AM	0	0	5	0	0	0	0	12	0	10	58	0	85
07:45 AM	---	08:00 AM	0	0	6	0	0	0	0	14	0	24	82	0	126
08:00 AM	---	08:15 AM	0	0	9	0	0	0	0	18	0	38	104	0	169
08:15 AM	---	08:30 AM	0	0	12	0	0	0	0	25	0	45	122	0	204
08:30 AM	---	08:45 AM	0	0	16	0	0	0	0	35	0	49	134	0	234
08:45 AM	---	09:00 AM	0	0	19	0	0	0	0	37	0	55	148	0	259

TOTAL BY PERIOD

07:00 AM	---	07:15 AM	0	0	2	0	0	0	0	3	0	2	13	0	20
07:15 AM	---	07:30 AM	0	0	1	0	0	0	0	5	0	5	20	0	31
07:30 AM	---	07:45 AM	0	0	2	0	0	0	0	4	0	3	25	0	34
07:45 AM	---	08:00 AM	0	0	1	0	0	0	0	2	0	14	24	0	41
08:00 AM	---	08:15 AM	0	0	3	0	0	0	0	4	0	14	22	0	43
08:15 AM	---	08:30 AM	0	0	3	0	0	0	0	7	0	7	18	0	35
08:30 AM	---	08:45 AM	0	0	4	0	0	0	0	10	0	4	12	0	30
08:45 AM	---	09:00 AM	0	0	3	0	0	0	0	2	0	6	14	0	25

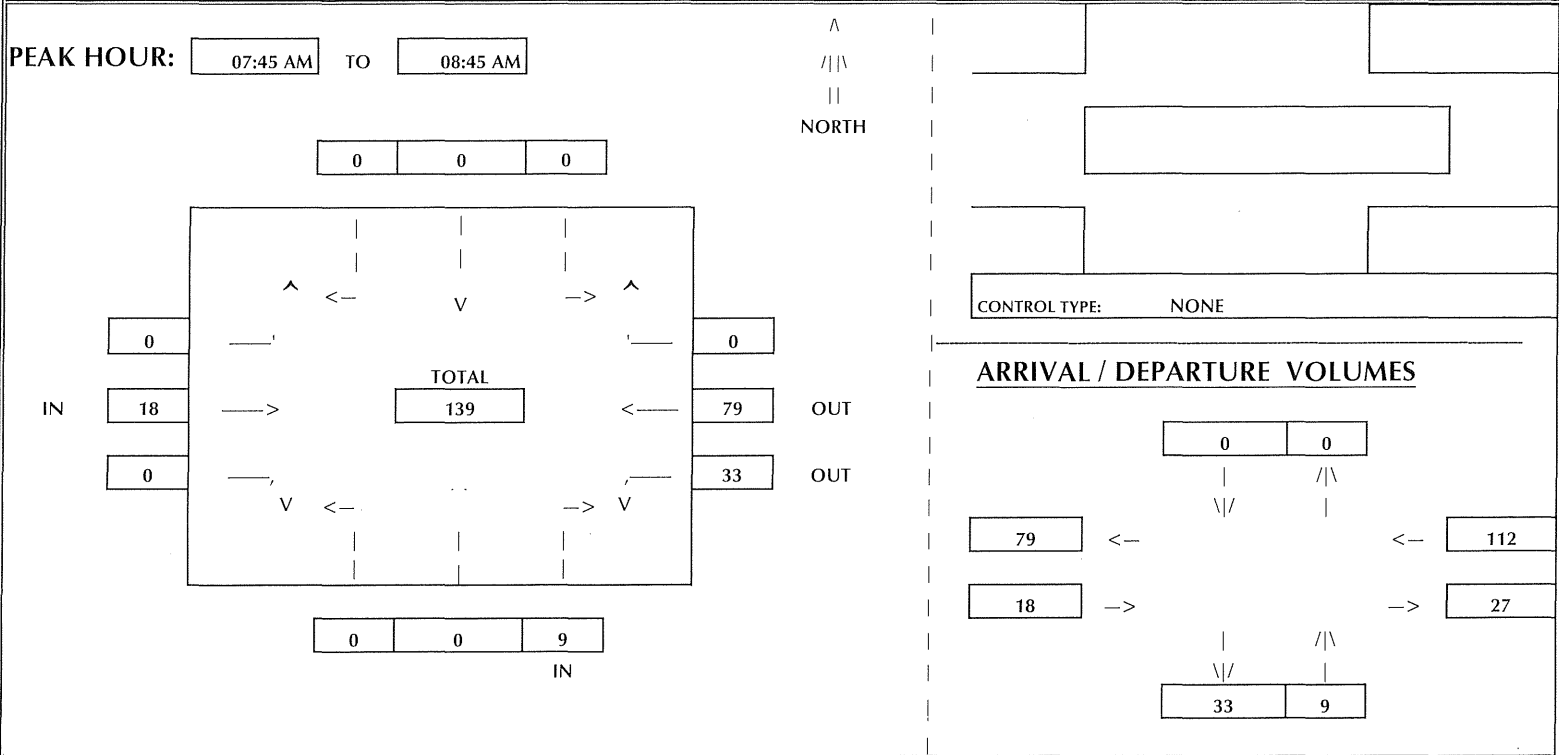
HOURLY TOTALS

07:00 AM	---	08:00 AM	0	0	6	0	0	0	0	14	0	24	82	0	126
07:15 AM	---	08:15 AM	0	0	7	0	0	0	0	15	0	36	91	0	149
07:30 AM	---	08:30 AM	0	0	9	0	0	0	0	17	0	38	89	0	153
07:45 AM	---	08:45 AM	0	0	11	0	0	0	0	23	0	39	76	0	149
08:00 AM	---	09:00 AM	0	0	13	0	0	0	0	23	0	31	66	0	133

ASSOCIATED TRANSPORTATION ENGINEERS

INTERSECTION TURNING MOVEMENT SUMMARY

PROJECT: WILLOW SPRINGS II **PROJECT #:** 08098 **COUNT DATE:** 10-09-08 **FILE NAME:** AM_TGEN
N-S Approach: IN/OUT **COUNT TIME:** 07:00 AM TO 9:00
E-W Approach: **CITY:** GOLETA **WEATHER:** SUNNY



TIME PERIOD		NORTHBOUND			SOUTHBOUND			EASTBOUND			WESTBOUND			TOTAL
From	To	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	VOLUMES

COUNT DATA

07:00 AM	---	07:15 AM	0	0	1	0	0	0	0	2	0	2	6	0	11
07:15 AM	---	07:30 AM	0	0	1	0	0	0	0	5	0	5	21	0	32
07:30 AM	---	07:45 AM	0	0	2	0	0	0	0	10	0	7	43	0	62
07:45 AM	---	08:00 AM	0	0	4	0	0	0	0	13	0	14	62	0	93
08:00 AM	---	08:15 AM	0	0	6	0	0	0	0	18	0	25	90	0	139
08:15 AM	---	08:30 AM	0	0	9	0	0	0	0	23	0	33	105	0	170
08:30 AM	---	08:45 AM	0	0	11	0	0	0	0	28	0	40	122	0	201
08:45 AM	---	09:00 AM	0	0	12	0	0	0	0	31	0	45	137	0	225

TOTAL BY PERIOD

07:00 AM	---	07:15 AM	0	0	1	0	0	0	0	2	0	2	6	0	11
07:15 AM	---	07:30 AM	0	0	0	0	0	0	0	3	0	3	15	0	21
07:30 AM	---	07:45 AM	0	0	1	0	0	0	0	5	0	2	22	0	30
07:45 AM	---	08:00 AM	0	0	2	0	0	0	0	3	0	7	19	0	31
08:00 AM	---	08:15 AM	0	0	2	0	0	0	0	5	0	11	28	0	46
08:15 AM	---	08:30 AM	0	0	3	0	0	0	0	5	0	8	15	0	31
08:30 AM	---	08:45 AM	0	0	2	0	0	0	0	5	0	7	17	0	31
08:45 AM	---	09:00 AM	0	0	1	0	0	0	0	3	0	5	15	0	24

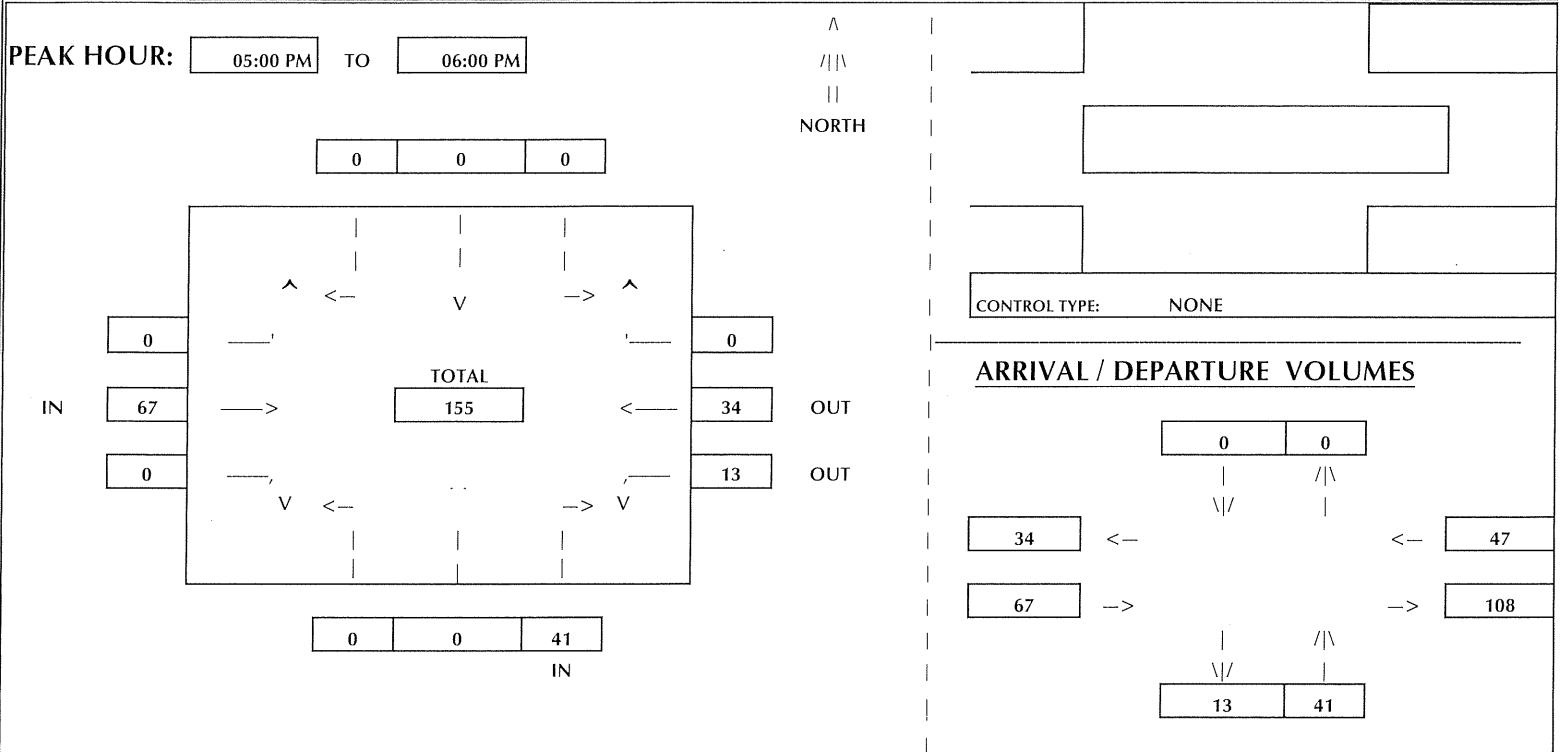
HOURLY TOTALS

07:00 AM	---	08:00 AM	0	0	4	0	0	0	0	13	0	14	62	0	93
07:15 AM	---	08:15 AM	0	0	5	0	0	0	0	16	0	23	84	0	128
07:30 AM	---	08:30 AM	0	0	8	0	0	0	0	18	0	28	84	0	138
07:45 AM	---	08:45 AM	0	0	9	0	0	0	0	18	0	33	79	0	139
08:00 AM	---	09:00 AM	0	0	8	0	0	0	0	18	0	31	75	0	132

ASSOCIATED TRANSPORTATION ENGINEERS

INTERSECTION TURNING MOVEMENT SUMMARY

PROJECT: WILLOW SPRINGS II **PROJECT #:** 08098 **COUNT DATE:** 9-23-08 **FILE NAME:** PM_TGEN
N-S Approach: IN/OUT **COUNT TIME:** 04:00 PM TO 6:00 **WEATHER:** SUNNY
E-W Approach: **CITY:** GOLETA



TIME PERIOD	NORTHBOUND			SOUTHBOUND			EASTBOUND			WESTBOUND			TOTAL VOLUMES	
	From	To	Left	Thru	Right	From	To	Left	Thru	Right	From	To		Left

COUNT DATA

04:00 PM	04:15 PM	0	0	3	0	0	0	0	5	0	1	7	0	16
04:15 PM	04:30 PM	0	0	6	0	0	0	0	23	0	4	10	0	43
04:30 PM	04:45 PM	0	0	11	0	0	0	0	32	0	6	19	0	68
04:45 PM	05:00 PM	0	0	14	0	0	0	0	50	0	8	30	0	102
05:00 PM	05:15 PM	0	0	24	0	0	0	0	65	0	12	36	0	137
05:15 PM	05:30 PM	0	0	35	0	0	0	0	78	0	19	47	0	179
05:30 PM	05:45 PM	0	0	43	0	0	0	0	103	0	20	52	0	218
05:45 PM	06:00 PM	0	0	55	0	0	0	0	117	0	21	64	0	257

TOTAL BY PERIOD

04:00 PM	04:15 PM	0	0	3	0	0	0	0	5	0	1	7	0	16
04:15 PM	04:30 PM	0	0	3	0	0	0	0	18	0	3	3	0	27
04:30 PM	04:45 PM	0	0	5	0	0	0	0	9	0	2	9	0	25
04:45 PM	05:00 PM	0	0	3	0	0	0	0	18	0	2	11	0	34
05:00 PM	05:15 PM	0	0	10	0	0	0	0	15	0	4	6	0	35
05:15 PM	05:30 PM	0	0	11	0	0	0	0	13	0	7	11	0	42
05:30 PM	05:45 PM	0	0	8	0	0	0	0	25	0	1	5	0	39
05:45 PM	06:00 PM	0	0	12	0	0	0	0	14	0	1	12	0	39

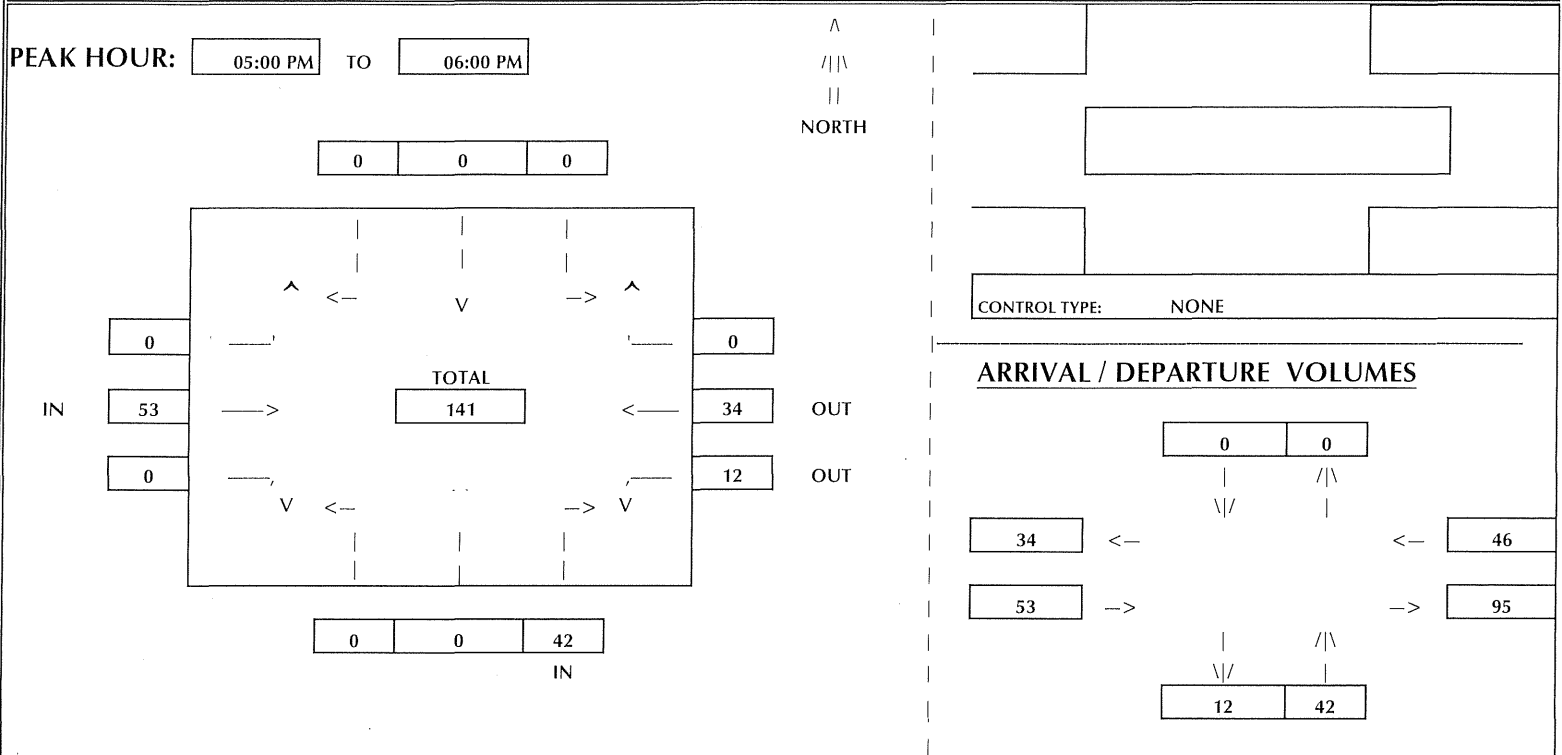
HOURLY TOTALS

04:00 PM	05:00 PM	0	0	14	0	0	0	0	50	0	8	30	0	102
04:15 PM	05:15 PM	0	0	21	0	0	0	0	60	0	11	29	0	121
04:30 PM	05:30 PM	0	0	29	0	0	0	0	55	0	15	37	0	136
04:45 PM	05:45 PM	0	0	32	0	0	0	0	71	0	14	33	0	150
05:00 PM	06:00 PM	0	0	41	0	0	0	0	67	0	13	34	0	155

ASSOCIATED TRANSPORTATION ENGINEERS

INTERSECTION TURNING MOVEMENT SUMMARY

PROJECT: WILLOW SPRINGS II **PROJECT #:** 08098 **COUNT DATE:** 10-08-08 **FILE NAME:** PM_TGEN
N-S Approach: IN/OUT **COUNT TIME:** 04:00 PM TO 6:00
E-W Approach: **CITY:** GOLETA **WEATHER:** SUNNY



TIME PERIOD	NORTHBOUND			SOUTHBOUND			EASTBOUND			WESTBOUND			TOTAL VOLUMES
	From	To	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	

COUNT DATA

04:00 PM	---	04:15 PM	0	0	1	0	0	0	0	10	0	3	2	0	16
04:15 PM	---	04:30 PM	0	0	3	0	0	0	0	20	0	7	11	0	41
04:30 PM	---	04:45 PM	0	0	5	0	0	0	0	28	0	9	16	0	58
04:45 PM	---	05:00 PM	0	0	9	0	0	0	0	42	0	16	25	0	92
05:00 PM	---	05:15 PM	0	0	16	0	0	0	0	51	0	19	33	0	119
05:15 PM	---	05:30 PM	0	0	31	0	0	0	0	72	0	23	39	0	165
05:30 PM	---	05:45 PM	0	0	41	0	0	0	0	82	0	25	49	0	197
05:45 PM	---	06:00 PM	0	0	51	0	0	0	0	95	0	28	59	0	233

TOTAL BY PERIOD

04:00 PM	---	04:15 PM	0	0	1	0	0	0	0	10	0	3	2	0	16
04:15 PM	---	04:30 PM	0	0	2	0	0	0	0	10	0	4	9	0	25
04:30 PM	---	04:45 PM	0	0	2	0	0	0	0	8	0	2	5	0	17
04:45 PM	---	05:00 PM	0	0	4	0	0	0	0	14	0	7	9	0	34
05:00 PM	---	05:15 PM	0	0	7	0	0	0	0	9	0	3	8	0	27
05:15 PM	---	05:30 PM	0	0	15	0	0	0	0	21	0	4	6	0	46
05:30 PM	---	05:45 PM	0	0	10	0	0	0	0	10	0	2	10	0	32
05:45 PM	---	06:00 PM	0	0	10	0	0	0	0	13	0	3	10	0	36

HOURLY TOTALS

04:00 PM	---	05:00 PM	0	0	9	0	0	0	0	42	0	16	25	0	92
04:15 PM	---	05:15 PM	0	0	15	0	0	0	0	41	0	16	31	0	103
04:30 PM	---	05:30 PM	0	0	28	0	0	0	0	52	0	16	28	0	124
04:45 PM	---	05:45 PM	0	0	36	0	0	0	0	54	0	16	33	0	139
05:00 PM	---	06:00 PM	0	0	42	0	0	0	0	53	0	12	34	0	141

Willowsprings Trip Generation Calculation Sheet

Count Date	Units	A.M. Volume	A.M. Rate	P.M. Volume	P.M. Rate
September 2008	235	153	0.65	155	0.66
October 2008	235	139	0.59	141	0.6

*September 2008 rates used for traffic study.

UNIGNALIZED INTERSECTION V/C INCREASE CALCULATIONS

HERITAGE RIDGE PROJECT (#12089.01)
 % IN ENTERING VOLUME CALCULATIONS

INTERSECTION: CALLE REAL/LOS CARNEROS ROAD

<u>A.M. PEAK HOUR</u>	<u>EX</u>	<u>EX+PR</u>	<u>% INCREASE</u>	<u>P.M. PEAK HOUR</u>	<u>EX</u>	<u>EX+PR</u>	<u>% INCREASE</u>
WB-L	241	243		WB-L	319	325	
WB-R	14	14		WB-R	24	24	
NB-T	118	125		NB-T	209	212	
NB-R	162	169		NB-R	396	399	
SB-L	20	20		SB-L	12	12	
SB-T	197	199		SB-T	86	92	
TOTAL:	752	770	2%	TOTAL:	1046	1064	2%

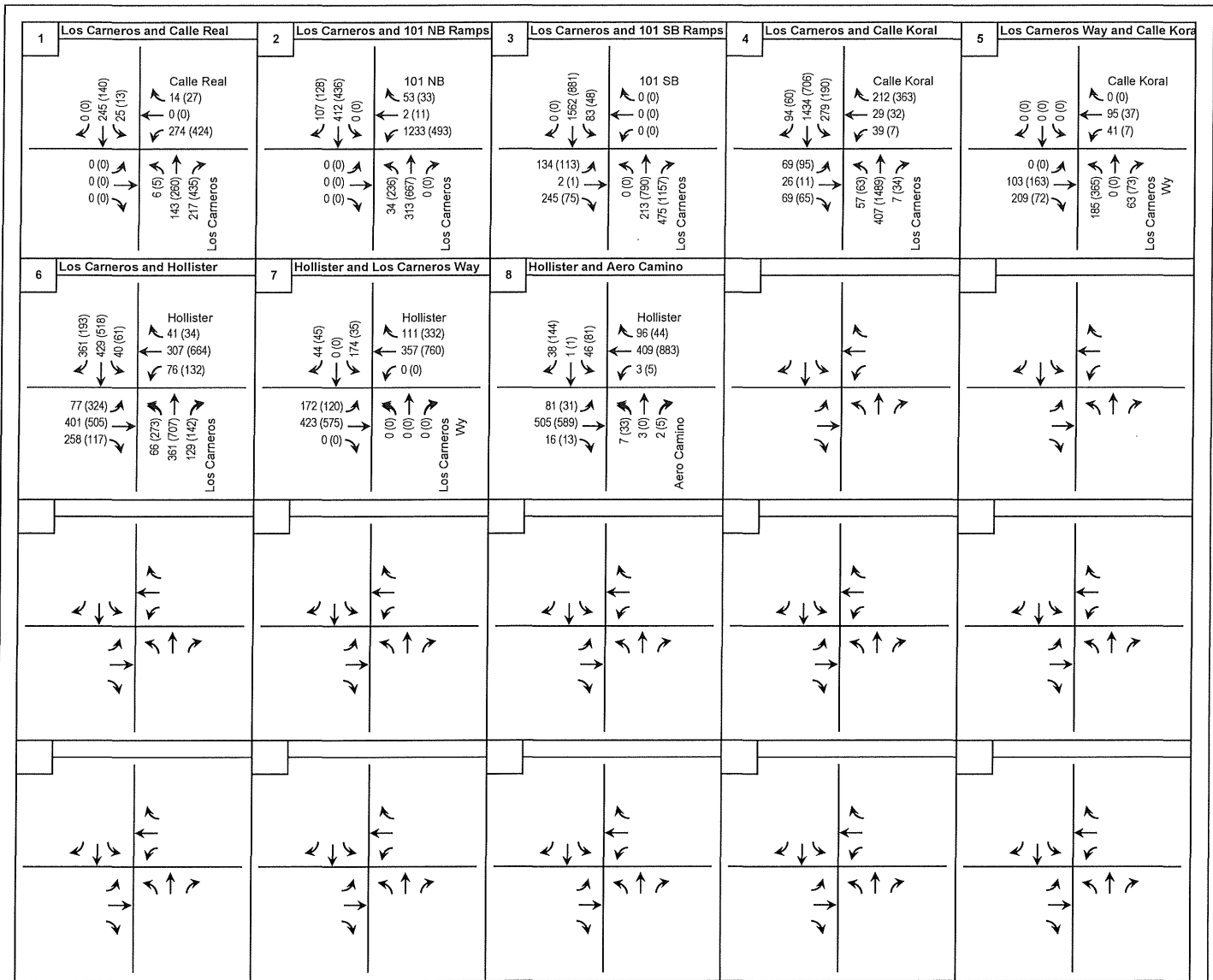
<u>A.M. PEAK HOUR</u>	<u>CU</u>	<u>CU+PR</u>	<u>% INCREASE</u>	<u>P.M. PEAK HOUR</u>	<u>CU</u>	<u>CU+PR</u>	<u>% INCREASE</u>
WB-L	274	276		WB-L	424	430	
WB-R	14	14		WB-R	27	27	
NB-T	143	152		NB-T	260	263	
NB-R	217	228		NB-R	435	442	
SB-L	25	25		SB-L	13	13	
SB-T	245	247		SB-T	140	146	
TOTAL:	918	942	3%	TOTAL:	1299	1321	2%

INTERSECTION: CALLE KORAL/LOS CARNEROS WAY

<u>A.M. PEAK HOUR</u>	<u>EX</u>	<u>EX+PR</u>	<u>% INCREASE</u>	<u>P.M. PEAK HOUR</u>	<u>EX</u>	<u>EX+PR</u>	<u>% INCREASE</u>
EB-T	36	81		EB-T	71	159	
EB-R	171	171		EB-R	42	42	
WB-L	41	66		WB-L	7	18	
WB-T	107	208		WB-T	79	122	
NB-L	40	40		NB-L	225	225	
NB-R	11	18		NB-R	43	65	
TOTAL:	406	584	44%	TOTAL:	467	631	35%

<u>A.M. PEAK HOUR</u>	<u>CU</u>	<u>CU+PR</u>	<u>% INCREASE</u>	<u>P.M. PEAK HOUR</u>	<u>CU</u>	<u>CU+PR</u>	<u>% INCREASE</u>
EB-T	103	127		EB-T	163	251	
EB-R	209	209		EB-R	72	72	
WB-L	41	66		WB-L	7	18	
WB-T	95	196		WB-T	37	80	
NB-L	185	185		NB-L	365	365	
NB-R	63	70		NB-R	73	95	
TOTAL:	696	853	23%	TOTAL:	717	881	23%

CITY OF GOLETA TRAFFIC MODEL DATA/CUMULATIVE PROJECT LIST



Key
 31 (27) = AM(PM)
 ↖ = Movement Volume



File Information
 CBS
 Excel File = volmap8.xls
 Turns (AM) = am.csv
 Turns (PM) = pm.csv
 10/9/2015 16:10
 © Kittelson & Associates

AM Peak Hour

Intersection	ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
Los Carneros and Calle Real	1	6	143	217	25	245	0	0	0	0	274	0	14
Los Carneros and 101 NB Ramps	2	34	313	0	0	412	107	0	0	0	1233	2	53
Los Carneros and 101 SB Ramps	3	0	213	475	83	1562	0	134	2	245	0	0	0
Los Carneros and Calle Koral	4	57	407	7	279	1434	94	69	26	69	39	29	212
Calle Koral and Los Carneros Way	5	185	0	63	0	0	0	0	103	209	41	95	0
Los Carneros and Hollister	6	66	361	129	40	429	361	77	401	258	76	307	41
Hollister and Los Carneros Way	7	0	0	0	174	0	44	172	423	0	0	357	111
Hollister and Aero Camino	8	7	3	2	46	1	38	81	505	16	3	409	96

PM Peak Hour

Intersection	ID	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
Los Carneros and Calle Real	1	5	260	435	13	140	0	0	0	0	424	0	27
Los Carneros and 101 NB Ramps	2	236	667	0	0	436	128	0	0	0	493	11	33
Los Carneros and 101 SB Ramps	3	0	790	1157	48	881	0	113	1	75	0	0	0
Los Carneros and Calle Koral	4	63	1489	34	190	706	60	95	11	65	7	32	363
Calle Koral and Los Carneros Way	5	365	0	73	0	0	0	0	163	72	7	37	0
Los Carneros and Hollister	6	273	707	142	61	518	193	324	505	117	132	664	34
Hollister and Los Carneros Way	7	0	0	0	35	0	45	120	575	0	0	760	332
Hollister and Aero Camino	8	33	0	5	81	1	144	31	589	13	5	883	44

City of Goleta
Cumulative Project List
Updated 9/1/15

Project	Address	APN	Land Use	Acreage	Project Description	Status
PROJECTS UNDER CONSTRUCTION						
Haskell's Landing (The Hideaway)	Hollister Avenue & Las Armas Road	079-210-049	Residential	14.23	101 residential units	Under construction
Goleta Valley Cottage Hospital	351 S. Patterson at Hollister Avenue	065-090-022; -028	Commercial	18.38	Hospital 93,090 sf Existing; 152,658 sf Approved; 59,568 sf Net New	Under construction
Cabrillo Business Park	6767 Hollister Avenue	073-450-005	Commercial	91.4	Business Park - New structures total 693,100 sf (R&D, self storage, service uses); 241,682 sf existing Pre-Development Plan; 934,800 sf total; *Under Approved Projects (Projects Under Construction), also see Discovery Self-Storage Case No.14-009-PCR, -LM and Pacific Beverage Case No. 14-070-PCR, -LLA, -OSP	Under construction
Westar (Hollister Village)	7000 Hollister Avenue (N/E corner of Glen Annie Road and Hollister)	073-030-020; -021	Residential and Commercial	23.55	266 residential units; Approx. 86,000 sf of commercial	Under construction
GVCH Medical Office Building Reconstruction	5333 Hollister Avenue	065-090-023	Commercial	2.17	Medical Office Building Demo Existing 41,224 sf; 52,000 sf Approved; 10,776 sf Net New	Under construction
Ice in Paradise	Santa Felicia Drive	073-440-022	Commercial	4.8	46,479 sf ice skating rink	Under construction
Village at Los Carneros	Adjacent to 71 South Los Carneros Road	073-330-024, -026, -027, -028, -029	Residential	43.14	465 units on 43.14 acres	Under construction
Rincon Palms Hotel (Hilton Garden Inn)	6868/6878 Hollister Avenue	073-140-004	Commercial	3.05	95,678 sf hotel; 138 rooms with meeting space	Under construction
Investec (Discovery Self-Storage) at Cabrillo Business Park	350 Coromar Drive and 6640 Discovery Drive	073-610-015, -016	Commercial	6.02	111,100 sf self-storage facility (Note: Square footage is already included within the overall Cabrillo Business Park Scope)	Under construction
Pacific Beverage at Cabrillo Business Park	SW Corner of Coromar Drive and Discovery Road	073-610-022, -023, -027, -029	Industrial	7.6	Includes a Lot Line Adjustment among 4 lots. On newly adjusted Lot 19, 2 new buildings will be constructed: a 93,780 sf office/warehouse building and a 3,200 sf truck maintenance/storage building.	Under construction

City of Goleta
Cumulative Project List
Updated 9/1/15

Project	Address	APN	Land Use	Acreage	Project Description	Status
APPROVED PROJECTS (NOT CONSTRUCTED)						
Islamic Society of SB	N/E Corner of Los Carneros and Calle Real	077-160-035	Commercial	0.59	6,183 sf building with prayer room, meeting area and 1 caretaker unit	Approved
Citrus Village	7388 Calle Real	077-490-043	Residential	1.02	10 residential units	Approved
Mariposa at Ellwood Shores	7760 Hollister Avenue	079-210-057	Commercial	2.95	62,481 sf assisted living (90 residents)	Approved
Schwann Self Storage	10 S. Kellogg Avenue	071-090-082	Industrial	2.06	111,730 sf self-storage facility	Approved
Marriott Residence Inn	6300 Hollister Avenue	073-050-020	Commercial	10.57	80,989 sf hotel (118 rooms)	Approved
Cortona Apartments	6830 Cortona Drive	073-140-016	Residential	8.82	176 residential units	Approved
Harvest Hill Ranch	880 Cambridge Drive	069-620-044	Residential	4.73	7 lot subdivision with net of 6 homes	Approved
Somera Medical Office Building	454 S. Patterson Avenue	065-090-013	Commercial	8	20,000 sf net new medical/dental office building	Approved

City of Goleta
Cumulative Project List
Updated 9/1/15

Project	Address	APN	Land Use	Acreage	Project Description	Status
PENDING PROJECTS						
Shelby	7400 Cathedral Oaks Road	077-530-019	Residential	14.38	60 residential units	Pending
Sturgeon Building	S/E Corner of Los Carneros and Calle Real	077-160-040	Commercial	0.53	6,046 sf retail/medical office	Pending (On Hold)
Kenwood Village	Calle Real w/o Calaveras Avenue	077-130-066, -019; 077-141-049	Residential	10	60 residential units	Pending
Saint George Mixed Use Project	5392 & 5400 Hollister Avenue	071-101-002; -015	Residential and Commercial	0.95	New 3-story mixed-use residential building; 4 new residential buildings with 2 units each.	Pending
Fairview Gardens	598 North Fairview Avenue	069-090-052	Agriculture	11.65	Farm Labor Camp Revision; Special Events Permit; and Sale of Ag related products grown offsite	Pending
Fuel Depot with Car Washes	370 Storke Road	073-100-008	Commercial	1	1,667 sf new drive-in carwash, self-serve car wash, gas fueling dispensers and manager's residence; Zizzo's Coffee building to remain	Pending
Old Town Industrial Center	891 S. Kellogg Avenue	071-170-074, -080, -083	Industrial	14.76	186,770 sf new Light Industrial with outdoor storage and 5,100 sf office building	Pending
Old Town Village	South Kellogg Avenue	071-130-023	Residential and Commercial	12.31	Mixed Use of 175 townhomes with shopkeeper and livework units	Pending
Heritage Ridge	North of Calle Koral and West of Los Carneros	073-060-031 thru -043	Residential	16.2	228 residential apartments and 132 senior apartments	Pending

PARKING SURVEY DATA AND DEMAND RATE CALCULATIONS

THE TOWBES GROUP

RPMD Parking Space Analysis - Occupied Spaces Equation Spreadsheet - 7/25/12

<u>Property</u>	<u>Total Spaces</u>	<u>Dates Observed</u>	<u>Vacant Visitor Spaces</u>	<u>Vacant Reserved But Unassigned</u>	<u>Occupied Spaces</u>
Cypress Meadows	111	7/14/2012	27	0	84
		7/15/2012	34	0	77
Rancho Franciscan	112	7/16/2012	6	0	106
		7/17/2012	7	0	105
Shepard Place	154	7/16/2012	10	3	141
		7/17/2012	3	3	148

THE TOWBES GROUP
RPMD PARKING SPACE ANALYSIS - 7/3/12

<u>Property</u>	<u>Total Apartments</u>	<u>Carport/Reserved Spaces</u>	<u>Open Spaces</u>	<u>Handicap Spaces</u>	<u>Total Spaces</u>	<u>Dates Observed</u>	<u>Vacant Visitor Spaces</u>	<u>Occupancy</u>	<u>Bedrooms</u>	<u>Number of Apartments</u>	<u>Parking Spaces Required Per Current City of Goleta Zoning Code</u>	<u>Notes</u>
Cypress Point	268	268	165	11	444	6/4/2012	5	96%	1 bedroom	104	104	
						6/5/2012	7		2 bedrooms	164	328	
									Visitor		54	
											486	
Ralston Courtyards	108	108	41	5	154	6/4/2012	10	94%	1 bedroom	84	84	
						6/5/2012	14		2 bedrooms	24	48	
									Visitor		22	
											154	
Encina Meadows	168	168	90	2	260	6/4/2012	38	95%	1 bedroom	103	103	
						6/5/2012	35		2 bedrooms	65	130	
									Visitor		34	
											267	
Pacific Oaks	183	189	43	3	235	6/6/2012	3	98%	1 bedroom	112	112	6 carport spaces are for rent
						6/7/2012	1		2 bedrooms	71	142	
									Visitor		37	
											291	
Patterson Place	168	169	42	1	212	6/6/2012	0	96%	1 bedroom	135	135	
						6/7/2012	0		2 bedrooms	33	66	
									Visitor		34	
											235	
Sumida Gardens	200	200	156	8	364	6/4/2012	37	98%	1 bedroom	60	60	
						6/5/2012	29		2 bedrooms	124	248	
									3 bedrooms	16	40	
									Visitor		40	
				388								
Willow Springs	235	236	248	10	494	5/2/2012	93	96%	1 bedroom	32	32	
						5/3/2012	93		2 bedrooms	147	294	
									3 bedrooms	56	140	
									Visitor		47	
				513								
Knollwood Meadows	128	182	30	8	220	6/5/2012	6	99%	1 bedroom	65	65	
						6/6/2012	3		2 bedrooms	63	126	
									Visitor		26	
											217	
Oceanwood Apartments	133	133	88	7	228	6/5/2012	30	92%	1 bedroom	80	80	
						6/6/2012	41		2 bedrooms	53	106	
									Visitor		27	
											213	

WILLOWSPRINGS PARKING SURVEY

DATE	TIME	AVAILABLE SPACES	VEHICLES			% OCCUPIED
			ON-SITE	ON-STREET	TOTAL	
12/8/2008	12:00 A.M.	449	375	7	382	85%
1/13/2009	6:00 A.M.	449	300	9	309	69%



ASSOCIATED TRANSPORTATION ENGINEERS

100 N. Hope Avenue, Suite 4, Santa Barbara, CA 93110 • (805) 687-4418 • (805) 682-8509-F

#07119 - LA SUMIDA GARDENS APARTMENTS 2010 PARKING SURVEYS

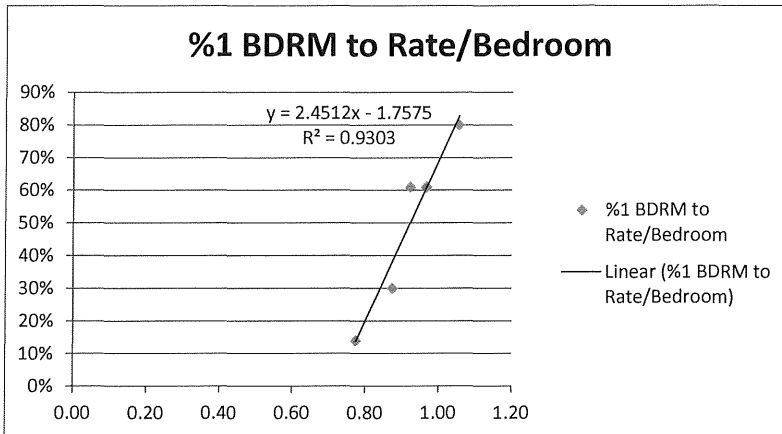
DATE	TIME	OCCUPIED SPACES	AVAILABLE SPACES	TOTAL SPACES	% OCCUPIED	VEHICLES ON-STREET
3/6/2013	12:00 AM	300	68	368	82%	7
3/7/2013	12:00 AM	301	67	368	82%	6
9/12/2012	12:00 A.M.	303	65	368	82%	8
9/13/2012	12:00 A.M.	298	70	368	81%	9
3/13/2012	12:00 AM	298	70	368	81%	8
3/14/2012	12:00 AM	301	67	368	82%	7
9/6/2011	11:00 P.M.	292	76	368	79%	6
9/7/2011	11:00 P.M.	287	81	368	78%	8
3/9/2011	11:00 P.M.	301	67	368	82%	6
3/10/2011	11:00 P.M.	286	82	368	78%	3
9/7/2010	10:30 PM	281	87	368	76%	7
9/8/2010	10:30 PM	293	75	368	80%	8

#12089.01 WILLOW SPRINGS NORTH PROJECT

GOLETA APARTMENT PARKING DEMAND RATES PER BEDROOM

SITE	TOTAL UNITS	% 1 BDRM	BEDROOMS	PEAK DEMAND	RATE PER BEDROOM
WILLOW SPRINGS I	235	14%	494	382	0.77
SUMIDA GARDENS	200	30%	356	311	0.87
ENCINA MEADOWS	168	61%	233	225	0.97
PACIFIC OAKS	183	61%	254	234	0.92
PATTERSON PLACE	168	80%	201	212	1.05

AVERAGE RATE: **1538** **1364** **0.89**



$$y = 2.4512x - 1.7575 \quad y = 0.65 \text{ or } 65\%$$

$$0.65 = 2.4512x - 1.7575$$

$$0.65 + 1.7575 = 2.4512x$$

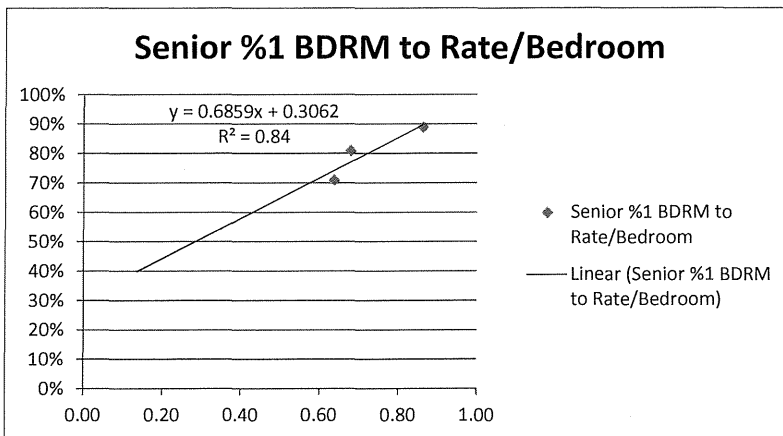
$$2.4075 = 2.4512x$$

$$x = 2.4075 / 2.4512 \quad \boxed{x = 0.98}$$

VENTURA SENIOR APARTMENT PARKING DEMAND RATES PER BEDROOM

SITE	TOTAL UNITS	% 1 BDRM	BEDROOMS	PEAK DEMAND	RATE PER BEDROOM
SHEPARD PLACE	169	71%	233	148	0.64
CYPRESS MEADOWS	104	81%	124	84	0.68
RANCHO FRANCISCAN	111	89%	123	106	0.86

AVERAGE RATE: **480** **338** **0.70**



$$y = 0.6859x + 0.3062 \quad y = 0.82 \text{ or } 82\%$$

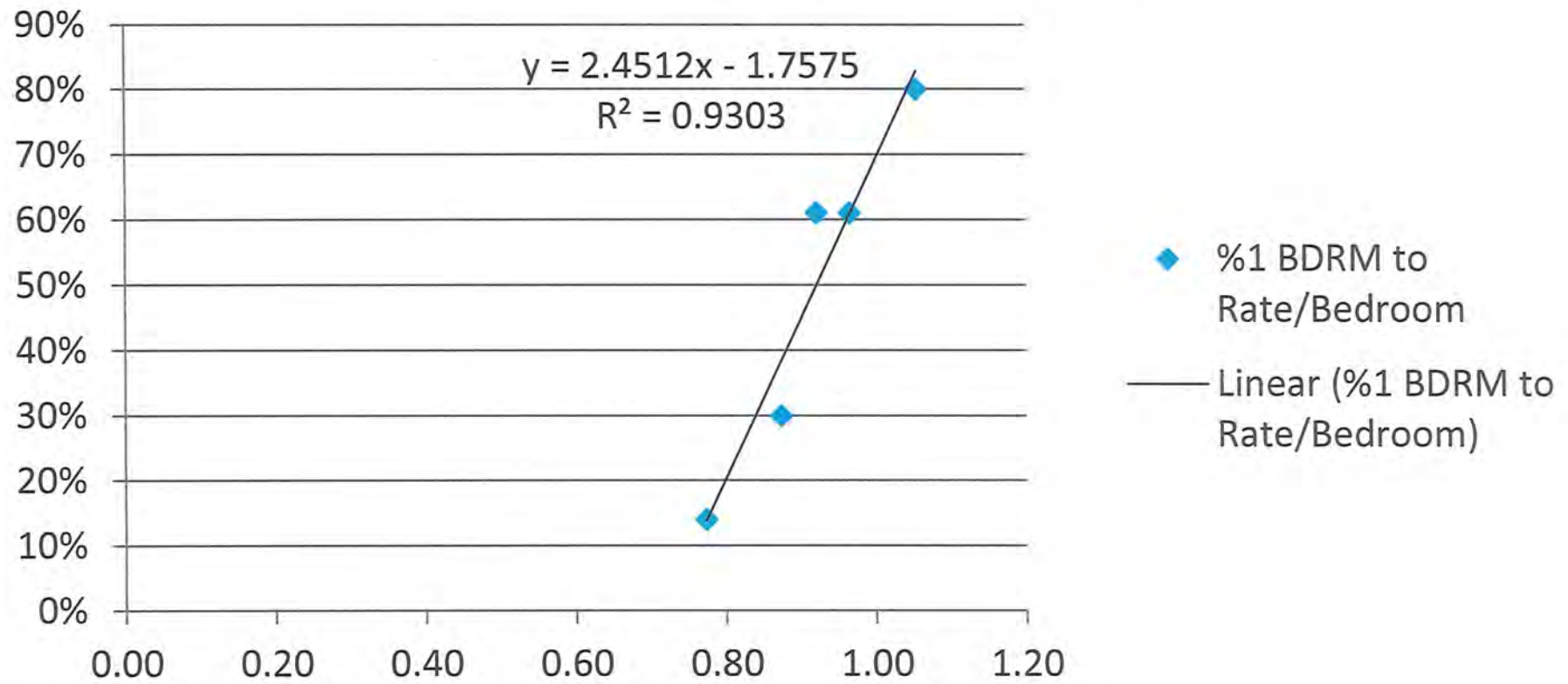
$$0.82 = 0.6859x + 0.3062$$

$$0.82 - 0.3062 = 0.6859x$$

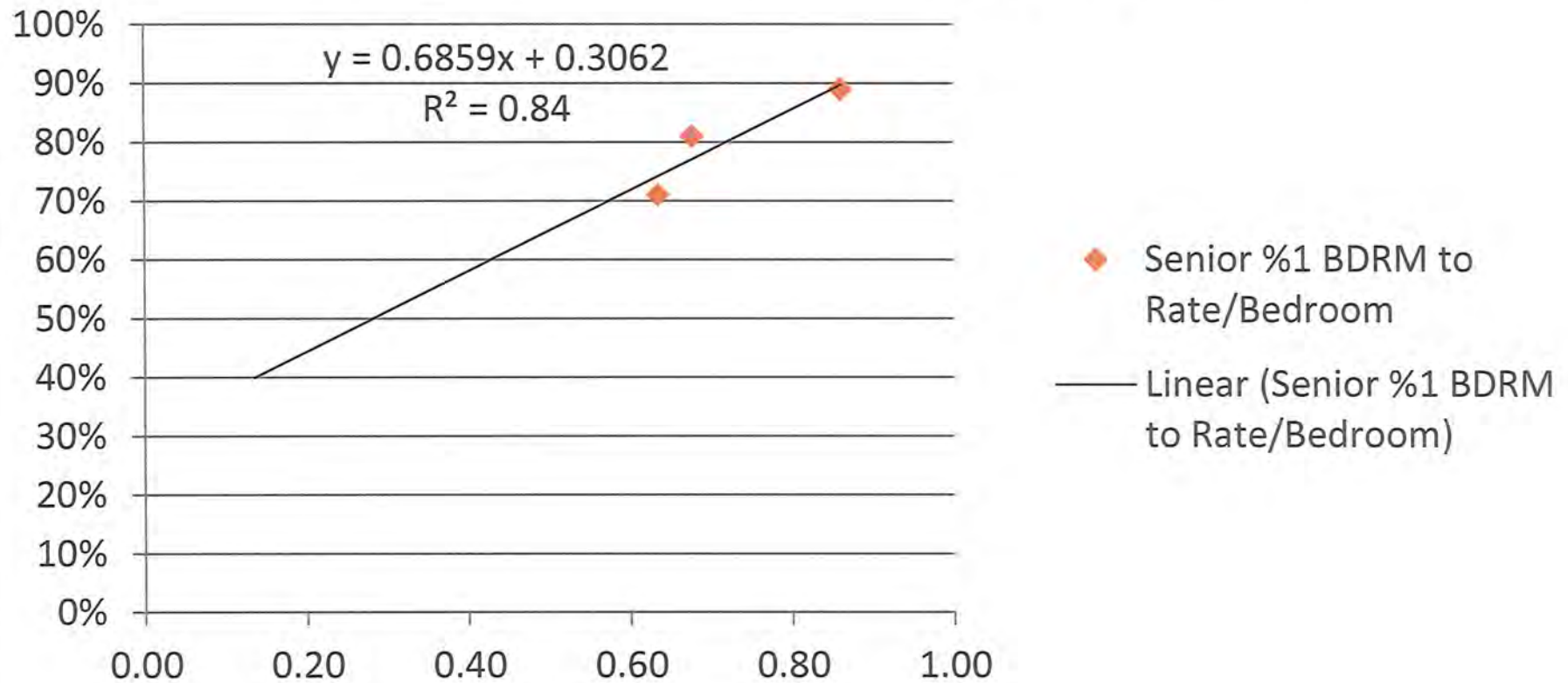
$$0.5138 = 0.6859x$$

$$x = 0.5138 / 0.6859 \quad \boxed{x = 0.75}$$

%1 BDRM to Rate/Bedroom



Senior %1 BDRM to Rate/Bedroom



SBCAG FREEWAY LOS DATA

SEGMENT	AVG. DAILY TRIPS	A.M. PEAK HOUR				P.M. PEAK HOUR				# OF LANES (ONE DIRECTION)	PHF	TERRAIN	HEAVY VEHICLES			BASE FFS	LANE WIDTH	SHOULDER CLEARANCE	INT. DENSITY
		PEAK HOUR TRIPS		DENSITY/LOS		PEAK HOUR TRIPS		DENSITY/LOS					% TRUCK	% BUS	% RV				
		NB	SB	NB	SB	NB	SB	NB	SB										
<i>South Coast</i>																			
Bates-150	65,000	3,170	910	15.6 / B	6.4 / A	1,670	2,860	8.2 / A	14.0 / B	3	0.95	Flat	0.07	0.01	0.01	70 mph	12'	6'	0.67
150-Baillard	64,000	3,120	1,040	24.8 / C	5.1 / A	1,710	2,900	12.6 / B	14.2 / B	2 NB / 3 SB	0.95	Flat	0.07	0.01	0.01	70 mph	12'	6'	0.67
Baillard-Casitas Pass	65,800	3,210	1,070	25.8 / C	7.9 / A	1,900	2,840	14.0 / B	21.9 / C	2	0.95	Flat	0.07	0.01	0.01	70 mph	12'	6'	0.67
Casitas Pass-Linden	60,000	3,150	1,050	25.2 / C	7.7 / A	1,860	2,580	13.7 / B	19.5 / C	2	0.95	Flat	0.07	0.01	0.01	70 mph	12'	6'	0.83
Linden-Santa Monica	64,800	3,410	1,130	28.3 / D	8.3 / A	2,020	2,780	14.9 / B	21.3 / C	2	0.95	Flat	0.07	0.01	0.01	70 mph	12'	6'	0.83
Santa Monica-Padaro (North)	61,400	3,330	1,150	27.3 / D	8.5 / A	2,100	2,570	15.5 / B	19.4 / C	2	0.95	Flat	0.07	0.01	0.01	70 mph	12'	6'	0.67
Padaro (North)-Padaro (South)	63,800	3,130	1,340	24.9 / C	9.8 / A	2,040	2,810	15.0 / B	21.6 / C	2	0.95	Flat	0.07	0.01	0.01	70 mph	12'	6'	0.5
Padaro (South)-Evans	64,700	3,170	1,360	25.4 / C	10.0 / A	2,070	2,850	15.3 / B	22.0 / C	2	0.95	Flat	0.07	0.01	0.01	70 mph	12'	6'	0.67
Evans-Sheffield	66,500	3,260	1,400	15.3 / B	10.3 / A	2,120	2,930	10.4 / A	22.8 / C	3 NB / 2 SB	0.95	Rolling	0.07	0.01	0.01	70 mph	12'	6'	0.67
Sheffield-San Ysidro	68,100	3,220	1,550	29.1 / D	12.3 / B	2,180	3,000	17.5 / B	26.2 / D	2	0.95	Rolling	0.07	0.01	0.01	70 mph	12'	6'	0.67
San Ysidro-Olive Mill	72,400	3,280	1,720	26.5 / D	12.6 / B	2,350	3,150	17.4 / B	25.0 / C	2	0.95	Flat	0.07	0.01	0.01	70 mph	12'	6'	0.67
Olive Mill-Cabrillo	67,300	2,590	2,120	19.2 / C	15.4 / B	2,120	2,930	15.4 / B	22.4 / C	2	0.95	Flat	0.06	0.01	0.01	70 mph	12'	6'	0.83
Cabrillo-Milpas	66,500	2,450	2,270	12.7 / B	11.7 / B	1,980	2,940	10.2 / A	15.2 / B	3	0.95	Flat	0.05	0.01	0.01	70 mph	12'	6'	1.17
Milpas-Garden	85,700	3,210	2,960	17.0 / B	15.7 / B	3,540	2,890	18.8 / C	15.3 / B	3	0.92	Flat	0.03	0.01	0.01	70 mph	12'	6'	1
Garden-Castillo	95,200	4,010	2,840	21.5 / C	15.0 / B	4,020	3,120	21.6 / C	16.5 / B	3	0.92	Flat	0.03	0.01	0.01	70 mph	12'	6'	1
Castillo-Carrillo	104,000	3,760	3,620	20.0 / C	19.2 / C	3,900	3,900	20.8 / C	20.8 / C	3	0.92	Flat	0.03	0.01	0.01	70 mph	12'	6'	1
Carrillo-Mission	119,000	4,380	4,550	23.9 / C	25.0 / C	4,760	4,760	26.6 / D	26.6 / D	3	0.92	Flat	0.03	0.01	0.01	70 mph	12'	6'	1.17
Mission-Las Positas	136,000	5,020	5,310	28.6 / D	31.1 / D	5,490	5,600	32.8 / D	33.9 / D	3	0.92	Flat	0.02	0.01	0.01	70 mph	12'	6'	1.3
Las Positas-La Cumbre	130,000	5,330	5,330	31.1 / D	31.1 / D	5,530	5,520	32.9 / D	32.8 / D	3	0.92	Flat	0.02	0.01	0.01	70 mph	12'	6'	1.17
La Cumbre-154	127,000	5,300	5,080	20.2 / C	19.2 / C	5,660	5,510	21.9 / C	21.2 / C	4	0.92	Flat	0.02	0.01	0.01	70 mph	12'	6'	0.83
154-El Sueno	119,000	4,880	4,880	26.5 / D	26.5 / D	5,060	5,060	27.9 / D	27.9 / D	3	0.92	Flat	0.02	0.01	0.01	70 mph	12'	6'	0.83
El Sueno-Turnpike	116,000	4,550	4,730	24.0 / C	25.3 / C	4,440	5,420	23.2 / C	31.2 / D	3	0.92	Flat	0.02	0.01	0.01	70 mph	12'	6'	0.83
Turnpike-Rte 217	112,000	4,140	4,480	21.2 / C	23.5 / C	3,720	5,580	18.7 / C	32.8 / D	3	0.92	Flat	0.02	0.01	0.01	70 mph	12'	6'	0.83
Rte 217-Fairview	79,200	3,030	3,150	15.0 / B	15.6 / B	3,030	3,460	15.0 / B	17.3 / B	3	0.92	Flat	0.03	0.01	0.01	70 mph	12'	6'	0.83
Fairview-Los Carneros	71,100	2,510	2,820	19.2 / C	22.2 / C	2,920	2,910	23.2 / C	23.1 / C	2	0.92	Flat	0.05	0.01	0.01	70 mph	12'	6'	0.83
Los Carneros-Storke	63,000	1,640	2,700	12.4 / B	13.6 / B	2,920	2,210	23.5 / C	11.1 / B	2 NB / 3 SB	0.92	Flat	0.07	0.01	0.01	70 mph	12'	6'	0.83
Storke-Hollister	34,700	730	1,350	5.5 / A	10.2 / A	1,670	1,110	12.6 / B	8.4 / A	2	0.92	Flat	0.08	0.01	0.01	75 mph	12'	6'	0.5
Hollister-El Capitan	31,000					1,570	850	13.8 / B	7.5 / A	2	0.88	Rolling	0.09	0.01	0.01	75 mph	12'	6'	0.29
El Capitan-Rte. 1	29,700					1,520	730	13.2 / B	6.3 / A	2	0.88	Rolling	0.09	0.01	0.01	75 mph	12'	6'	0.13
<i>North County</i>																			
Rte. 1-La Lata Pl.	22,200					1,010	670	7.2 / A	7.2 / A	3 NB / 2 SB	0.88	Mountainous	0.11	0.01	0.01	75 mph	12'	6'	0.29
La Lata Pl.-Santa Rosa	22,200					1,010	670	11.1 / B	4.9 / A	2 NB / 3 SB	0.88	Mountainous	0.12	0.01	0.01	75 mph	12'	6'	0.29
Santa Rosa-Rte. 246	20,300					930	620	8.4 / A	5.6 / A	2	0.88	Rolling	0.12	0.01	0.01	75 mph	12'	6'	0.5
Rte 246-N. Buellton I/C	22,900					820	540	7.4 / A	4.9 / A	2	0.88	Rolling	0.12	0.01	0.01	75 mph	12'	6'	0.5
N. Buellton I/C-Rte. 154	27,900					870	670	7.8 / A	6.0 / A	2	0.88	Rolling	0.12	0.01	0.01	75 mph	12'	6'	0.2
Rte. 154-Rte. 135	26,400					1,050	850	9.2 / A	7.5 / A	2	0.88	Rolling	0.1	0.01	0.01	75 mph	12'	6'	0.24
Rte. 135-Clark	36,500					1,000	820	8.9 / A	7.3 / A	2	0.88	Rolling	0.11	0.01	0.01	75 mph	12'	6'	0.185
Clark-Santa Maria Way	43,000					1,380	1,180	11.1 / B	9.5 / A	2	0.92	Rolling	0.07	0.01	0.01	75 mph	12'	6'	0.33
Santa Maria Way-Betteravia	54,100					1,650	1,400	8.9 / A	7.5 / A	3	0.92	Rolling	0.07	0.01	0.01	70 mph	12'	6'	0.5
Betteravia-Stowell	59,300					2,070	1,830	10.3 / A	9.1 / A	3	0.92	Flat	0.05	0.01	0.01	70 mph	12'	6'	0.83
Stowell-Rte. 166	57,400					2,290	2,040	11.4 / B	10.1 / A	3	0.92	Flat	0.05	0.01	0.01	70 mph	12'	6'	0.83
Rte. 166-Donovan	54,300					2,210	2,040	11.0 / A	10.1 / A	3	0.92	Flat	0.05	0.01	0.01	70 mph	12'	6'	0.83
Donovan-Rte. 135	61,300					2,120	1,950	10.5 / A	9.7 / A	3	0.92	Flat	0.05	0.01	0.01	70 mph	12'	6'	0.83
Rte. 135-SB/SLO County Line	65,000					2,350	2,250	17.7 / B	16.9 / B	2	0.92	Flat	0.05	0.01	0.01	70 mph	12'	6'	0.67

MEMORANDUM



To: Chris Bersbach
Rincon Consultants, Inc. Date: November 30, 2015

From: Clare Look-Jaeger, P.E. *CL-Jaeger* LLG Ref: 1-15-4119-1
Alfred Ying, P.E., PTP *ACY*
LLG, Engineers

Subject: Pre-Construction Soil Removal Phase Traffic Impact Analysis –
Heritage Ridge Project, Goleta, California

Engineers & Planners
Traffic
Transportation
Parking

Linscott, Law &
Greenspan, Engineers

600 S. Lake Avenue
Suite 500
Pasadena, CA 91106

626.796.2322 T

626.792.0941 F

www.llgengineers.com

Pasadena
Irvine
San Diego
Woodland Hills

This memorandum has been prepared by Linscott, Law & Greenspan, Engineers (LLG) to summarize the truck traffic analysis during the pre-construction soil removal phase of the proposed Heritage Ridge project. The proposed project site is located on the northwest side of Camino Vista north of Calle Koral in the City of Goleta, California. The proposed project consists of the development of 132 senior apartment units, 228 apartment units, and a two-acre park. The project site is currently vacant. Vehicular access to the proposed project site is planned to be provided via three driveways on Camino Vista. A total of 510 parking spaces would be provided by the project. This would include 152 parking spaces for the senior apartments, 345 parking spaces for the apartments, and 13 parking spaces for the park. The Updated Traffic, Circulation and Parking Study, prepared by Associated Transportation Engineers (ATE), dated October 30, 2015, has been submitted to the City of Goleta.

The following sections summarize a review of potential traffic impacts during the proposed project's pre-construction soil removal phase. Based on information provided in the project applicant's grading plan, the amount of stockpiled dirt on-site totals 293,100 cubic yards (cy). Of the 293,100 cy, a total of 115,000 cy of soil is expected to be exported off-site prior to the construction of the proposed project.

Project Trip Generation

Traffic generation is expressed in vehicle trip ends, defined as one-way vehicular movements, either entering or exiting the generating land use. Trip generation equations and/or rates provided in the ITE *Trip Generation Manual* publication¹ were not utilized to forecast traffic generation during the soil removal phase, as the ITE document does not contain trip rates for specifically this type of expected short term hauling operation. Therefore, the project trip generation forecast was derived based on the development of employee and truck forecasts given the expected hauling capacities as well as the application of passenger car equivalency (PCE) factors, as described more fully below.

Traffic volumes expected to be generated by the haul trucks during the pre-construction soil removal phase were based on coordination with City of Goleta staff. The forecast of employee vehicle trips was provided and accounts for the employees that will be on-site during the soil removal phase. Three inbound trips during the AM

¹ *Trip Generation Manual*, Institute of Transportation Engineers, 9th Edition, 2012.

peak hour and three outbound trips during the PM peak hour are anticipated for the employees.

In developing the forecast of truck trip generation, several factors were taken into consideration:

- Hours of Hauling Operation (8:00 AM to 5:00 PM on weekdays)
- Capacity of Haul Trucks (9 or 20 cubic yards per truck)
- Application of PCE Factors (2.0 or 3.0)
- Amount of Anticipated Export (115,000 cy)

Based on information provided by the City, a total of 12 inbound and 12 outbound truck trips per hour are expected using trucks with a hauling capacity of 9 cubic yards while a total of six inbound and six outbound truck trips per hour are expected using trucks with a 20 cubic yard hauling capacity. In order to account for the affect that trucks have on overall intersection operations, Passenger Car Equivalency (PCE) factors were accounted for in the analysis of potential short-term traffic impacts. Based on a review of the size of haul trucks expected to be utilized, a PCE factor of 2.0 or 3.0 was incorporated into the traffic analysis (i.e., it is assumed that a single 9 cubic yard haul truck has the same overall affect on traffic operations as two passenger cars and that a single 20 cubic yard haul truck has the same overall affect on traffic operations as three passenger cars). This assumption is conservative and accounts for the heavy vehicle type and slower speeds when fully loaded. The traffic generation forecast for the proposed project is summarized in *Table 1*.

As presented in *Table 1*, using haul trucks with a capacity of 9 cubic yards, the proposed short-term soil removal phase is expected to generate 51 PCE-adjusted vehicle trips (27 inbound trips and 24 outbound trips) during the weekday AM peak hour, 51 PCE-adjusted vehicle trips (24 inbound trips and 27 outbound trips) during the weekday PM peak hour, and 390 PCE-adjusted daily trip ends during a typical weekday (195 inbound trips and 195 outbound trips). As presented in *Table 1*, using haul trucks with a capacity of 20 cubic yards, the proposed short-term soil removal phase is expected to generate 39 PCE-adjusted vehicle trips (21 inbound trips and 18 outbound trips) during the weekday AM peak hour, 39 PCE-adjusted vehicle trips (18 inbound trips and 21 outbound trips) during the weekday PM peak hour, and 294 PCE-adjusted daily trip ends during a typical weekday (147 inbound trips and 147 outbound trips). By comparison, using haul trucks with a capacity of 9 cubic yards will provide a slightly higher and more conservative assessment of potential project trip generation.

Project Trip Distribution and Assignment

Haul trucks are anticipated to utilize the US 101 Freeway to access the project site via the Los Carneros Road interchange onto Calle Koral and Camino Vista. Based on information provided by the City, 50% of the haul trucks will access the US 101

Table 1
PROJECT TRIP GENERATION DURING PRE-CONSTRUCTION SOIL REMOVAL PHASE [1]

LAND USE	DAILY TRIP ENDS [2]	AM PEAK HOUR VOLUMES [2]			PM PEAK HOUR VOLUMES [2]		
	VOLUMES	IN	OUT	TOTAL	IN	OUT	TOTAL
[A] Employees	6	3	0	3	0	3	3
[B] 9-cy Truck Trips (unadjusted) [3]	192	12	12	24	12	12	24
[C] 20-cy Truck Trips (unadjusted) [4]	96	6	6	12	6	6	12
[D] PCE Adjusted 9-cy Truck Trips [5]	384	24	24	48	24	24	48
[E] PCE Adjusted 20-cy Truck Trips [6]	288	18	18	36	18	18	36
Net Increase with 9-CY Trucks ([A]+[D])	390	27	24	51	24	27	51
Net Increase with 20-CY Trucks ([A]+[E])	294	21	18	39	18	21	39

[1] Source: Based on coordination with City of Goleta and Rincon Consultants, Inc. (a total of 115,000 cubic yard [cy] of soil to be exported).

[2] Trips are one-way traffic movements, entering or leaving.

[3] Peak hour and daily truck trips were derived based on the following, using 9-cy capacity per haul truck:

12 trucks inbound / 12 trucks outbound per hour between 8:00 am and 5:00 pm on weekdays.

Daily Truck Trips = 24 Peak Hour Truck Trips x 8 hours = 192 total truck trips per day (i.e., 96 inbound trips + 96 outbound trips).

[4] Peak hour and daily truck trips were derived based on the following, using 20-cy capacity per haul truck:

6 trucks inbound / 6 trucks outbound per hour between 8:00 am and 5:00 pm on weekdays.

Daily Truck Trips = 12 Peak Hour Truck Trips x 8 hours = 96 total truck trips per day (i.e., 48 inbound trips + 48 outbound trips).

[5] A passenger car equivalency (PCE) factor of 2.0 was employed for analysis purposes. This accounts for the assumption that a single 9 cubic yard capacity haul truck has the same overall affect on intersection traffic operations as 2.0 passenger cars.

[6] A passenger car equivalency (PCE) factor of 3.0 was employed for analysis purposes. This accounts for the assumption that a single 20 cubic yard capacity haul truck has the same overall affect on intersection traffic operations as 3.0 passenger cars.

Freeway to/from the south while the remaining 50% of the haul trucks will access the US 101 Freeway to/from the north. The following study intersections have been identified for evaluation (note: the intersection numbers/references coincide with the ATE study):

2. Los Carneros Road/US 101 Northbound Ramps
3. Los Carneros Road/US 101 Southbound Ramps
4. Los Carneros Road/Calle Koral
5. Los Carneros Way/Calle Koral

Based on a review of the existing traffic count data included in the ATE study, the highest one-hour total of overall traffic volumes traversing through the above study intersections generally occurs between 7:45 AM and 8:45 AM during the morning commute peak period and between 4:45 PM and 5:45 PM during the afternoon commute peak period. As stated previously, the anticipated hours of truck hauling operations during the pre-construction soil removal phase will begin at 8:00 AM and end at 5:00 PM on weekdays. Therefore, this traffic analysis provides a conservative assessment of potential impacts since only a portion of the truck hauling activities will overlap with the actual 7:45-8:45 AM and 4:45-5:45 PM commute peak hours.

Traffic Analysis

The traffic impact analyses were prepared for the study intersections using the Intersection Capacity Utilization (ICU) and the Highway Capacity Manual (HCM) methodologies as well as the City of Goleta's significant traffic impact criteria, consistent with the ATE study. Additionally, the existing and cumulative without project AM and PM peak hour traffic volumes were obtained from the ATE study. **Tables 2** and **3** summarize the traffic analysis assuming utilization of haul trucks with a capacity of 9 cubic yards for the existing and cumulative analysis conditions, respectively. **Tables 4** and **5** summarize the traffic analysis assuming utilization of haul trucks with a capacity of 20 cubic yards for the existing and cumulative analysis conditions, respectively. The corresponding weekday AM and PM peak hour level of service data worksheets are contained in **Attachment A**.

Existing With Project Conditions

As shown in column [2] of **Table 2** (which assumes utilization of 9-cy haul trucks) and column [2] of **Table 4** (which assumes utilization of 20-cy haul trucks), application of the City of Goleta's threshold criteria to the respective "Existing With Project" scenarios indicates that the pre-construction soil removal phase is not expected to create any short-term/temporary significant traffic impacts at the four study intersections. Incremental but less than significant impacts are noted at the intersections.

Table 2
SUMMARY OF VOLUME TO CAPACITY RATIOS AND LEVELS OF SERVICE
AM AND PM PEAK HOURS
 Pre-Construction Soil Removal Phase Using 9 Cubic-Yard Haul Trucks

NO.	INTERSECTION	PEAK HOUR	[1]		[2]				
			EXISTING V/C or Delay	LOS	EXISTING W/ PROJECT V/C or Delay	LOS	CHANGE IN V/C [(2)-(1)]	ADDED PROJECT TRIPS	PROJECT IMPACT
2	Los Carneros Road/ US-101 NB Ramps	AM	0.54	A	0.56	A	0.012	26	NO
		PM	0.49	A	0.50	A	0.013	26	NO
3	Los Carneros Road/ US-101 SB Ramps	AM	0.55	A	0.55	A	0.004	51	NO
		PM	0.72	C	0.73	C	0.009	51	NO
4	Los Carneros Road/ Calle Koral	AM	0.46	A	0.47	A	0.010	51	NO
		PM	0.51	A	0.54	A	0.032	51	NO
5	Los Carneros Way/ Calle Koral [a]	AM	8.4	A	8.5	A	0.000	51	NO
		PM	10.5	B	10.7	B	0.015	51	NO
		AM	0.25		0.25				
		PM	0.31		0.33				

[a] Two-Way Stop Control intersection.

Table 3
SUMMARY OF VOLUME TO CAPACITY RATIOS AND LEVELS OF SERVICE
AM AND PM PEAK HOURS
 Pre-Construction Soil Removal Phase Using 9 Cubic-Yard Haul Trucks

NO.	INTERSECTION	PEAK HOUR	[1]		[2]			CUMUL. IMPACT
			CUMULATIVE W/O PROJECT V/C or Delay	LOS	CUMULATIVE WITH PROJECT V/C or Delay	LOS	CHANGE IN V/C [(2)-(1)]	
2	Los Carneros Road/ US-101 NB Ramps	AM	0.68	B	0.69	B	0.012	NO
		PM	0.57	A	0.58	A	0.013	NO
3	Los Carneros Road/ US-101 SB Ramps	AM	0.67	B	0.68	B	0.005	NO
		PM	0.84	D	0.85	D	0.008	NO
4	Los Carneros Road/ Calle Koral	AM	0.70	B	0.70	B	0.000	NO
		PM	0.66	B	0.69	B	0.032	NO
5	Los Carneros Way/ Calle Koral [a]	AM	11.0	B	11.3	B	0.000	NO
		PM	14.8	B	15.8	C	0.015	NO
		AM	0.40		0.40			
		PM	0.48		0.49			

[a] Two-Way Stop Control intersection.

Table 4
SUMMARY OF VOLUME TO CAPACITY RATIOS AND LEVELS OF SERVICE
AM AND PM PEAK HOURS
 Pre-Construction Soil Removal Phase Using 20 Cubic-Yard Haul Trucks

NO.	INTERSECTION	PEAK HOUR	[1]		[2]				
			EXISTING V/C or Delay	LOS	EXISTING W/ PROJECT V/C or Delay	LOS	CHANGE IN V/C [(2)-(1)]	ADDED PROJECT TRIPS	PROJECT IMPACT
2	Los Carneros Road/ US-101 NB Ramps	AM	0.54	A	0.55	A	0.009	20	NO
		PM	0.49	A	0.50	A	0.010	20	NO
3	Los Carneros Road/ US-101 SB Ramps	AM	0.55	A	0.55	A	0.003	39	NO
		PM	0.72	C	0.72	C	0.007	39	NO
4	Los Carneros Road/ Calle Koral	AM	0.46	A	0.47	A	0.006	39	NO
		PM	0.51	A	0.54	A	0.024	39	NO
5	Los Carneros Way/ Calle Koral [a]	AM	8.4	A	8.5	A	0.000	39	NO
		PM	10.5	B	10.6	B	0.011	39	NO
		AM	0.25		0.25				
		PM	0.31		0.33				

[a] Two-Way Stop Control intersection.

Table 5
SUMMARY OF VOLUME TO CAPACITY RATIOS AND LEVELS OF SERVICE
AM AND PM PEAK HOURS
 Pre-Construction Soil Removal Phase Using 20 Cubic-Yard Haul Trucks

NO.	INTERSECTION	PEAK HOUR	[1]		[2]			CUMUL. IMPACT
			CUMULATIVE W/O PROJECT V/C or Delay	LOS	CUMULATIVE WITH PROJECT V/C or Delay	LOS	CHANGE IN V/C [(2)-(1)]	
2	Los Carneros Road/ US-101 NB Ramps	AM	0.68	B	0.69	B	0.009	NO
		PM	0.57	A	0.58	A	0.010	NO
3	Los Carneros Road/ US-101 SB Ramps	AM	0.67	B	0.68	B	0.004	NO
		PM	0.84	D	0.84	D	0.006	NO
4	Los Carneros Road/ Calle Koral	AM	0.70	B	0.70	B	0.000	NO
		PM	0.66	B	0.68	B	0.024	NO
5	Los Carneros Way/ Calle Koral [a]	AM	11.0	B	11.3	B	0.000	NO
		PM	14.8	B	15.5	C	0.011	NO
		AM	0.40		0.40			
		PM	0.48		0.49			

[a] Two-Way Stop Control intersection.

For informational purposes, a supplemental analysis was also prepared to evaluate the Los Carneros Road/Calle Koral intersection by incorporating the traffic associated with the Village at Los Carneros project and the construction of the west leg of this intersection. **Table 6** summarizes the corresponding traffic analyses for the existing and existing with project conditions. The corresponding weekday AM and PM peak hour level of service data worksheets are also contained in *Attachment A*. As shown in column [2] of *Table 6* (which assumes utilization of 9-cy haul trucks) and column [3] of *Table 6* (which assumes utilization of 20-cy haul trucks), application of the City of Goleta's threshold criteria to the respective "Existing With Project" scenarios indicates that the pre-construction soil removal phase is not expected to create any short-term/temporary significant traffic impacts at the Los Carneros Road/Calle Koral intersection. Incremental but less than significant impacts are noted at the intersection.

Cumulative With Project Conditions

As shown in column [2] of *Table 3* (which assumes utilization of 9-cy haul trucks) and column [2] of *Table 5* (which assumes utilization of 20-cy haul trucks), application of the City of Goleta's threshold criteria to the respective "Cumulative With Project" scenarios indicates that the pre-construction soil removal phase is not expected to create any short-term/temporary significant contribution to cumulative impacts at the four study intersections. Incremental but less than significant impacts are noted at the study intersections under the respective cumulative with project conditions.

Potential Use of Aero Camino During the Pre-Construction Soil Removal Phase

Based on the recommended traffic distribution pattern provided previously by the City as well as LLG's review of the surrounding roadway system, haul trucks associated with the pre-construction soil removal operation have been distributed and assigned to utilize Camino Vista, Calle Koral, and Los Carneros Road for access to and from the US 101 Freeway. As these roadways offer the most direct path of travel between the project site and the US 101 Freeway, use of Aero Camino east of the project site for haul truck access is not expected. Nonetheless, it is recommended that the truck operators be notified such that all haul trucks associated with the pre-construction soil removal phase be restricted from using Aero Camino for access.

Please feel free to contact us at 626.796.2322, with any questions or comments regarding the pre-construction soil removal phase traffic impact analyses prepared and summarized in this memorandum.

c: File

Table 6
 SUMMARY OF VOLUME TO CAPACITY RATIOS AND LEVELS OF SERVICE - AM AND PM PEAK HOURS
 ANALYSIS OF LOS CARNEROS ROAD/CALLE KORAL INTERSECTION INCLUDING THE WEST LEG
 Pre-Construction Soil Removal Phase

NO.	INTERSECTION	PEAK HOUR	[1]		[2]				[3]					
			EXISTING V/C	LOS	EXISTING W/ PROJECT (Utilizing 9-CY Capacity Trucks) V/C	LOS	CHANGE IN V/C [(2)-(1)]	ADDED PROJECT TRIPS	PROJECT IMPACT	EXISTING W/ PROJECT (Utilizing 20-CY Capacity Trucks) V/C	LOS	CHANGE IN V/C [(3)-(1)]	ADDED PROJECT TRIPS	PROJECT IMPACT
4	Los Carneros Road/ Calle Koral [a]	AM	0.59	A	0.59	A	0.000	51	NO	0.59	A	0.000	39	NO
		PM	0.57	A	0.60	A	0.032	51	NO	0.59	A	0.024	39	NO

[a] Based on input provided by the City, for informational purposes the Existing and Existing With Project analysis conditions also include traffic associated with the completion/occupancy of the Village at Los Carneros Project and the construction of the west leg of this intersection.

ATTACHMENT A

**ICU DATA WORKSHEETS
WEEKDAY AM AND PM PEAK HOURS**

**HCS DATA WORKSHEETS
WEEKDAY AM AND PM PEAK HOURS**

LINSCOTT, LAW & GREENSPAN, ENGINEERS
 600 S. Lake Avenue, Ste 500, Pasadena 91106
 (626) 796.2322 Fax (626) 792.0941

INTERSECTION CAPACITY UTILIZATION

Los Carneros Road @ US-101 NB Ramps
 Peak hr: AM

Date: 11/25/2015
 Date of Count: 2014

Utilizing trucks with 9-cubic yard capacity

N-S St: Los Carneros Road
 E-W St: US-101 NB Ramps
 Project: Heritage Ridge Residential Project/1-15-4119-1
 File: ICU2

Movement	EXIST. TRAFFIC			EXIST. WITH PROJECT				EXIST. W/PROJECT + MITIGATION				CUMULATIVE W/O PROJECT				CUMULATIVE W/ PROJECT			
	1	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C
	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio
Nb Left	21	1600	0.013 *	12	33	1600	0.021 *	0	33	1600	0.021 *	13	34	1600	0.021 *	12	46	1600	0.029 *
Nb Thru	276	3200	0.086	0	276	3200	0.086	0	276	3200	0.086	37	313	3200	0.098	0	313	3200	0.098
Nb Right	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-
Sb Left	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000
Sb Thru	423	3200	0.165 *	0	423	3200	0.165 *	0	423	3200	0.165 *	-11	412	3200	0.161 *	0	412	3200	0.161 *
Sb Right [3]	106	0	-	0	106	0	-	0	106	0	-	-3	103	0	-	0	103	0	-
Eb Left	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *
Eb Thru	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000
Eb Right	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-
Wb Left	805	0	0.252	14	819	0	0.256	0	819	0	0.256	428	1233	0	0.385	14	1247	0	0.390
Wb Thru	2	3200	0.265 *	0	2	3200	0.269 *	0	2	3200	0.269 *	0	2	3200	0.398 *	0	2	3200	0.403 *
Wb Right [4]	40	0	-	0	40	0	-	0	40	0	-	0	40	0	-	0	40	0	-
Yellow Allowance:			0.100 *					0.100 *					0.100 *					0.100 *	
ICU	0.543			0.555				0.555				0.681				0.693			
LOS	A			A				A				B				B			

02:18 PM

* Key conflicting movement as a part of ICU

- Counts conducted by Quality Counts. Existing and cumulative without project traffic volumes obtained from the Heritage Ridge Project Updated Traffic Circulation and Parking Study (prepared by ATE, 10-30-15).
- Capacity expressed in veh/hour of green
- 4% of southbound right-turns were made as Right-Turn-On-Red and have therefore been excluded from the movement volume and corresponding capacity calculations.
- 25% of westbound right-turns were made as Right-Turn-On-Red and have therefore been excluded from the movement volume and corresponding capacity calculations.

LINSCOTT, LAW & GREENSPAN, ENGINEERS
 600 S. Lake Avenue, Ste 500, Pasadena 91106
 (626) 796.2322 Fax (626) 792.0941

INTERSECTION CAPACITY UTILIZATION

Los Carneros Road @ US-101 NB Ramps
 Peak hr: PM

Date: 11/25/2015
 Date of Count: 2014

N-S St: Los Carneros Road
 E-W St: US-101 NB Ramps
 Project: Heritage Ridge Residential Project/1-15-4119-1
 File: ICU2

Utilizing trucks with 9-cubic yard capacity

Movement	EXIST. TRAFFIC			EXIST. WITH PROJECT				EXIST. W/PROJECT + MITIGATION				CUMULATIVE W/O PROJECT				CUMULATIVE W/ PROJECT						
	Volume	Capacity	V/C	Added Volume	Total Volume	Capacity	V/C	Added Volume	Total Volume	Capacity	V/C	Added Volume	Total Volume	Capacity	V/C	Added Volume	Total Volume	Capacity	V/C			
Nb Left	236	1600	0.148 *	14	250	1600	0.156 *	0	250	1600	0.156 *	0	236	1600	0.148 *	14	250	1600	0.156 *			
Nb Thru	618	3200	0.193	0	618	3200	0.193	0	618	3200	0.193	49	667	3200	0.208	0	667	3200	0.208			
Nb Right	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-			
Sb Left	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000			
Sb Thru	250	3200	0.100 *	0	250	3200	0.100 *	0	250	3200	0.100 *	186	436	3200	0.163 *	0	436	3200	0.163 *			
Sb Right [3]	71	0	-	0	71	0	-	0	71	0	-	13	84	0	-	0	84	0	-			
Eb Left	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *			
Eb Thru	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000			
Eb Right	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-			
Wb Left	436	0	0.136	12	448	0	0.140	0	448	0	0.140	57	493	0	0.154	12	505	0	0.158			
Wb Thru	7	3200	0.141 *	0	7	3200	0.145 *	0	7	3200	0.145 *	4	11	3200	0.161 *	0	11	3200	0.165 *			
Wb Right [4]	9	0	-	0	9	0	-	0	9	0	-	3	12	0	-	0	12	0	-			
Yellow Allowance:			0.100 *					0.100 *					0.100 *					0.100 *				
ICU	0.489			0.502				0.502				0.571				0.584						
LOS	A			A				A				A				A						

02:18 PM

* Key conflicting movement as a part of ICU

- Counts conducted by National Data & Surveying Services. Existing and cumulative w/o project volumes obtained from Heritage Ridge Project Updated Traffic Circulation and Parking Study (by ATE, 10-30-15).
- Capacity expressed in veh/hour of green
- 34% of southbound right-turns were made as Right-Turn-On-Red and have therefore been excluded from the movement volume and corresponding capacity calculations.
- 63% of westbound right-turns were made as Right-Turn-On-Red and have therefore been excluded from the movement volume and corresponding capacity calculations.

LINSCOTT, LAW & GREENSPAN, ENGINEERS
 600 S. Lake Avenue, Ste 500, Pasadena 91106
 (626) 796.2322 Fax (626) 792.0941

INTERSECTION CAPACITY UTILIZATION

Los Carneros Road @ US-101 SB Ramps
 Peak hr: AM

Date: 11/25/2015
 Date of Count: 2014

N-S St: Los Carneros Road
 E-W St: US-101 SB Ramps
 Project: Heritage Ridge Residential Project/1-15-4119-1
 File: ICU3

Utilizing trucks with 9-cubic yard capacity

Movement	EXIST. TRAFFIC			EXIST. WITH PROJECT				EXIST. W/PROJECT + MITIGATION				CUMULATIVE W/O PROJECT				CUMULATIVE W/ PROJECT						
	1	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C			
	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio			
Nb Left	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *			
Nb Thru	160	3200	0.050	12	172	3200	0.054	0	172	3200	0.054	53	213	3200	0.067	12	225	3200	0.070			
Nb Right [3]	286	1600	0.179	12	298	1600	0.186	0	298	1600	0.186	94	380	1600	0.238	12	392	1600	0.245			
Sb Left	72	1600	0.045	0	72	1600	0.045	0	72	1600	0.045	11	83	1600	0.052	0	83	1600	0.052			
Sb Thru	1168	3200	0.365 *	14	1182	3200	0.369 *	0	1182	3200	0.369 *	394	1562	3200	0.488 *	14	1576	3200	0.493 *			
Sb Right	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-			
Eb Left	134	0	0.084	0	134	0	0.084	0	134	0	0.084	0	134	0	0.084	0	134	0	0.084			
Eb Thru	2	1600	0.085 *	0	2	1600	0.085 *	0	2	1600	0.085 *	0	2	1600	0.085 *	0	2	1600	0.085 *			
Eb Right [3]	50	1600	0.031	14	64	1600	0.040	0	64	1600	0.040	6	56	1600	0.035	14	70	1600	0.044			
Wb Left	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *			
Wb Thru	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000			
Wb Right	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-			
Yellow Allowance:			0.100 *					0.100 *					0.100 *					0.100 *				
ICU	0.550			0.554				0.554				0.673				0.678						
LOS	A			A				A				B				B						

02:18 PM

* Key conflicting movement as a part of ICU

- Counts conducted by Quality Counts. Existing and cumulative without project traffic volumes obtained from the Heritage Ridge Project Updated Traffic Circulation and Parking Study (prepared by ATE, 10-30-15).
- Capacity expressed in veh/hour of green
- 20% of northbound right-turns were made as Right-Turn-On-Red and have therefore been excluded from the movement volume and corresponding capacity calculations.
- 77% of eastbound right-turns were made as Right-Turn-On-Red and have therefore been excluded from the movement volume and corresponding capacity calculations.

LINSCOTT, LAW & GREENSPAN, ENGINEERS
 600 S. Lake Avenue, Ste 500, Pasadena 91106
 (626) 796.2322 Fax (626) 792.0941

INTERSECTION CAPACITY UTILIZATION

Los Carneros Road @ US-101 SB Ramps
 Peak hr: PM

Date: 11/25/2015
 Date of Count: 2014

Utilizing trucks with 9-cubic yard capacity

N-S St: Los Carneros Road
 E-W St: US-101 SB Ramps
 Project: Heritage Ridge Residential Project/1-15-4119-1
 File: ICU3

Movement	EXIST. TRAFFIC			EXIST. WITH PROJECT				EXIST. W/PROJECT + MITIGATION				CUMULATIVE W/O PROJECT				CUMULATIVE W/ PROJECT						
	Volume	Capacity	V/C	Added Volume	Total Volume	2 Capacity	V/C	Added Volume	Total Volume	2 Capacity	V/C	Added Volume	Total Volume	2 Capacity	V/C	Added Volume	Total Volume	2 Capacity	V/C			
Nb Left	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000			
Nb Thru	773	3200	0.242	14	787	3200	0.246	0	787	3200	0.246	17	790	3200	0.247	14	804	3200	0.251			
Nb Right [3]	845	1600	0.528 *	14	859	1600	0.537 *	0	859	1600	0.537 *	173	1018	1600	0.636 *	14	1032	1600	0.645 *			
Sb Left	43	1600	0.027 *	0	43	1600	0.027 *	0	43	1600	0.027 *	5	48	1600	0.030 *	0	48	1600	0.030 *			
Sb Thru	601	3200	0.188	12	613	3200	0.192	0	613	3200	0.192	280	881	3200	0.275	12	893	3200	0.279			
Sb Right	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-			
Eb Left	97	0	0.061	0	97	0	0.061	0	97	0	0.061	16	113	0	0.071	0	113	0	0.071			
Eb Thru	1	1600	0.061 *	0	1	1600	0.061 *	0	1	1600	0.061 *	0	1	1600	0.071 *	0	1	1600	0.071 *			
Eb Right [3]	0	1600	0.000	12	12	1600	0.008	0	12	1600	0.008	15	15	1600	0.009	12	27	1600	0.017			
Wb Left	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *			
Wb Thru	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000			
Wb Right	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-			
Yellow Allowance:			0.100 *					0.100 *					0.100 *					0.100 *				
ICU	0.716			0.725				0.725				0.838				0.846						
LOS	C			C				C				D				D						

02:18 PM

* Key conflicting movement as a part of ICU

- Counts conducted by National Data & Surveying Services. Existing and cumulative w/o project volumes obtained from Heritage Ridge Project Updated Traffic Circulation and Parking Study (by ATE, 10-30-15).
- Capacity expressed in veh/hour of green
- 12% of northbound right-turns were made as Right-Turn-On-Red and have therefore been excluded from the movement volume and corresponding capacity calculations.
- 80% of eastbound right-turns were made as Right-Turn-On-Red and have therefore been excluded from the movement volume and corresponding capacity calculations.

LINSCOTT, LAW & GREENSPAN, ENGINEERS
 600 S. Lake Avenue, Ste 500, Pasadena 91106
 (626) 796.2322 Fax (626) 792.0941

INTERSECTION CAPACITY UTILIZATION

N-S St: Los Carneros Road
 E-W St: Calle Koral
 Project: Heritage Ridge Residential Project/1-15-4119-1
 File: ICU4

Los Carneros Road @ Calle Koral
 Peak hr: AM

Date: 11/25/2015
 Date of Count: 2014

Utilizing trucks with 9-cubic yard capacity

Movement	EXIST. TRAFFIC			EXIST. WITH PROJECT				EXIST. W/PROJECT + MITIGATION				CUMULATIVE W/O PROJECT				CUMULATIVE W/ PROJECT						
	1 Volume	2 Capacity	V/C Ratio	Added Volume	Total Volume	2 Capacity	V/C Ratio	Added Volume	Total Volume	2 Capacity	V/C Ratio	Added Volume	Total Volume	2 Capacity	V/C Ratio	Added Volume	Total Volume	2 Capacity	V/C Ratio			
Nb Left	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	57	57	1600	0.036 *	0	57	1600	0.036 *			
Nb Thru	399	4800	0.084	0	399	4800	0.084	0	399	4800	0.084	8	407	4800	0.086	0	407	4800	0.086			
Nb Right	4	0	-	0	4	0	-	0	4	0	-	3	7	0	-	0	7	0	-			
Sb Left	247	1600	0.154	27	274	1600	0.171	0	274	1600	0.171	32	279	1600	0.174	27	306	1600	0.191			
Sb Thru	1137	3200	0.355 *	0	1137	3200	0.355 *	0	1137	3200	0.355 *	297	1434	3200	0.478 *	0	1434	3200	0.478 *			
Sb Right	0	0	-	0	0	0	-	0	0	0	-	94	94	0	-	0	94	0	-			
Eb Left	0	0	0.000	0	0	0	0.000 *	0	0	0	0.000 *	69	69	1600	0.043 *	0	69	1600	0.043 *			
Eb Thru	0	0	0.000	0	0	0	0.000	0	0	0	0.000	26	26	1600	0.059	0	26	1600	0.059			
Eb Right	0	0	-	0	0	0	-	0	0	0	-	69	69	0	-	0	69	0	-			
Wb Left	13	1600	0.008	0	13	1600	0.008	0	13	1600	0.008	26	39	0	0.024	0	39	0	0.024			
Wb Thru	0	0	0.000	0	0	0	0.000	0	0	0	0.000	29	29	1600	0.043 *	0	29	1600	0.043 *			
Wb Right [3	4	1600	0.003	24	28	1600	0.018 *	0	28	1600	0.018 *	2	6	1600	0.004	24	30	1600	0.019			
Yellow Allowance:			0.100 *					0.100 *					0.100 *					0.100 *				
ICU			0.463					0.473					0.699					0.699				
LOS			A					A					B					B				

02:18 PM

* Key conflicting movement as a part of ICU

- Counts conducted by Quality Counts. Existing and cumulative without project traffic volumes obtained from the Heritage Ridge Project Updated Traffic Circulation and Parking Study (prepared by ATE, 10-30-15).
- Capacity expressed in veh/hour of green
- 97% of westbound right-turns were made as Right-Turn-On-Red and have therefore been excluded from the movement volume and corresponding capacity calculations.

LINSCOTT, LAW & GREENSPAN, ENGINEERS
 600 S. Lake Avenue, Ste 500, Pasadena 91106
 (626) 796.2322 Fax (626) 792.0941

INTERSECTION CAPACITY UTILIZATION

Los Carneros Road @ Calle Koral
 Peak hr: PM

Date: 11/25/2015
 Date of Count: 2014

Utilizing trucks with 9-cubic yard capacity

N-S St: Los Carneros Road
 E-W St: Calle Koral
 Project: Heritage Ridge Residential Project/1-15-4119-1
 File: ICU4

Movement	EXIST. TRAFFIC			EXIST. WITH PROJECT				EXIST. W/PROJECT + MITIGATION				CUMULATIVE W/O PROJECT				CUMULATIVE W/ PROJECT						
	Volume	Capacity	V/C	Added Volume	Total Volume	2 Capacity	V/C	Added Volume	Total Volume	2 Capacity	V/C	Added Volume	Total Volume	2 Capacity	V/C	Added Volume	Total Volume	2 Capacity	V/C			
Nb Left	0	0	0.000	0	0	0	0.000	0	0	0	0.000	63	63	1600	0.039	0	63	1600	0.039			
Nb Thru	1394	4800	0.296 *	0	1394	4800	0.296 *	0	1394	4800	0.296 *	95	1489	4800	0.317 *	0	1489	4800	0.317 *			
NB Right [3]	25	0	-	0	25	0	-	0	25	0	-	8	33	0	-	0	33	0	-			
Sb Left	101	1600	0.063 *	24	125	1600	0.078 *	0	125	1600	0.078 *	89	190	1600	0.119 *	24	214	1600	0.134 *			
Sb Thru	577	3200	0.180	0	577	3200	0.180	0	577	3200	0.180	129	706	3200	0.239	0	706	3200	0.239			
Sb Right	0	0	-	0	0	0	-	0	0	0	-	60	60	0	-	0	60	0	-			
Eb Left	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	95	95	1600	0.059 *	0	95	1600	0.059 *			
Eb Thru	0	0	0.000	0	0	0	0.000	0	0	0	0.000	11	11	1600	0.048	0	11	1600	0.048			
Eb Right	0	0	-	0	0	0	-	0	0	0	-	65	65	0	-	0	65	0	-			
Wb Left	5	1600	0.003	0	5	1600	0.003	0	5	1600	0.003	2	7	0	0.004	0	7	0	0.004			
Wb Thru	0	0	0.000	0	0	0	0.000	0	0	0	0.000	32	32	1600	0.024	0	32	1600	0.024			
Wb Right [4]	83	1600	0.052 *	27	110	1600	0.069 *	0	110	1600	0.069 *	19	102	1600	0.064 *	27	129	1600	0.081 *			
Yellow Allowance:			0.100 *					0.100 *					0.100 *					0.100 *				
ICU	0.511			0.543				0.543				0.659				0.691						
LOS	A			A				A				B				B						

02:18 PM

* Key conflicting movement as a part of ICU

- Counts conducted by National Data & Surveying Services. Existing and cumulative w/o project volumes obtained from Heritage Ridge Project Updated Traffic Circulation and Parking Study (by ATE, 10-30-15).
- Capacity expressed in veh/hour of green
- 4% of northbound right-turns were made as Right-Turn-On-Red and have therefore been excluded from the movement volume and corresponding capacity calculations.
- 72% of westbound right-turns were made as Right-Turn-On-Red and have therefore been excluded from the movement volume and corresponding capacity calculations.

LINSCOTT, LAW & GREENSPAN, ENGINEERS
 600 S. Lake Avenue, Ste 500, Pasadena 91106
 (626) 796.2322 Fax (626) 792.0941

INTERSECTION CAPACITY UTILIZATION

N-S St: Los Carneros Road
 E-W St: Calle Koral
 Project: Heritage Ridge Residential Project/1-15-4119-1
 File: ICU4A

Los Carneros Road @ Calle Koral
 Peak hr: AM

Date: 11/25/2015
 Date of Count: 2014

Utilizing trucks with 9-cubic yard capacity

Movement	EXIST. TRAFFIC [4]			EXIST. WITH PROJECT [4]				EXIST. W/PROJECT + MITIGATION						
	1 Volume	2 Capacity	V/C Ratio	Added Volume	Total Volume	2 Capacity	V/C Ratio	Added Volume	Total Volume	2 Capacity	V/C Ratio			
Nb Left	57	1600	0.036 *	0	57	1600	0.036 *	0	57	1600	0.036 *			
Nb Thru	399	4800	0.084	0	399	4800	0.084	0	399	4800	0.084			
Nb Right	4	0	-	0	4	0	-	0	4	0	-			
Sb Left	247	1600	0.154	27	274	1600	0.171	0	274	1600	0.171			
Sb Thru	1137	3200	0.385 *	0	1137	3200	0.385 *	0	1137	3200	0.385 *			
Sb Right	94	0	-	0	94	0	-	0	94	0	-			
Eb Left	69	1600	0.043 *	0	69	1600	0.043 *	0	69	1600	0.043 *			
Eb Thru	26	1600	0.059	0	26	1600	0.059	0	26	1600	0.059			
Eb Right	69	0	-	0	69	0	-	0	69	0	-			
Wb Left	13	0	0.008	0	13	0	0.008	0	13	0	0.008			
Wb Thru	29	1600	0.026 *	0	29	1600	0.026 *	0	29	1600	0.026 *			
Wb Right [3]	4	1600	0.003	24	28	1600	0.018	0	28	1600	0.018			
Yellow Allowance:			0.100 *					0.100 *					0.100 *	
ICU			0.590					0.590					0.590	
LOS			A					A					A	

11:59 AM

* Key conflicting movement as a part of ICU

- Counts conducted by Quality Counts. Existing and cumulative without project traffic volumes obtained from the Heritage Ridge Project Updated Traffic Circulation and Parking Study (prepared by ATE, 10-30-15).
- Capacity expressed in veh/hour of green
- 97% of westbound right-turns were made as Right-Turn-On-Red and have therefore been excluded from the movement volume and corresponding capacity calculations.
- Based on input provided by the City, for informational purposes the Existing and Existing with Project analysis conditions also include traffic associated with the completion/occupancy of the Village at Los Carneros Project and the construction of the west leg of this intersection.

LINSCOTT, LAW & GREENSPAN, ENGINEERS
 600 S. Lake Avenue, Ste 500, Pasadena 91106
 (626) 796.2322 Fax (626) 792.0941

INTERSECTION CAPACITY UTILIZATION

N-S St: Los Carneros Road
 E-W St: Calle Koral
 Project: Heritage Ridge Residential Project/1-15-4119-1
 File: ICU4A

Los Carneros Road @ Calle Koral
 Peak hr: PM

Date: 11/25/2015
 Date of Count: 2014

Utilizing trucks with 9-cubic yard capacity

Movement	EXIST. TRAFFIC [5]			EXIST. WITH PROJECT [5]				EXIST. W/PROJECT + MITIGATION						
	Volume	Capacity	V/C	Added Volume	Total Volume	Capacity	V/C	Added Volume	Total Volume	Capacity	V/C			
Nb Left	63	1600	0.039	0	63	1600	0.039	0	63	1600	0.039			
Nb Thru	1394	4800	0.296 *	0	1394	4800	0.296 *	0	1394	4800	0.296 *			
NB Right [3]	25	0	-	0	25	0	-	0	25	0	-			
Sb Left	101	1600	0.063 *	24	125	1600	0.078 *	0	125	1600	0.078 *			
Sb Thru	577	3200	0.199	0	577	3200	0.199	0	577	3200	0.199			
Sb Right	60	0	-	0	60	0	-	0	60	0	-			
Eb Left	95	1600	0.059 *	0	95	1600	0.059 *	0	95	1600	0.059 *			
Eb Thru	11	1600	0.048	0	11	1600	0.048	0	11	1600	0.048			
Eb Right	65	0	-	0	65	0	-	0	65	0	-			
Wb Left	5	0	0.003	0	5	0	0.003	0	5	0	0.003			
Wb Thru	32	1600	0.023	0	32	1600	0.023	0	32	1600	0.023			
Wb Right [4]	83	1600	0.052 *	27	110	1600	0.069 *	0	110	1600	0.069 *			
Yellow Allowance:			0.100 *					0.100 *					0.100 *	
ICU	0.570							0.602						
LOS	A							B						

11:59 AM

* Key conflicting movement as a part of ICU

1 Counts conducted by National Data & Surveying Services. Existing and cumulative w/o project volumes obtained from Heritage Ridge Project Updated Traffic Circulation and Parking Study (by ATE, 10-30-15).

2 Capacity expressed in veh/hour of green

3 4% of northbound right-turns were made as Right-Turn-On-Red and have therefore been excluded from the movement volume and corresponding capacity calculations.

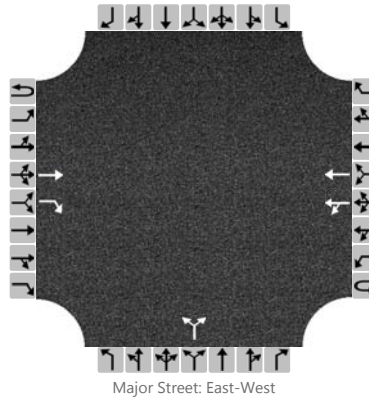
4 72% of westbound right-turns were made as Right-Turn-On-Red and have therefore been excluded from the movement volume and corresponding capacity calculations.

5 Based on input provided by the City, for informational purposes the Existing and Existing with Project analysis conditions also include traffic associated with the completion/occupancy of the Village at Los Carneros Project and the construction of the west leg of this intersection.

HCS 2010 Two-Way Stop Control Summary Report

General Information		Site Information	
Analyst	ACY	Intersection	5 - EXISTING
Agency/Co.	LLG Engineers	Jurisdiction	City of Goleta
Date Performed	11/23/2015	East/West Street	Calle Koral
Analysis Year	2015	North/South Street	Los Carneros Way
Time Analyzed	AM Peak Hour	Peak Hour Factor	1.00
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Heritage Ridge Residential Project/1-15-4119-1		

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Priority																
Number of Lanes	0	0	1	1	0	0	2	0		0	0	0		0	0	0
Configuration			T	R		LT	T				LR					
Volume (veh/h)			36	171		41	107			40		11				
Percent Heavy Vehicles						3				3		3				
Proportion Time Blocked																
Right Turn Channelized	No				No				No				No			
Median Type	Undivided															
Median Storage																

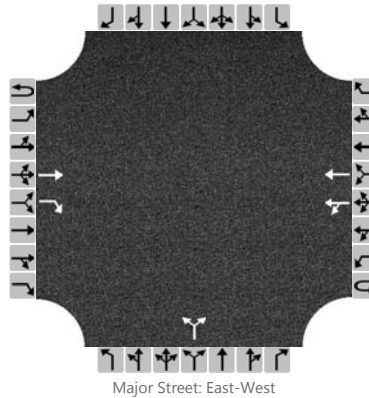
Delay, Queue Length, and Level of Service

Flow Rate (veh/h)					95					51						
Capacity					1354					819						
v/c Ratio					0.07					0.06						
95% Queue Length					0.1					0.2						
Control Delay (s/veh)					7.7					9.7						
Level of Service (LOS)					A					A						
Approach Delay (s/veh)					2.2				9.7							
Approach LOS					A				A							

HCS 2010 Two-Way Stop Control Summary Report

General Information		Site Information	
Analyst	ACY	Intersection	5 - EXISTING
Agency/Co.	LLG Engineers	Jurisdiction	City of Goleta
Date Performed	11/23/2015	East/West Street	Calle Koral
Analysis Year	2015	North/South Street	Los Carneros Way
Time Analyzed	PM Peak Hour	Peak Hour Factor	1.00
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Heritage Ridge Residential Project/1-15-4119-1		

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	1	0	0	2	0		0	0	0		0	0	0
Configuration			T	R		LT	T				LR					
Volume (veh/h)			71	42		7	79			225		43				
Percent Heavy Vehicles						3				3		3				
Proportion Time Blocked																
Right Turn Channelized	No				No				No				No			
Median Type	Undivided															
Median Storage																

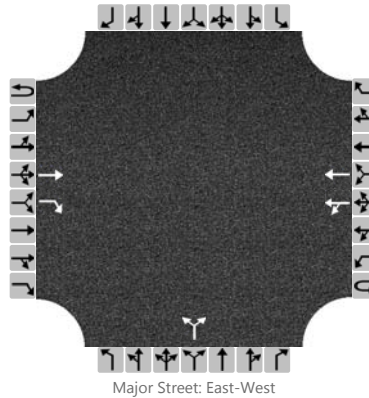
Delay, Queue Length, and Level of Service

Flow Rate (veh/h)						47						268				
Capacity						1467						870				
v/c Ratio						0.03						0.31				
95% Queue Length						0.0						1.3				
Control Delay (s/veh)						7.5						11.0				
Level of Service (LOS)						A						B				
Approach Delay (s/veh)					0.6				11.0							
Approach LOS					A				B							

HCS 2010 Two-Way Stop Control Summary Report

General Information		Site Information	
Analyst	ACY	Intersection	5- EXISTING w/ PROJ 9-CY
Agency/Co.	LLG Engineers	Jurisdiction	City of Goleta
Date Performed	11/23/2015	East/West Street	Calle Koral
Analysis Year	2015	North/South Street	Los Carneros Way
Time Analyzed	AM Peak Hour	Peak Hour Factor	1.00
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Heritage Ridge Residential Project/1-15-4119-1		

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Priority																
Number of Lanes	0	0	1	1	0	0	2	0		0	0	0		0	0	0
Configuration			T	R		LT	T				LR					
Volume (veh/h)			63	171		41	131			40		11				
Percent Heavy Vehicles						3				3		3				
Proportion Time Blocked																
Right Turn Channelized	No				No				No				No			
Median Type	Undivided															
Median Storage																

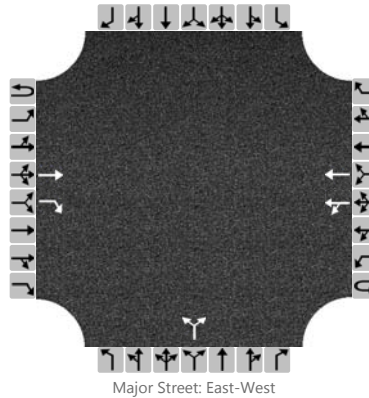
Delay, Queue Length, and Level of Service

Flow Rate (veh/h)						107						51				
Capacity						1323						777				
v/c Ratio						0.08						0.07				
95% Queue Length						0.1						0.2				
Control Delay (s/veh)						7.8						10.0				
Level of Service (LOS)						A						A				
Approach Delay (s/veh)					1.9				10.0							
Approach LOS					A				A							

HCS 2010 Two-Way Stop Control Summary Report

General Information		Site Information	
Analyst	ACY	Intersection	5- EXISTING w/ PROJ 9-CY
Agency/Co.	LLG Engineers	Jurisdiction	City of Goleta
Date Performed	11/23/2015	East/West Street	Calle Koral
Analysis Year	2015	North/South Street	Los Carneros Way
Time Analyzed	PM Peak Hour	Peak Hour Factor	1.00
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Heritage Ridge Residential Project/1-15-4119-1		

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	1	0	0	2	0		0	0	0		0	0	0
Configuration			T	R		LT	T				LR					
Volume (veh/h)			95	42		7	106			225		43				
Percent Heavy Vehicles						3				3		3				
Proportion Time Blocked																
Right Turn Channelized	No				No				No				No			
Median Type	Undivided															
Median Storage																

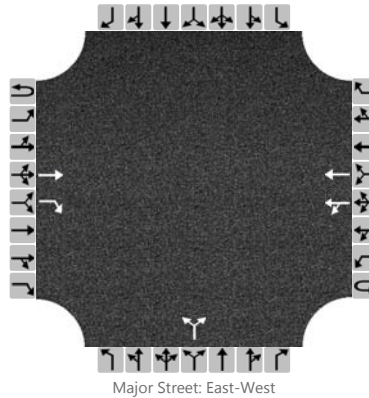
Delay, Queue Length, and Level of Service

Flow Rate (veh/h)					60					268						
Capacity					1437					827						
v/c Ratio					0.04					0.32						
95% Queue Length					0.0					1.4						
Control Delay (s/veh)					7.5					11.4						
Level of Service (LOS)					A					B						
Approach Delay (s/veh)					0.5				11.4							
Approach LOS					A				B							

HCS 2010 Two-Way Stop Control Summary Report

General Information				Site Information			
Analyst	ACY			Intersection	5 - CUM w/o PROJ		
Agency/Co.	LLG Engineers			Jurisdiction	City of Goleta		
Date Performed	11/23/2015			East/West Street	Calle Koral		
Analysis Year	2025			North/South Street	Los Carneros Way		
Time Analyzed	AM Peak Hour			Peak Hour Factor	1.00		
Intersection Orientation	East-West			Analysis Time Period (hrs)	0.25		
Project Description	Heritage Ridge Residential Project/1-15-4119-1						

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Priority																
Number of Lanes	0	0	1	1	0	0	2	0		0	0	0		0	0	0
Configuration			T	R		LT	T				LR					
Volume (veh/h)			103	209		41	95			185		63				
Percent Heavy Vehicles						3				3		3				
Proportion Time Blocked																
Right Turn Channelized	No				No				No				No			
Median Type	Undivided															
Median Storage																

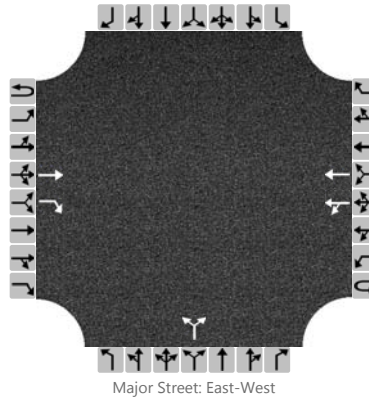
Delay, Queue Length, and Level of Service

Flow Rate (veh/h)						89						248				
Capacity						1238						757				
v/c Ratio						0.07						0.33				
95% Queue Length						0.1						1.4				
Control Delay (s/veh)						8.0						12.1				
Level of Service (LOS)						A						B				
Approach Delay (s/veh)					2.5				12.1							
Approach LOS					A				B							

HCS 2010 Two-Way Stop Control Summary Report

General Information		Site Information	
Analyst	ACY	Intersection	5 - CUM w/o PROJ
Agency/Co.	LLG Engineers	Jurisdiction	City of Goleta
Date Performed	11/23/2015	East/West Street	Calle Koral
Analysis Year	2025	North/South Street	Los Carneros Way
Time Analyzed	PM Peak Hour	Peak Hour Factor	1.00
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Heritage Ridge Residential Project/1-15-4119-1		

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Priority																
Number of Lanes	0	0	1	1	0	0	2	0		0	0	0		0	0	0
Configuration			T	R		LT	T				LR					
Volume (veh/h)			163	72		7	37			365		73				
Percent Heavy Vehicles						3				3		3				
Proportion Time Blocked																
Right Turn Channelized	No				No				No				No			
Median Type	Undivided															
Median Storage																

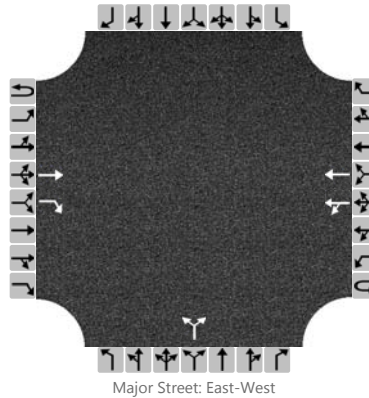
Delay, Queue Length, and Level of Service

Flow Rate (veh/h)						26						438				
Capacity						1322						784				
v/c Ratio						0.02						0.56				
95% Queue Length						0.0						3.5				
Control Delay (s/veh)						7.7						15.2				
Level of Service (LOS)						A						C				
Approach Delay (s/veh)					1.2				15.2							
Approach LOS					A				C							

HCS 2010 Two-Way Stop Control Summary Report

General Information		Site Information	
Analyst	ACY	Intersection	5- CUM with PROJ 9-CY
Agency/Co.	LLG Engineers	Jurisdiction	City of Goleta
Date Performed	11/23/2015	East/West Street	Calle Koral
Analysis Year	2025	North/South Street	Los Carneros Way
Time Analyzed	AM Peak Hour	Peak Hour Factor	1.00
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Heritage Ridge Residential Project/1-15-4119-1		

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Priority																
Number of Lanes	0	0	1	1	0	0	2	0		0	0	0		0	0	0
Configuration			T	R		LT	T				LR					
Volume (veh/h)			130	209		41	119			185		63				
Percent Heavy Vehicles						3				3		3				
Proportion Time Blocked																
Right Turn Channelized	No				No				No				No			
Median Type	Undivided															
Median Storage																

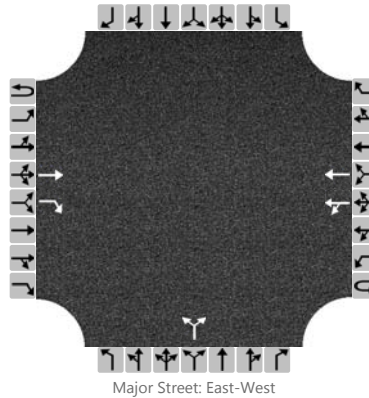
Delay, Queue Length, and Level of Service

Flow Rate (veh/h)						101						248				
Capacity						1210						718				
v/c Ratio						0.08						0.35				
95% Queue Length						0.1						1.5				
Control Delay (s/veh)						8.1						12.6				
Level of Service (LOS)						A						B				
Approach Delay (s/veh)					2.1				12.6							
Approach LOS					A				B							

HCS 2010 Two-Way Stop Control Summary Report

General Information		Site Information	
Analyst	ACY	Intersection	5- CUM with PROJ 9-CY
Agency/Co.	LLG Engineers	Jurisdiction	City of Goleta
Date Performed	11/23/2015	East/West Street	Calle Koral
Analysis Year	2025	North/South Street	Los Carneros Way
Time Analyzed	PM Peak Hour	Peak Hour Factor	1.00
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Heritage Ridge Residential Project/1-15-4119-1		

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Priority																
Number of Lanes	0	0	1	1	0	0	2	0		0	0	0		0	0	0
Configuration			T	R		LT	T				LR					
Volume (veh/h)			187	72		7	64			365		73				
Percent Heavy Vehicles						3				3		3				
Proportion Time Blocked																
Right Turn Channelized	No				No				No				No			
Median Type	Undivided															
Median Storage																

Delay, Queue Length, and Level of Service

Flow Rate (veh/h)						39						438				
Capacity						1296						746				
v/c Ratio						0.03						0.59				
95% Queue Length						0.0						3.9				
Control Delay (s/veh)						7.8						16.5				
Level of Service (LOS)						A						C				
Approach Delay (s/veh)					0.8				16.5							
Approach LOS					A				C							

Intersection No.: 5

N-S Street: Los Carneros Way

E-W Street: Calle Koral

	EXISTING TRAFFIC			EXISTING WITH PROJECT (USING 9-CY TRUCKS)			CUMULATIVE WITHOUT PROJECT			CUMULATIVE WITH PROJECT (USING 9-CY TRUCKS)		
	Volume (veh/hr)	Delay (sec/veh)	LOS	Volume (veh/hr)	Delay (sec/veh)	LOS	Volume (veh/hr)	Delay (sec/veh)	LOS	Volume (veh/hr)	Delay (sec/veh)	LOS
AM Peak Hour												
Westbound Thru/Left:	95	7.7	A	107	7.8	A	89	8.0	A	101	8.1	A
Northbound Left/Right:	51	9.7	A	51	10.0	A	248	12.1	B	248	12.6	B
Average Weighted Delay (sec/veh)		8.4	A		8.5	A		11.0	B		11.3	B
PM Peak Hour												
Westbound Thru/Left:	47	7.5	A	60	7.5	A	26	7.7	A	39	7.8	A
Northbound Left/Right:	268	11.0	B	268	11.4	B	438	15.2	C	438	16.5	C
Average Weighted Delay (sec/veh)		10.5	B		10.7	B		14.8	B		15.8	C

LINSCOTT, LAW & GREENSPAN, ENGINEERS
 600 S. Lake Avenue, Ste 500, Pasadena 91106
 (626) 796.2322 Fax (626) 792.0941

INTERSECTION CAPACITY UTILIZATION

N-S St: Los Carneros Way
 E-W St: Calle Koral
 Project: Heritage Ridge Residential Project/1-15-4119-1
 File: ICU5

Los Carneros Way @ Calle Koral
 Peak hr: AM

Date: 11/25/2015
 Date of Count: 2014

Utilizing trucks with 9-cubic yard capacity

Movement	EXIST. TRAFFIC			EXIST. WITH PROJECT				EXIST. W/PROJECT + MITIGATION				CUMULATIVE W/O PROJECT				CUMULATIVE W/ PROJECT						
	1	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C			
	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio			
Nb Left	40	0	0.025	0	40	0	0.025	0	40	0	0.025	145	185	0	0.116	0	185	0	0.116			
Nb Thru	0	1600	0.032 *	0	0	1600	0.032 *	0	0	1600	0.032 *	0	0	1600	0.155 *	0	0	1600	0.155 *			
Nb Right	11	0	-	0	11	0	-	0	11	0	-	52	63	0	-	0	63	0	-			
Sb Left	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *			
Sb Thru	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000			
Sb Right	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-			
Eb Left	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000			
Eb Thru	36	1600	0.023	27	63	1600	0.039	0	63	1600	0.039	67	103	1600	0.064	27	130	1600	0.081			
Eb Right	171	1600	0.107 *	0	171	1600	0.107 *	0	171	1600	0.107 *	38	209	1600	0.131 *	0	209	1600	0.131 *			
Wb Left	41	0	0.013 *	0	41	0	0.013 *	0	41	0	0.013 *	0	41	0	0.013 *	0	41	0	0.013 *			
Wb Thru	107	3200	0.046	24	131	3200	0.054	0	131	3200	0.054	-12	95	3200	0.043	24	119	3200	0.050			
Wb Right	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-			
Yellow Allowance:			0.100 *					0.100 *					0.100 *					0.100 *				
ICU	0.252			0.252				0.252				0.398				0.398						
LOS	A			A				A				A				A						

02:18 PM

* Key conflicting movement as a part of ICU

1 Counts conducted by Quality Counts. Existing and cumulative without project traffic volumes obtained from the Heritage Ridge Project Updated Traffic Circulation and Parking Study (prepared by ATE, 10-30-15).

2 Capacity expressed in veh/hour of green

LINSCOTT, LAW & GREENSPAN, ENGINEERS
 600 S. Lake Avenue, Ste 500, Pasadena 91106
 (626) 796.2322 Fax (626) 792.0941

INTERSECTION CAPACITY UTILIZATION

N-S St: Los Carneros Way
 E-W St: Calle Koral
 Project: Heritage Ridge Residential Project/1-15-4119-1
 File: ICU5

Los Carneros Way @ Calle Koral
 Peak hr: PM

Date: 11/25/2015
 Date of Count: 2014

Utilizing trucks with 9-cubic yard capacity

Movement	EXIST. TRAFFIC			EXIST. WITH PROJECT				EXIST. W/PROJECT + MITIGATION				CUMULATIVE W/O PROJECT				CUMULATIVE W/ PROJECT						
	Volume	Capacity	V/C	Added Volume	Total Volume	2 Capacity	V/C	Added Volume	Total Volume	2 Capacity	V/C	Added Volume	Total Volume	2 Capacity	V/C	Added Volume	Total Volume	2 Capacity	V/C			
Nb Left	225	0	0.141	0	225	0	0.141	0	225	0	0.141	140	365	0	0.228	0	365	0	0.228			
Nb Thru	0	1600	0.168 *	0	0	1600	0.168 *	0	0	1600	0.168 *	0	0	1600	0.274 *	0	0	1600	0.274 *			
Nb Right	43	0	-	0	43	0	-	0	43	0	-	30	73	0	-	0	73	0	-			
Sb Left	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *			
Sb Thru	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000			
Sb Right	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-			
Eb Left	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000			
Eb Thru	71	1600	0.044 *	24	95	1600	0.059 *	0	95	1600	0.059 *	92	163	1600	0.102 *	24	187	1600	0.117 *			
Eb Right	42	1600	0.026	0	42	1600	0.026	0	42	1600	0.026	30	72	1600	0.045	0	72	1600	0.045			
Wb Left	7	0	0.002 *	0	7	0	0.002 *	0	7	0	0.002 *	0	7	0	0.002 *	0	7	0	0.002 *			
Wb Thru	79	3200	0.027	27	106	3200	0.035	0	106	3200	0.035	-42	37	3200	0.014	27	64	3200	0.022			
Wb Right	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-			
Yellow Allowance:			0.100 *					0.100 *					0.100 *					0.100 *				
ICU	0.314			0.329				0.329				0.478				0.493						
LOS	A			A				A				A				A						

02:18 PM

* Key conflicting movement as a part of ICU

1 Counts conducted by National Data & Surveying Services. Existing and cumulative w/o project volumes obtained from Heritage Ridge Project Updated Traffic Circulation and Parking Study (by ATE, 10-30-15).
 2 Capacity expressed in veh/hour of green

LINSCOTT, LAW & GREENSPAN, ENGINEERS
 600 S. Lake Avenue, Ste 500, Pasadena 91106
 (626) 796.2322 Fax (626) 792.0941

INTERSECTION CAPACITY UTILIZATION

Los Carneros Road @ US-101 NB Ramps
 Peak hr: AM

Date: 11/25/2015
 Date of Count: 2014

Utilizing trucks with 20-cubic yard capacity

N-S St: Los Carneros Road
 E-W St: US-101 NB Ramps
 Project: Heritage Ridge Residential Project/1-15-4119-1
 File: ICU2

Movement	EXIST. TRAFFIC			EXIST. WITH PROJECT				EXIST. W/PROJECT + MITIGATION				CUMULATIVE W/O PROJECT				CUMULATIVE W/ PROJECT						
	1	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C			
	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio			
Nb Left	21	1600	0.013 *	9	30	1600	0.019 *	0	30	1600	0.019 *	13	34	1600	0.021 *	9	43	1600	0.027 *			
Nb Thru	276	3200	0.086	0	276	3200	0.086	0	276	3200	0.086	37	313	3200	0.098	0	313	3200	0.098			
Nb Right	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-			
Sb Left	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000			
Sb Thru	423	3200	0.165 *	0	423	3200	0.165 *	0	423	3200	0.165 *	-11	412	3200	0.161 *	0	412	3200	0.161 *			
Sb Right [3]	106	0	-	0	106	0	-	0	106	0	-	-3	103	0	-	0	103	0	-			
Eb Left	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *			
Eb Thru	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000			
Eb Right	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-			
Wb Left	805	0	0.252	11	816	0	0.255	0	816	0	0.255	428	1233	0	0.385	11	1244	0	0.389			
Wb Thru	2	3200	0.265 *	0	2	3200	0.268 *	0	2	3200	0.268 *	0	2	3200	0.398 *	0	2	3200	0.402 *			
Wb Right [4]	40	0	-	0	40	0	-	0	40	0	-	0	40	0	-	0	40	0	-			
Yellow Allowance:			0.100 *					0.100 *					0.100 *					0.100 *				
ICU	0.543			0.552				0.552				0.681				0.690						
LOS	A			A				A				B				B						

02:24 PM

* Key conflicting movement as a part of ICU

- Counts conducted by Quality Counts. Existing and cumulative without project traffic volumes obtained from the Heritage Ridge Project Updated Traffic Circulation and Parking Study (prepared by ATE, 10-30-15).
- Capacity expressed in veh/hour of green
- 4% of southbound right-turns were made as Right-Turn-On-Red and have therefore been excluded from the movement volume and corresponding capacity calculations.
- 25% of westbound right-turns were made as Right-Turn-On-Red and have therefore been excluded from the movement volume and corresponding capacity calculations.

LINSCOTT, LAW & GREENSPAN, ENGINEERS
 600 S. Lake Avenue, Ste 500, Pasadena 91106
 (626) 796.2322 Fax (626) 792.0941

INTERSECTION CAPACITY UTILIZATION

Los Carneros Road @ US-101 NB Ramps
 Peak hr: PM

Date: 11/25/2015
 Date of Count: 2014

N-S St: Los Carneros Road
 E-W St: US-101 NB Ramps
 Project: Heritage Ridge Residential Project/1-15-4119-1
 File: ICU2

Utilizing trucks with 20-cubic yard capacity

Movement	EXIST. TRAFFIC			EXIST. WITH PROJECT				EXIST. W/PROJECT + MITIGATION				CUMULATIVE W/O PROJECT				CUMULATIVE W/ PROJECT						
	Volume	Capacity	V/C	Added Volume	Total Volume	2 Capacity	V/C	Added Volume	Total Volume	2 Capacity	V/C	Added Volume	Total Volume	2 Capacity	V/C	Added Volume	Total Volume	2 Capacity	V/C			
Nb Left	236	1600	0.148 *	11	247	1600	0.154 *	0	247	1600	0.154 *	0	236	1600	0.148 *	11	247	1600	0.154 *			
Nb Thru	618	3200	0.193	0	618	3200	0.193	0	618	3200	0.193	49	667	3200	0.208	0	667	3200	0.208			
Nb Right	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-			
Sb Left	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000			
Sb Thru	250	3200	0.100 *	0	250	3200	0.100 *	0	250	3200	0.100 *	186	436	3200	0.163 *	0	436	3200	0.163 *			
Sb Right [3]	71	0	-	0	71	0	-	0	71	0	-	13	84	0	-	0	84	0	-			
Eb Left	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *			
Eb Thru	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000			
Eb Right	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-			
Wb Left	436	0	0.136	9	445	0	0.139	0	445	0	0.139	57	493	0	0.154	9	502	0	0.157			
Wb Thru	7	3200	0.141 *	0	7	3200	0.144 *	0	7	3200	0.144 *	4	11	3200	0.161 *	0	11	3200	0.164 *			
Wb Right [4]	9	0	-	0	9	0	-	0	9	0	-	3	12	0	-	0	12	0	-			
Yellow Allowance:			0.100 *					0.100 *					0.100 *					0.100 *				
ICU	0.489			0.499				0.499				0.571				0.581						
LOS	A			A				A				A				A						

02:24 PM

* Key conflicting movement as a part of ICU

- Counts conducted by National Data & Surveying Services. Existing and cumulative w/o project volumes obtained from Heritage Ridge Project Updated Traffic Circulation and Parking Study (by ATE, 10-30-15).
- Capacity expressed in veh/hour of green
- 34% of southbound right-turns were made as Right-Turn-On-Red and have therefore been excluded from the movement volume and corresponding capacity calculations.
- 63% of westbound right-turns were made as Right-Turn-On-Red and have therefore been excluded from the movement volume and corresponding capacity calculations.

LINSCOTT, LAW & GREENSPAN, ENGINEERS
 600 S. Lake Avenue, Ste 500, Pasadena 91106
 (626) 796.2322 Fax (626) 792.0941

INTERSECTION CAPACITY UTILIZATION

Los Carneros Road @ US-101 SB Ramps
 Peak hr: AM

Date: 11/25/2015
 Date of Count: 2014

Utilizing trucks with 20-cubic yard capacity

N-S St: Los Carneros Road
 E-W St: US-101 SB Ramps
 Project: Heritage Ridge Residential Project/1-15-4119-1
 File: ICU3

Movement	EXIST. TRAFFIC			EXIST. WITH PROJECT				EXIST. W/PROJECT + MITIGATION				CUMULATIVE W/O PROJECT				CUMULATIVE W/ PROJECT			
	1	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C
	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio
Nb Left	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *
Nb Thru	160	3200	0.050	9	169	3200	0.053	0	169	3200	0.053	53	213	3200	0.067	9	222	3200	0.069
Nb Right [3]	286	1600	0.179	9	295	1600	0.184	0	295	1600	0.184	94	380	1600	0.238	9	389	1600	0.243
Sb Left	72	1600	0.045	0	72	1600	0.045	0	72	1600	0.045	11	83	1600	0.052	0	83	1600	0.052
Sb Thru	1168	3200	0.365 *	11	1179	3200	0.368 *	0	1179	3200	0.368 *	394	1562	3200	0.488 *	11	1573	3200	0.492 *
Sb Right	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-
Eb Left	134	0	0.084	0	134	0	0.084	0	134	0	0.084	0	134	0	0.084	0	134	0	0.084
Eb Thru	2	1600	0.085 *	0	2	1600	0.085 *	0	2	1600	0.085 *	0	2	1600	0.085 *	0	2	1600	0.085 *
Eb Right [3]	50	1600	0.031	11	61	1600	0.038	0	61	1600	0.038	6	56	1600	0.035	11	67	1600	0.042
Wb Left	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *
Wb Thru	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000
Wb Right	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-
Yellow Allowance:			0.100 *				0.100 *				0.100 *				0.100 *				0.100 *
ICU			0.550				0.553				0.553				0.673				0.677
LOS			A				A				A				B				B

02:24 PM

* Key conflicting movement as a part of ICU

- Counts conducted by Quality Counts. Existing and cumulative without project traffic volumes obtained from the Heritage Ridge Project Updated Traffic Circulation and Parking Study (prepared by ATE, 10-30-15).
- Capacity expressed in veh/hour of green
- 20% of northbound right-turns were made as Right-Turn-On-Red and have therefore been excluded from the movement volume and corresponding capacity calculations.
- 77% of eastbound right-turns were made as Right-Turn-On-Red and have therefore been excluded from the movement volume and corresponding capacity calculations.

LINSCOTT, LAW & GREENSPAN, ENGINEERS
 600 S. Lake Avenue, Ste 500, Pasadena 91106
 (626) 796.2322 Fax (626) 792.0941

INTERSECTION CAPACITY UTILIZATION

Los Carneros Road @ US-101 SB Ramps
 Peak hr: PM

Date: 11/25/2015
 Date of Count: 2014

Utilizing trucks with 20-cubic yard capacity

N-S St: Los Carneros Road
 E-W St: US-101 SB Ramps
 Project: Heritage Ridge Residential Project/1-15-4119-1
 File: ICU3

Movement	EXIST. TRAFFIC			EXIST. WITH PROJECT				EXIST. W/PROJECT + MITIGATION				CUMULATIVE W/O PROJECT				CUMULATIVE W/ PROJECT							
	Volume	Capacity	V/C	Added Volume	Total Volume	2 Capacity	V/C	Added Volume	Total Volume	2 Capacity	V/C	Added Volume	Total Volume	2 Capacity	V/C	Added Volume	Total Volume	2 Capacity	V/C				
Nb Left	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000				
Nb Thru	773	3200	0.242	11	784	3200	0.245	0	784	3200	0.245	17	790	3200	0.247	11	801	3200	0.250				
Nb Right [3]	845	1600	0.528 *	11	856	1600	0.535 *	0	856	1600	0.535 *	173	1018	1600	0.636 *	11	1029	1600	0.643 *				
Sb Left	43	1600	0.027 *	0	43	1600	0.027 *	0	43	1600	0.027 *	5	48	1600	0.030 *	0	48	1600	0.030 *				
Sb Thru	601	3200	0.188	9	610	3200	0.191	0	610	3200	0.191	280	881	3200	0.275	9	890	3200	0.278				
Sb Right	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-				
Eb Left	97	0	0.061	0	97	0	0.061	0	97	0	0.061	16	113	0	0.071	0	113	0	0.071				
Eb Thru	1	1600	0.061 *	0	1	1600	0.061 *	0	1	1600	0.061 *	0	1	1600	0.071 *	0	1	1600	0.071 *				
Eb Right [3]	0	1600	0.000	9	9	1600	0.006	0	9	1600	0.006	15	15	1600	0.009	9	24	1600	0.015				
Wb Left	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *				
Wb Thru	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000				
Wb Right	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-				
Yellow Allowance:			0.100 *					0.100 *					0.100 *					0.100 *					
ICU	0.716							0.723								0.838							
LOS	C							C								D							

02:24 PM

* Key conflicting movement as a part of ICU

- Counts conducted by National Data & Surveying Services. Existing and cumulative w/o project volumes obtained from Heritage Ridge Project Updated Traffic Circulation and Parking Study (by ATE, 10-30-15).
- Capacity expressed in veh/hour of green
- 12% of northbound right-turns were made as Right-Turn-On-Red and have therefore been excluded from the movement volume and corresponding capacity calculations.
- 80% of eastbound right-turns were made as Right-Turn-On-Red and have therefore been excluded from the movement volume and corresponding capacity calculations.

LINSCOTT, LAW & GREENSPAN, ENGINEERS
 600 S. Lake Avenue, Ste 500, Pasadena 91106
 (626) 796.2322 Fax (626) 792.0941

INTERSECTION CAPACITY UTILIZATION

N-S St: Los Carneros Road
 E-W St: Calle Koral
 Project: Heritage Ridge Residential Project/1-15-4119-1
 File: ICU4

Los Carneros Road @ Calle Koral
 Peak hr: AM

Date: 11/25/2015
 Date of Count: 2014

Utilizing trucks with 20-cubic yard capacity

Movement	EXIST. TRAFFIC			EXIST. WITH PROJECT				EXIST. W/PROJECT + MITIGATION				CUMULATIVE W/O PROJECT				CUMULATIVE W/ PROJECT						
	1 Volume	2 Capacity	V/C Ratio	Added Volume	Total Volume	2 Capacity	V/C Ratio	Added Volume	Total Volume	2 Capacity	V/C Ratio	Added Volume	Total Volume	2 Capacity	V/C Ratio	Added Volume	Total Volume	2 Capacity	V/C Ratio			
Nb Left	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	57	57	1600	0.036 *	0	57	1600	0.036 *			
Nb Thru	399	4800	0.084	0	399	4800	0.084	0	399	4800	0.084	8	407	4800	0.086	0	407	4800	0.086			
Nb Right	4	0	-	0	4	0	-	0	4	0	-	3	7	0	-	0	7	0	-			
Sb Left	247	1600	0.154	21	268	1600	0.168	0	268	1600	0.168	32	279	1600	0.174	21	300	1600	0.188			
Sb Thru	1137	3200	0.355 *	0	1137	3200	0.355 *	0	1137	3200	0.355 *	297	1434	3200	0.478 *	0	1434	3200	0.478 *			
Sb Right	0	0	-	0	0	0	-	0	0	0	-	94	94	0	-	0	94	0	-			
Eb Left	0	0	0.000	0	0	0	0.000 *	0	0	0	0.000 *	69	69	1600	0.043 *	0	69	1600	0.043 *			
Eb Thru	0	0	0.000	0	0	0	0.000	0	0	0	0.000	26	26	1600	0.059	0	26	1600	0.059			
Eb Right	0	0	-	0	0	0	-	0	0	0	-	69	69	0	-	0	69	0	-			
Wb Left	13	1600	0.008	0	13	1600	0.008	0	13	1600	0.008	26	39	0	0.024	0	39	0	0.024			
Wb Thru	0	0	0.000	0	0	0	0.000	0	0	0	0.000	29	29	1600	0.043 *	0	29	1600	0.043 *			
Wb Right [3	4	1600	0.003	18	22	1600	0.014 *	0	22	1600	0.014 *	2	6	1600	0.004	18	24	1600	0.015			
Yellow Allowance:			0.100 *					0.100 *					0.100 *					0.100 *				
ICU	0.463			0.469				0.469				0.699				0.699						
LOS	A			A				A				B				B						

02:24 PM

* Key conflicting movement as a part of ICU

1 Counts conducted by Quality Counts. Existing and cumulative without project traffic volumes obtained from the Heritage Ridge Project Updated Traffic Circulation and Parking Study (prepared by ATE, 10-30-15).

2 Capacity expressed in veh/hour of green

3 97% of westbound right-turns were made as Right-Turn-On-Red and have therefore been excluded from the movement volume and corresponding capacity calculations.

LINSCOTT, LAW & GREENSPAN, ENGINEERS
 600 S. Lake Avenue, Ste 500, Pasadena 91106
 (626) 796.2322 Fax (626) 792.0941

INTERSECTION CAPACITY UTILIZATION

Los Carneros Road @ Calle Koral
 Peak hr: PM

Date: 11/25/2015
 Date of Count: 2014

Utilizing trucks with 20-cubic yard capacity

N-S St: Los Carneros Road
 E-W St: Calle Koral
 Project: Heritage Ridge Residential Project/1-15-4119-1
 File: ICU4

Movement	EXIST. TRAFFIC			EXIST. WITH PROJECT				EXIST. W/PROJECT + MITIGATION				CUMULATIVE W/O PROJECT				CUMULATIVE W/ PROJECT						
	Volume	Capacity	V/C	Added Volume	Total Volume	2 Capacity	V/C	Added Volume	Total Volume	2 Capacity	V/C	Added Volume	Total Volume	2 Capacity	V/C	Added Volume	Total Volume	2 Capacity	V/C			
Nb Left	0	0	0.000	0	0	0	0.000	0	0	0	0.000	63	63	1600	0.039	0	63	1600	0.039			
Nb Thru	1394	4800	0.296 *	0	1394	4800	0.296 *	0	1394	4800	0.296 *	95	1489	4800	0.317 *	0	1489	4800	0.317 *			
NB Right [3]	25	0	-	0	25	0	-	0	25	0	-	8	33	0	-	0	33	0	-			
Sb Left	101	1600	0.063 *	18	119	1600	0.074 *	0	119	1600	0.074 *	89	190	1600	0.119 *	18	208	1600	0.130 *			
Sb Thru	577	3200	0.180	0	577	3200	0.180	0	577	3200	0.180	129	706	3200	0.239	0	706	3200	0.239			
Sb Right	0	0	-	0	0	0	-	0	0	0	-	60	60	0	-	0	60	0	-			
Eb Left	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	95	95	1600	0.059 *	0	95	1600	0.059 *			
Eb Thru	0	0	0.000	0	0	0	0.000	0	0	0	0.000	11	11	1600	0.048	0	11	1600	0.048			
Eb Right	0	0	-	0	0	0	-	0	0	0	-	65	65	0	-	0	65	0	-			
Wb Left	5	1600	0.003	0	5	1600	0.003	0	5	1600	0.003	2	7	0	0.004	0	7	0	0.004			
Wb Thru	0	0	0.000	0	0	0	0.000	0	0	0	0.000	32	32	1600	0.024	0	32	1600	0.024			
Wb Right [4]	83	1600	0.052 *	21	104	1600	0.065 *	0	104	1600	0.065 *	19	102	1600	0.064 *	21	123	1600	0.077 *			
Yellow Allowance:			0.100 *					0.100 *					0.100 *					0.100 *				
ICU	0.511			0.535				0.535				0.659				0.683						
LOS	A			A				A				B				B						

02:24 PM

* Key conflicting movement as a part of ICU

- Counts conducted by National Data & Surveying Services. Existing and cumulative w/o project volumes obtained from Heritage Ridge Project Updated Traffic Circulation and Parking Study (by ATE, 10-30-15).
- Capacity expressed in veh/hour of green
- 4% of northbound right-turns were made as Right-Turn-On-Red and have therefore been excluded from the movement volume and corresponding capacity calculations.
- 72% of westbound right-turns were made as Right-Turn-On-Red and have therefore been excluded from the movement volume and corresponding capacity calculations.

LINSCOTT, LAW & GREENSPAN, ENGINEERS
 600 S. Lake Avenue, Ste 500, Pasadena 91106
 (626) 796.2322 Fax (626) 792.0941

INTERSECTION CAPACITY UTILIZATION

N-S St: Los Carneros Road
 E-W St: Calle Koral
 Project: Heritage Ridge Residential Project/1-15-4119-1
 File: ICU4A

Los Carneros Road @ Calle Koral
 Peak hr: AM

Date: 11/25/2015
 Date of Count: 2014

Utilizing trucks with 20-cubic yard capacity

Movement	EXIST. TRAFFIC [4]			EXIST. WITH PROJECT [4]				EXIST. W/PROJECT + MITIGATION						
	1 Volume	2 Capacity	V/C Ratio	Added Volume	Total Volume	2 Capacity	V/C Ratio	Added Volume	Total Volume	2 Capacity	V/C Ratio			
Nb Left	57	1600	0.036 *	0	57	1600	0.036 *	0	57	1600	0.036 *			
Nb Thru	399	4800	0.084	0	399	4800	0.084	0	399	4800	0.084			
Nb Right	4	0	-	0	4	0	-	0	4	0	-			
Sb Left	247	1600	0.154	21	268	1600	0.168	0	268	1600	0.168			
Sb Thru	1137	3200	0.385 *	0	1137	3200	0.385 *	0	1137	3200	0.385 *			
Sb Right	94	0	-	0	94	0	-	0	94	0	-			
Eb Left	69	1600	0.043 *	0	69	1600	0.043 *	0	69	1600	0.043 *			
Eb Thru	26	1600	0.059	0	26	1600	0.059	0	26	1600	0.059			
Eb Right	69	0	-	0	69	0	-	0	69	0	-			
Wb Left	13	0	0.008	0	13	0	0.008	0	13	0	0.008			
Wb Thru	29	1600	0.026 *	0	29	1600	0.026 *	0	29	1600	0.026 *			
Wb Right [3]	4	1600	0.003	18	22	1600	0.014	0	22	1600	0.014			
Yellow Allowance:			0.100 *					0.100 *					0.100 *	
ICU LOS			0.590 A					0.590 A					0.590 A	

12:04 PM

* Key conflicting movement as a part of ICU

- Counts conducted by Quality Counts. Existing and cumulative without project traffic volumes obtained from the Heritage Ridge Project Updated Traffic Circulation and Parking Study (prepared by ATE, 10-30-15).
- Capacity expressed in veh/hour of green
- 97% of westbound right-turns were made as Right-Turn-On-Red and have therefore been excluded from the movement volume and corresponding capacity calculations.
- Based on input provided by the City, for informational purposes the Existing and Existing with Project analysis conditions also include traffic associated with the completion/occupancy of the Village at Los Carneros Project and the construction of the west leg of this intersection.

LINSCOTT, LAW & GREENSPAN, ENGINEERS
 600 S. Lake Avenue, Ste 500, Pasadena 91106
 (626) 796.2322 Fax (626) 792.0941

INTERSECTION CAPACITY UTILIZATION

N-S St: Los Carneros Road
 E-W St: Calle Koral
 Project: Heritage Ridge Residential Project/1-15-4119-1
 File: ICU4A

Los Carneros Road @ Calle Koral
 Peak hr: PM

Date: 11/25/2015
 Date of Count: 2014

Utilizing trucks with 20-cubic yard capacity

Movement	EXIST. TRAFFIC [5]			EXIST. WITH PROJECT [5]				EXIST. W/PROJECT + MITIGATION						
	Volume	Capacity	V/C	Added Volume	Total Volume	Capacity	V/C	Added Volume	Total Volume	Capacity	V/C			
Nb Left	63	1600	0.039	0	63	1600	0.039	0	63	1600	0.039			
Nb Thru	1394	4800	0.296 *	0	1394	4800	0.296 *	0	1394	4800	0.296 *			
NB Right [3]	25	0	-	0	25	0	-	0	25	0	-			
Sb Left	101	1600	0.063 *	18	119	1600	0.074 *	0	119	1600	0.074 *			
Sb Thru	577	3200	0.199	0	577	3200	0.199	0	577	3200	0.199			
Sb Right	60	0	-	0	60	0	-	0	60	0	-			
Eb Left	95	1600	0.059 *	0	95	1600	0.059 *	0	95	1600	0.059 *			
Eb Thru	11	1600	0.048	0	11	1600	0.048	0	11	1600	0.048			
Eb Right	65	0	-	0	65	0	-	0	65	0	-			
Wb Left	5	0	0.003	0	5	0	0.003	0	5	0	0.003			
Wb Thru	32	1600	0.023	0	32	1600	0.023	0	32	1600	0.023			
Wb Right [4]	83	1600	0.052 *	21	104	1600	0.065 *	0	104	1600	0.065 *			
Yellow Allowance:			0.100 *					0.100 *					0.100 *	
ICU	0.570							0.594						
LOS	A							A						

12:04 PM

* Key conflicting movement as a part of ICU

1 Counts conducted by National Data & Surveying Services. Existing and cumulative w/o project volumes obtained from Heritage Ridge Project Updated Traffic Circulation and Parking Study (by ATE, 10-30-15).

2 Capacity expressed in veh/hour of green

3 4% of northbound right-turns were made as Right-Turn-On-Red and have therefore been excluded from the movement volume and corresponding capacity calculations.

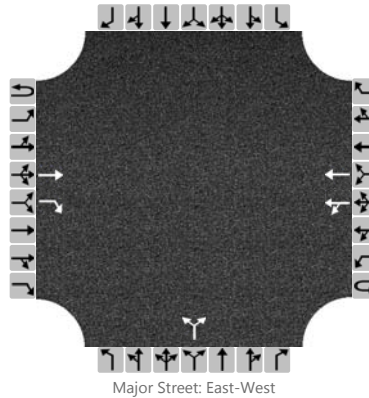
4 72% of westbound right-turns were made as Right-Turn-On-Red and have therefore been excluded from the movement volume and corresponding capacity calculations.

5 Based on input provided by the City, for informational purposes the Existing and Existing with Project analysis conditions also include traffic associated with the completion/occupancy of the Village at Los Carneros Project and the construction of the west leg of this intersection.

HCS 2010 Two-Way Stop Control Summary Report

General Information		Site Information	
Analyst	ACY	Intersection	5 - EXISTING
Agency/Co.	LLG Engineers	Jurisdiction	City of Goleta
Date Performed	11/23/2015	East/West Street	Calle Koral
Analysis Year	2015	North/South Street	Los Carneros Way
Time Analyzed	AM Peak Hour	Peak Hour Factor	1.00
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Heritage Ridge Residential Project/1-15-4119-1		

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	1	0	0	2	0		0	0	0		0	0	0
Configuration			T	R		LT	T				LR					
Volume (veh/h)			36	171		41	107			40		11				
Percent Heavy Vehicles						3				3		3				
Proportion Time Blocked																
Right Turn Channelized	No				No				No				No			
Median Type	Undivided															
Median Storage																

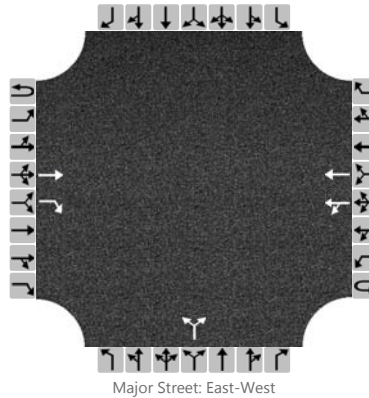
Delay, Queue Length, and Level of Service

Flow Rate (veh/h)						95						51				
Capacity						1354						819				
v/c Ratio						0.07						0.06				
95% Queue Length						0.1						0.2				
Control Delay (s/veh)						7.7						9.7				
Level of Service (LOS)						A						A				
Approach Delay (s/veh)					2.2				9.7							
Approach LOS					A				A							

HCS 2010 Two-Way Stop Control Summary Report

General Information		Site Information	
Analyst	ACY	Intersection	5 - EXISTING
Agency/Co.	LLG Engineers	Jurisdiction	City of Goleta
Date Performed	11/23/2015	East/West Street	Calle Koral
Analysis Year	2015	North/South Street	Los Carneros Way
Time Analyzed	PM Peak Hour	Peak Hour Factor	1.00
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Heritage Ridge Residential Project/1-15-4119-1		

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Priority																
Number of Lanes	0	0	1	1	0	0	2	0		0	0	0		0	0	0
Configuration			T	R		LT	T				LR					
Volume (veh/h)			71	42		7	79			225		43				
Percent Heavy Vehicles						3				3		3				
Proportion Time Blocked																
Right Turn Channelized	No				No				No				No			
Median Type	Undivided															
Median Storage																

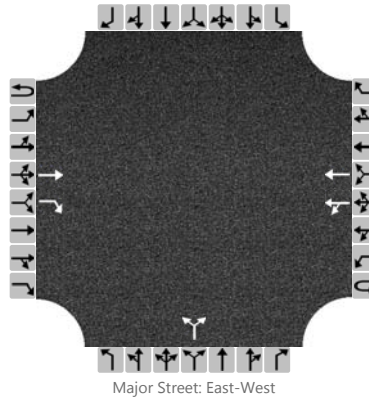
Delay, Queue Length, and Level of Service

Flow Rate (veh/h)						47						268				
Capacity						1467						870				
v/c Ratio						0.03						0.31				
95% Queue Length						0.0						1.3				
Control Delay (s/veh)						7.5						11.0				
Level of Service (LOS)						A						B				
Approach Delay (s/veh)					0.6				11.0							
Approach LOS					A				B							

HCS 2010 Two-Way Stop Control Summary Report

General Information		Site Information	
Analyst	ACY	Intersection	5- EXISTING w/ PROJ 20-CY
Agency/Co.	LLG Engineers	Jurisdiction	City of Goleta
Date Performed	11/23/2015	East/West Street	Calle Koral
Analysis Year	2015	North/South Street	Los Carneros Way
Time Analyzed	AM Peak Hour	Peak Hour Factor	1.00
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Heritage Ridge Residential Project/1-15-4119-1		

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Priority																
Number of Lanes	0	0	1	1	0	0	2	0		0	0	0		0	0	0
Configuration			T	R		LT	T				LR					
Volume (veh/h)			57	171		41	125			40		11				
Percent Heavy Vehicles						3				3		3				
Proportion Time Blocked																
Right Turn Channelized	No				No				No				No			
Median Type	Undivided															
Median Storage																

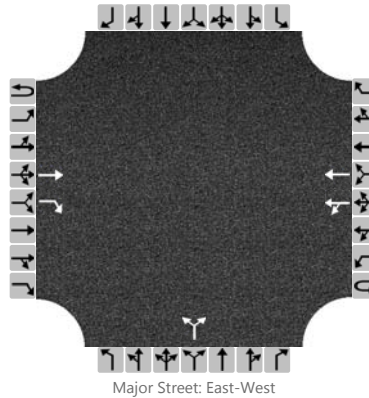
Delay, Queue Length, and Level of Service

Flow Rate (veh/h)						104						51				
Capacity						1330						786				
v/c Ratio						0.08						0.06				
95% Queue Length						0.1						0.2				
Control Delay (s/veh)						7.8						9.9				
Level of Service (LOS)						A						A				
Approach Delay (s/veh)					2.0				9.9							
Approach LOS					A				A							

HCS 2010 Two-Way Stop Control Summary Report

General Information		Site Information	
Analyst	ACY	Intersection	5- EXISTING w/ PROJ 20-CY
Agency/Co.	LLG Engineers	Jurisdiction	City of Goleta
Date Performed	11/23/2015	East/West Street	Calle Koral
Analysis Year	2015	North/South Street	Los Carneros Way
Time Analyzed	PM Peak Hour	Peak Hour Factor	1.00
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Heritage Ridge Residential Project/1-15-4119-1		

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Priority																
Number of Lanes	0	0	1	1	0	0	2	0		0	0	0		0	0	0
Configuration			T	R		LT	T				LR					
Volume (veh/h)			89	42		7	100			225		43				
Percent Heavy Vehicles						3				3		3				
Proportion Time Blocked																
Right Turn Channelized	No				No				No				No			
Median Type	Undivided															
Median Storage																

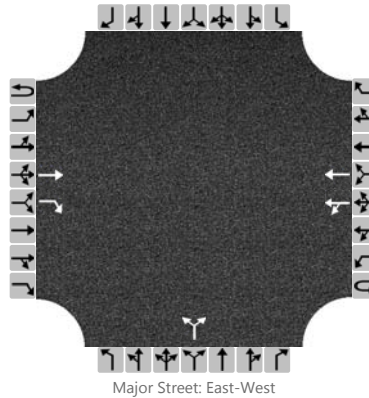
Delay, Queue Length, and Level of Service

Flow Rate (veh/h)						57						268				
Capacity						1445						837				
v/c Ratio						0.04						0.32				
95% Queue Length						0.0						1.4				
Control Delay (s/veh)						7.5						11.3				
Level of Service (LOS)						A						B				
Approach Delay (s/veh)					0.5				11.3							
Approach LOS					A				B							

HCS 2010 Two-Way Stop Control Summary Report

General Information		Site Information	
Analyst	ACY	Intersection	5 - CUM w/o PROJ
Agency/Co.	LLG Engineers	Jurisdiction	City of Goleta
Date Performed	11/23/2015	East/West Street	Calle Koral
Analysis Year	2025	North/South Street	Los Carneros Way
Time Analyzed	AM Peak Hour	Peak Hour Factor	1.00
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Heritage Ridge Residential Project/1-15-4119-1		

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Priority																
Number of Lanes	0	0	1	1	0	0	2	0		0	0	0		0	0	0
Configuration			T	R		LT	T				LR					
Volume (veh/h)			103	209		41	95			185		63				
Percent Heavy Vehicles						3				3		3				
Proportion Time Blocked																
Right Turn Channelized	No				No				No				No			
Median Type	Undivided															
Median Storage																

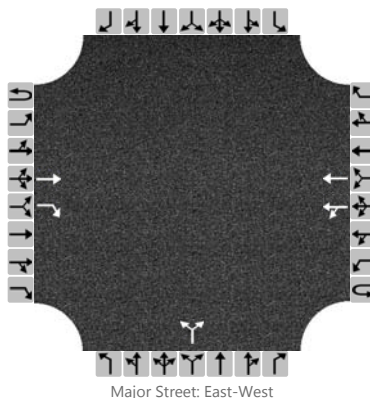
Delay, Queue Length, and Level of Service

Flow Rate (veh/h)						89						248				
Capacity						1238						757				
v/c Ratio						0.07						0.33				
95% Queue Length						0.1						1.4				
Control Delay (s/veh)						8.0						12.1				
Level of Service (LOS)						A						B				
Approach Delay (s/veh)					2.5				12.1							
Approach LOS					A				B							

HCS 2010 Two-Way Stop Control Summary Report

General Information		Site Information	
Analyst	ACY	Intersection	5 - CUM w/o PROJ
Agency/Co.	LLG Engineers	Jurisdiction	City of Goleta
Date Performed	11/23/2015	East/West Street	Calle Koral
Analysis Year	2025	North/South Street	Los Carneros Way
Time Analyzed	PM Peak Hour	Peak Hour Factor	1.00
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Heritage Ridge Residential Project/1-15-4119-1		

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Priority																
Number of Lanes	0	0	1	1	0	0	2	0		0	0	0		0	0	0
Configuration			T	R		LT	T				LR					
Volume (veh/h)			163	72		7	37			365		73				
Percent Heavy Vehicles						3				3		3				
Proportion Time Blocked																
Right Turn Channelized	No				No				No				No			
Median Type	Undivided															
Median Storage																

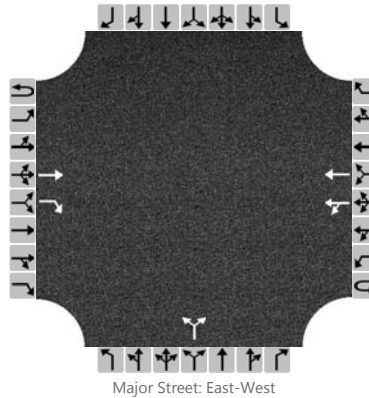
Delay, Queue Length, and Level of Service

Flow Rate (veh/h)						26						438				
Capacity						1322						784				
v/c Ratio						0.02						0.56				
95% Queue Length						0.0						3.5				
Control Delay (s/veh)						7.7						15.2				
Level of Service (LOS)						A						C				
Approach Delay (s/veh)					1.2				15.2							
Approach LOS					A				C							

HCS 2010 Two-Way Stop Control Summary Report

General Information		Site Information	
Analyst	ACY	Intersection	5- CUM with PROJ 20-CY
Agency/Co.	LLG Engineers	Jurisdiction	City of Goleta
Date Performed	11/23/2015	East/West Street	Calle Koral
Analysis Year	2025	North/South Street	Los Carneros Way
Time Analyzed	AM Peak Hour	Peak Hour Factor	1.00
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Heritage Ridge Residential Project/1-15-4119-1		

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Priority																
Number of Lanes	0	0	1	1	0	0	2	0		0	0	0		0	0	0
Configuration			T	R		LT	T				LR					
Volume (veh/h)			124	209		41	113			185		63				
Percent Heavy Vehicles						3				3		3				
Proportion Time Blocked																
Right Turn Channelized	No				No				No				No			
Median Type	Undivided															
Median Storage																

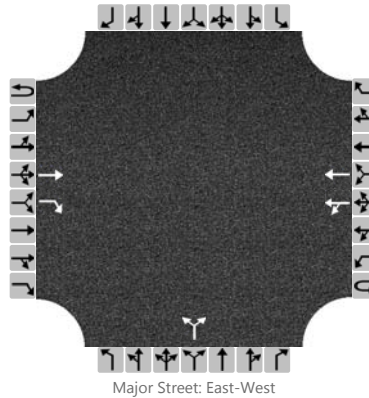
Delay, Queue Length, and Level of Service

Flow Rate (veh/h)					98					248						
Capacity					1216					727						
v/c Ratio					0.08					0.34						
95% Queue Length					0.1					1.5						
Control Delay (s/veh)					8.1					12.5						
Level of Service (LOS)					A					B						
Approach Delay (s/veh)					2.2				12.5							
Approach LOS					A				B							

HCS 2010 Two-Way Stop Control Summary Report

General Information		Site Information	
Analyst	ACY	Intersection	5- CUM with PROJ 20-CY
Agency/Co.	LLG Engineers	Jurisdiction	City of Goleta
Date Performed	11/23/2015	East/West Street	Calle Koral
Analysis Year	2025	North/South Street	Los Carneros Way
Time Analyzed	PM Peak Hour	Peak Hour Factor	1.00
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Heritage Ridge Residential Project/1-15-4119-1		

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Priority																
Number of Lanes	0	0	1	1	0	0	2	0		0	0	0		0	0	0
Configuration			T	R		LT	T				LR					
Volume (veh/h)			181	72		7	58			365		73				
Percent Heavy Vehicles						3				3		3				
Proportion Time Blocked																
Right Turn Channelized	No				No				No				No			
Median Type	Undivided															
Median Storage																

Delay, Queue Length, and Level of Service

Flow Rate (veh/h)						36						438				
Capacity						1302						755				
v/c Ratio						0.03						0.58				
95% Queue Length						0.0						3.8				
Control Delay (s/veh)						7.8						16.1				
Level of Service (LOS)						A						C				
Approach Delay (s/veh)					0.9				16.1							
Approach LOS					A				C							

Intersection No.: 5

N-S Street: Los Carneros Way

E-W Street: Calle Koral

	EXISTING TRAFFIC			EXISTING WITH PROJECT (USING 20-CY TRUCKS)			CUMULATIVE WITHOUT PROJECT			CUMULATIVE WITH PROJECT (USING 20-CY TRUCKS)		
	Volume (veh/hr)	Delay (sec/veh)	LOS	Volume (veh/hr)	Delay (sec/veh)	LOS	Volume (veh/hr)	Delay (sec/veh)	LOS	Volume (veh/hr)	Delay (sec/veh)	LOS
AM Peak Hour												
Westbound Thru/Left:	95	7.7	A	104	7.8	A	89	8.0	A	98	8.1	A
Northbound Left/Right:	51	9.7	A	51	9.9	A	248	12.1	B	248	12.5	B
Average Weighted Delay (sec/veh)		8.4	A		8.5	A		11.0	B		11.3	B
PM Peak Hour												
Westbound Thru/Left:	47	7.5	A	57	7.5	A	26	7.7	A	36	7.8	A
Northbound Left/Right:	268	11.0	B	268	11.3	B	438	15.2	C	438	16.1	C
Average Weighted Delay (sec/veh)		10.5	B		10.6	B		14.8	B		15.5	C

LINSCOTT, LAW & GREENSPAN, ENGINEERS
 600 S. Lake Avenue, Ste 500, Pasadena 91106
 (626) 796.2322 Fax (626) 792.0941

INTERSECTION CAPACITY UTILIZATION

N-S St: Los Carneros Way
 E-W St: Calle Koral
 Project: Heritage Ridge Residential Project/1-15-4119-1
 File: ICU5

Los Carneros Way @ Calle Koral
 Peak hr: AM

Date: 11/25/2015
 Date of Count: 2014

Utilizing trucks with 20-cubic yard capacity

Movement	EXIST. TRAFFIC			EXIST. WITH PROJECT				EXIST. W/PROJECT + MITIGATION				CUMULATIVE W/O PROJECT				CUMULATIVE W/ PROJECT						
	1 Volume	2 Capacity	V/C Ratio	Added Volume	Total Volume	2 Capacity	V/C Ratio	Added Volume	Total Volume	2 Capacity	V/C Ratio	Added Volume	Total Volume	2 Capacity	V/C Ratio	Added Volume	Total Volume	2 Capacity	V/C Ratio			
Nb Left	40	0	0.025	0	40	0	0.025	0	40	0	0.025	145	185	0	0.116	0	185	0	0.116			
Nb Thru	0	1600	0.032 *	0	0	1600	0.032 *	0	0	1600	0.032 *	0	0	1600	0.155 *	0	0	1600	0.155 *			
Nb Right	11	0	-	0	11	0	-	0	11	0	-	52	63	0	-	0	63	0	-			
Sb Left	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *			
Sb Thru	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000			
Sb Right	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-			
Eb Left	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000			
Eb Thru	36	1600	0.023	21	57	1600	0.036	0	57	1600	0.036	67	103	1600	0.064	21	124	1600	0.078			
Eb Right	171	1600	0.107 *	0	171	1600	0.107 *	0	171	1600	0.107 *	38	209	1600	0.131 *	0	209	1600	0.131 *			
Wb Left	41	0	0.013 *	0	41	0	0.013 *	0	41	0	0.013 *	0	41	0	0.013 *	0	41	0	0.013 *			
Wb Thru	107	3200	0.046	18	125	3200	0.052	0	125	3200	0.052	-12	95	3200	0.043	18	113	3200	0.048			
Wb Right	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-			
Yellow Allowance:			0.100 *					0.100 *					0.100 *					0.100 *				
ICU	0.252			0.252				0.252				0.398				0.398						
LOS	A			A				A				A				A						

02:24 PM

* Key conflicting movement as a part of ICU

1 Counts conducted by Quality Counts. Existing and cumulative without project traffic volumes obtained from the Heritage Ridge Project Updated Traffic Circulation and Parking Study (prepared by ATE, 10-30-15).

2 Capacity expressed in veh/hour of green

LINSCOTT, LAW & GREENSPAN, ENGINEERS
 600 S. Lake Avenue, Ste 500, Pasadena 91106
 (626) 796.2322 Fax (626) 792.0941

INTERSECTION CAPACITY UTILIZATION

N-S St: Los Carneros Way
 E-W St: Calle Koral
 Project: Heritage Ridge Residential Project/1-15-4119-1
 File: ICU5

Los Carneros Way @ Calle Koral
 Peak hr: PM

Date: 11/25/2015
 Date of Count: 2014

Utilizing trucks with 20-cubic yard capacity

Movement	EXIST. TRAFFIC			EXIST. WITH PROJECT				EXIST. W/PROJECT + MITIGATION				CUMULATIVE W/O PROJECT				CUMULATIVE W/ PROJECT						
	1	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C	Added	Total	2	V/C			
	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio	Volume	Volume	Capacity	Ratio			
Nb Left	225	0	0.141	0	225	0	0.141	0	225	0	0.141	140	365	0	0.228	0	365	0	0.228			
Nb Thru	0	1600	0.168 *	0	0	1600	0.168 *	0	0	1600	0.168 *	0	0	1600	0.274 *	0	0	1600	0.274 *			
Nb Right	43	0	-	0	43	0	-	0	43	0	-	30	73	0	-	0	73	0	-			
Sb Left	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *	0	0	0	0.000 *			
Sb Thru	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000			
Sb Right	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-			
Eb Left	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000	0	0	0	0.000			
Eb Thru	71	1600	0.044 *	18	89	1600	0.056 *	0	89	1600	0.056 *	92	163	1600	0.102 *	18	181	1600	0.113 *			
Eb Right	42	1600	0.026	0	42	1600	0.026	0	42	1600	0.026	30	72	1600	0.045	0	72	1600	0.045			
Wb Left	7	0	0.002 *	0	7	0	0.002 *	0	7	0	0.002 *	0	7	0	0.002 *	0	7	0	0.002 *			
Wb Thru	79	3200	0.027	21	100	3200	0.033	0	100	3200	0.033	-42	37	3200	0.014	21	58	3200	0.020			
Wb Right	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-	0	0	0	-			
Yellow Allowance:			0.100 *					0.100 *					0.100 *					0.100 *				
ICU	0.314			0.325				0.325				0.478				0.489						
LOS	A			A				A				A				A						

02:24 PM

* Key conflicting movement as a part of ICU

1 Counts conducted by National Data & Surveying Services. Existing and cumulative w/o project volumes obtained from Heritage Ridge Project Updated Traffic Circulation and Parking Study (by ATE, 10-30-15).

2 Capacity expressed in veh/hour of green